

Planning Department Development Application Form

Complete Application

A complete development application consists of the following:

- 1. A completed, signed, and notarized application form
- Supporting information adequate to illustrate your proposal as indicated in Section H of this application form
- 3. Written authorization from the registered owner of the subject lands where the applicant is not the owner as per Section N
- 4. Cash, debit, credit or cheque payable to Norfolk County in the amount set out in the user fees By-Law that will be accepted and deposited once the application has been deemed complete.

Pre-Submission Consultation:

Norfolk County requires a Pre-Consultation Meeting for all applications; however, minor applications may be exempted depending on the nature of the proposal. The purpose of a Pre-Consultation Meeting is to provide the applicant with an opportunity to present the proposed application, discuss potential issues, and for the Norfolk County and Agency staff to identify the application requirements. Application requirements, as detailed in the Pre-Consultation Meeting Comments, are valid for one year after the meeting date.

Development Application Process

Once an application has been deemed complete by a Planner, Norfolk County staff will circulate the application to adjacent landowners, public agencies, and internal departments for comment. The time involved in application processing varies depending on its complexity, acceptability to the other agencies, and statutory Planning Act decision time-frames.

Payment is required once your application is deemed complete. Pre-payments will not be accepted.



Norfolk County collects personal information submitted through this form under the Municipal Freedom of Information and Protection Act's authority. Norfolk County will use this information for the purposes indicated or implied by this form. You can direct questions about collecting personal information to Norfolk GIS Services at NorfolkGIS@norfolkcounty.ca.

Additional studies required for the complete application shall be at the applicant's sole expense. Sometimes, peer reviews may be necessary to review particular studies at the applicant's expense. In these caseds, Norfolk County staff will select the company to complete the peer review.

Norfolk County will refund the original fee if applicants withdraw their applications before circulation. If Norfolk County must recirculate your drawings, there will be an additional fee. If Norfolk County must do more than three reviews of engineering drawings due to revisions by the owner or failure to revise engineering drawings as requested, Norfolk County will charge an additional fee. Full refunds are only available before Norfolk County has circulated the application.

Notification Sign Requirements

For public notification, Norfolk County will provide you with a sign to indicate the intent and purpose of your development application. It is your responsibility to:

- 1. Post one sign per frontage in a conspicuous location on the subject lands.
- 2. Ensure one sign is posted at the front of the subject lands at least three feet above ground level and not on a tree.
- 3. Notify the Planner when the sign is in place.
- 4. Maintain the sign until the development application is finalized and, after that, remove it.

Contact Us

For additional information or assistance completing this application, please contact a Planner at 519-426-5870 or 519-875-4485 extension 1842 or planning@norfolkcounty.ca. Please submit the completed application and fees to the attention of the Planning Department at 185 Robinson Street, Suite 200, Simcoe, ON N3Y 5L6.



For Office Use Only: File Number Related File Number Pre-consultation Meeting Application Submitted Complete Application		Conservation Authority Fee Well & Septic Info Provided		
Che	eck the type of planning applic	ation(s) you are submitting.		
	Official Plan Amendment			
	Zoning By-Law Amendment			
	Draft Plan of Subdivision/Vac	cant Land Condominium		
	Condominium Exemption			
X	•			
	• •			
	· · · · · · · · · · · · · · · · · · ·			
	Cash-in-Lieu of Parking			
	Renewable Energy Project or Radio Communication Tower			
pro	vision on the subject lands to inc	Ilt of this application (for example, a special zoning lude additional use(s), changing the zone or official s, creating a certain number of lots, or similar)		
	This application is being submitted for the	e purpose of the site plan approval of 2 proposed buildings for Bilger		
	Slinger Services Inc This application al	so includes a gravel lot extension, 1 future storage building, and a		
		nd). This propoerty is existing and is already zoned correctly for the		
		ing changes are requested. The existing shop and showroom is also		
	to remain as part of this application.			
Pro	operty Assessment Roll Numbe	er: 54102007600		



A. Applicant Information			
Name of Owner	Bilger Slinger Services Inc. (Rob Bilger)		
Address	480 ON-3		
Town and Postal Code	Tillsonburg		
Phone Number	519-688-3315		
Cell Number			
Email	rob.bilger@icloud.com		
Name of Applicant	Peter Penner		
Address	261 Broadway		
Town and Postal Code	Tillsonburg		
Phone Number	519-688-1000		
Cell Number	519-842-0107		
Email	ppenner@cjdleng.com		
Name of Agent	N/A		
Address	N/A		
Town and Postal Code	N/A		
Phone Number	N/A		
Cell Number	N/A		
Email	N/A		
Unless otherwise directed, Norfolk County will forward all correspondence and notices			
regarding this application	to both owner and agent noted above.		
Owner	☐ Agent ■ Applicant		
Names and addresses of	f any holder of any mortgagees, charges or other		



encumbrances on the subject lands:

B. Location, Legal Description and Property Information 1. Legal Description (include Geographic Township, Concession Number, Lot Number, Block Number and Urban Area or Hamlet): Part of Lot 18, Concession 2, Geographic Township of Middleton Municipal Civic Address: 494 ON-3, Courtland, Ontario Present Official Plan Designation(s): Agricultural Present Zoning: Rural Commercial Zone 2. Is there a special provision or site specific zone on the subject lands? ☐ Yes ☐ No If yes, please specify corresponding number: 3. Present use of the subject lands: The site is currently used as a retail location and shop for Blue West Equipment Inc. 4. Please describe all existing buildings or structures on the subject lands and whether they will be retained, demolished or removed. If retaining the buildings or structures, please describe the type of buildings or structures, and illustrate the setback, in metric units, from the front, rear and side lot lines, ground floor area, gross floor area, lot coverage, number of storeys, width, length, and height on your attached sketch which must be included with your application: There are currently 2 existing buildings on the property. A retail showroom and shop for Blue West Equipment Inc. is the main building on the property and will remain as a part of this development. There is also a metal storage building which will be removed as a result of this development. 5. If an addition to an existing building is being proposed, please explain what it will be used for (for example: bedroom, kitchen, or bathroom). If new fixtures are proposed, please describe. No additions are proposed for the existing building at this time. 6. Please describe all proposed buildings or structures/additions on the subject lands. Describe the type of buildings or structures/additions, and illustrate the setback, in metric units, from front, rear and side lot lines, ground floor area, gross floor area, lot coverage, number of storeys, width, length, and height on your attached sketch

A Site Plan has been included in the application that defines all of the proposed buildings. There are 2 proposed building at this site, a bay shop for the trucks and a covered storage building that will house the trucks overnight to keep them out of the elements.

which must be included with your application:



7.	Are any existing buildings on the subject lands designated under the <i>Ontario</i> Heritage Act as being architecturally and/or historically significant? Yes No		
	If yes, identify and provide details of the building:		
	N/A		
8.	If known, the length of time the existing uses have continued on the subject lands:		
	Blue West was established originally in 2007		
9.	Existing use of abutting properties: Both are zoned agricultural and contain single detached residential dwellings.		
10	Are there any easements or restrictive covenants affecting the subject lands?		
	☐ Yes ■ No If yes, describe the easement or restrictive covenant and its effect:		
C.	Purpose of Development Application		
No	te: Please complete all that apply.		
1.	Please explain what you propose to do on the subject lands/premises which makes this development application necessary: Construct 2 buildings and 1 future building. Extend and regrade existing gravel parking area.		
	Construct new stormwater management facility and connect the outlet to the existing		
	Ronson Municipal Drain - Section B. Construct storm sewers that collect water from the		
2.	parking area and direct it towards the stormwater management facility. Please explain why it is not possible to comply with the provision(s) of the Zoning		
۷.	By-law/and or Official Plan: N/A		
3.	Does the requested amendment alter all or any part of the boundary of an area of settlement in the municipality or implement a new area of settlement in the municipality? ☐ Yes ■ No If yes, describe its effect: N/A		
4.	Does the requested amendment remove the subject land from an area of employment? ☐ Yes ■ No If yes, describe its effect: N/A		



☐ Yes ■ No If	yes, identify the policy, and also include a proposed text of the entity the policy, and also include a proposed text of the ent (if additional space is required, please attach a separate sheet):		
Description of la Frontage:	nd intended to be severed in metric units: N/A		
Depth:	N/A		
Width:	N/A		
Lot Area:	N/A		
Present Use:	N/A		
Proposed Use:	N/A		
Proposed final lo	ot size (if boundary adjustment): N/A		
If a boundary ad	If a boundary adjustment, identify the assessment roll number and property owner of		
the lands to which the parcel will be added: N/A			
Description of la Frontage:	nd intended to be retained in metric units: N/A		
Depth:	N/A		
Width:	N/A		
Lot Area:	N/A		
Present Use:	N/A		
Proposed Use:	N/A		
Buildings on reta	ained land: N/A		
Description of pr Frontage:	roposed right-of-way/easement: N/A		
Depth:	N/A		
Width:	N/A		
Area:	N/A		
Proposed use:	N/A		
<u> </u>	(s), if known, to whom lands or interest in lands to be transferred,		



9. Site Information Zoning **Proposed** Please indicate unit of measurement, for example: m, m² or % 163.5m² 30.0 Lot frontage N/A N/A Lot depth N/A N/A Lot width 1860m² 27650m² Lot area 70% 12.3% Lot coverage 13.0m 16.9m Front yard 0m* (Setback from Rail) 2.0m Rear yard 3.0m 3.0m Left Interior side yard 3.0m 30.0m Right Interior side yard 13.0m N/A Exterior side yard (corner lot) 5% 38.8% Landscaped open space 9.0m 9.0m Entrance access width 9.0m 9.0m Exit access width N/A N/A Size of fencing or screening N/A N/A Type of fencing 10. Building Size N/A N/A Number of storeys 11.0m 7.4m **Building height** 40m² (min) 1161.3m² Total ground floor area 1161.3m² 40m² (min) Total gross floor area N/A N/A Total useable floor area 11. Off Street Parking and Loading Facilities 38 Number of off street parking spaces 38 N/A Number of visitor parking spaces N/A 2 Number of accessible parking spaces 2 0 Number of off street loading facilities 0



12. Residential (if applicab	ole)		
Number of buildings existi	NI/A		
Number of buildings proposed: N/A Is this a conversion or addition to an existing building? □ Yes ■ No			
Туре	Number of Units	Floor Area per Unit in m2	
Single Detached	N/A	N/A	
Semi-Detached	N/A	N/A	
Duplex	N/A	N/A	
Triplex	N/A	N/A	
Four-plex	N/A	N/A	
Street Townhouse	N/A	N/A	
Stacked Townhouse	N/A	N/A	
Apartment - Bachelor	N/A	N/A	
Apartment - One bedroom Apartment - Two bedroom N/A N/A		N/A	
		N/A	
Apartment - Three bedroo	m N/A	N/A	
Other facilities provided (for swimming pool):	or example: play facilities, ui	nderground parking, games room,	
13. Commercial/Industrial	Uses (if applicable)		
Number of buildings existing	ng: 2		
Number of buildings proposed: 2			
Is this a conversion or add	lition to an existing building?	P □ Yes ■ No	
If yes, describe:			
Indicate the gross floor are Proposed Bay Sho		ample: office, retail, or storage):	
Proposed Covered	d Truck Storage (11	61.3m²)	
Future Storage (22	3m ²) Existing Sho	p & Showroom (865.1m²)	



Seating Capacity (for assembly halls or similar	_{r):}	
Total number of fixed seats:	N/A	
Describe the type of business(es) proposed:	Stone Slinger Services 27	
Total number of staff proposed initially:		
Total number of staff proposed in five years:	25	
Maximum number of staff on the largest shift:	7	
Is open storage required: ■ Yes □ No		
Is a residential use proposed as part of, or ac	cessory to commercial/industrial use?	
☐ Yes ■ No If yes please describe:N/A		
	·	
14. Institutional (if applicable)		
Describe the type of use proposed:	N/A	
Seating capacity (if applicable):	N/A N/A N/A N/A	
Number of beds (if applicable):		
Total number of staff proposed initially:		
Total number of staff proposed in five years:		
Maximum number of staff on the largest shift: N/A		
Indicate the gross floor area by the type of us N/A	e (for example: office, retail, or storage):	
15. Describe Recreational or Other Use(s) (if	applicable)	
N/A		



D.	Previous Use of the Property
1.	Has there been an industrial or commercial use on the subject lands or adjacent lands? ■ Yes □ No □ Unknown
	If yes, specify the uses (for example: gas station or petroleum storage): Equipment Retail and Shop
_	
2.	Is there reason to believe the subject lands may have been contaminated by former uses on the site or adjacent sites? ☐ Yes ■ No ☐ Unknown
3.	Provide the information you used to determine the answers to the above questions: Discussions with current land owner on previous uses of the property.
4.	If you answered yes to any of the above questions in Section D, a previous use inventory showing all known former uses of the subject lands, or if appropriate, the adjacent lands, is needed. Is the previous use inventory attached? Yes No
E.	Provincial Policy
1.	Is the requested amendment consistent with the provincial policy statements issued under subsection 3(1) of the <i>Planning Act, R.S.O. 1990, c. P. 13</i> ? ■ Yes □ No
	If no, please explain:
2.	It is owner's responsibility to be aware of and comply with all relevant federal or provincial legislation, municipal by-laws or other agency approvals, including the Endangered Species Act, 2007. Have the subject lands been screened to ensure that development or site alteration will not have any impact on the habitat for endangered or threatened species further to the provincial policy statement subsection 2.1.7? Yes No
	If no, please explain:
	There is no alteration to the proposed use of the site and therefore this was not required



3.	Have the subject lands been screened to ensure that development or site alteration will not have any impact on source water protection? ☐ Yes ■ No
	If no, please explain: Not required as it is not in a well head protection zone
	Note: If in an area of source water Wellhead Protection Area (WHPA) A, B or C please attach relevant information and approved mitigation measures from the Risk Manager Official.
4.	Are any of the following uses or features on the subject lands or within 500 metres of the subject lands, unless otherwise specified? Please check boxes, if applicable.
	Livestock facility or stockyard (submit MDS Calculation with application)
	☐ On the subject lands or ☐ within 500 meters – distance Wooded area
	☐ On the subject lands or ■ within 500 meters – distance 20m Municipal Landfill
	☐ On the subject lands or ☐ within 500 meters – distance
	Sewage treatment plant or waste stabilization plant
	☐ On the subject lands or ☐ within 500 meters – distance
	Provincially significant wetland (class 1, 2 or 3) or other environmental feature
	☐ On the subject lands or ☐ within 500 meters – distance Floodplain
	☐ On the subject lands or ☐ within 500 meters – distance
	Rehabilitated mine site
	☐ On the subject lands or ☐ within 500 meters – distance
	Non-operating mine site within one kilometre
	☐ On the subject lands or ☐ within 500 meters – distance
	Active mine site within one kilometre
	☐ On the subject lands or ☐ within 500 meters – distance
	Industrial or commercial use (specify the use(s))
	☐ On the subject lands or ☐ within 500 meters – distance
	Active railway line
	☐ On the subject lands or ☐ within 500 meters – distance 10m Seasonal wetness of lands
	☐ On the subject lands or ☐ within 500 meters – distance
	☐ On the subject lands or ☐ within 500 meters – distance
	Abandoned gas wells
	☐ On the subject lands or ☐ within 500 meters – distance



F.	Servicing and Access			
1.	Indicate what services are available or proposed: Water Supply			
	☐ Municipal piped water		Communal wells	
	Individual wells		Other (describe below)	
	Sewage Treatment			
	Municipal sewers		Communal system	
	■ Septic tank and tile bed in good working order		Other (describe below)	
	Storm Drainage			
	■ Storm sewers		Open ditches	
	Other (describe below)			
2.	Existing or proposed access to subject lands:			
	☐ Municipal road		Provincial highway	
	☐ Unopened road		Other (describe below)	
	Name of road/street: Ontario Highway 3			
G.	Other Information			
1.	. Does the application involve a local business? ■ Yes □ No			
	If yes, how many people are employed on the subject lands?			
2.	Is there any other information that you think may be useful in the review of this application? If so, explain below or attach on a separate page.			



H. Supporting Material to be submitted by Applicant

In order for your application to be considered complete, **folded** hard copies (number of paper copies as directed by the planner) and an **electronic version (PDF) of the properly named site plan drawings, additional plans, studies and reports** will be required, including but not limited to the following details:

- 1. Concept/Layout Plan
- 2. All measurements in metric
- 3. Key map
- 4. Scale, legend and north arrow
- 5. Legal description and municipal address
- 6. Development name
- 7. Drawing title, number, original date and revision dates
- 8. Owner's name, address and telephone number
- 9. Engineer's name, address and telephone number
- 10. Professional engineer's stamp
- 11. Existing and proposed easements and right of ways
- 12. Zoning compliance table required versus proposed
- 13. Parking space totals required and proposed
- 14. All entrances to parking areas marked with directional arrows
- 15. Loading spaces, facilities and routes (for commercial developments)
- 16. All dimensions of the subject lands
- 17. Dimensions and setbacks of all buildings and structures
- 18. Location and setbacks of septic system and well from all existing and proposed lot lines, and all existing and proposed structures
- 19. Gross, ground and useable floor area
- 20. Lot coverage
- 21. Floor area ratio
- 22. Building entrances, building type, height, grades and extent of overhangs
- 23. Names, dimensions and location of adjacent streets including daylighting triangles
- 24. Driveways, curbs, drop curbs, pavement markings, widths, radii and traffic directional signs
- 25. All exterior stairways and ramps with dimensions and setbacks
- 26. Retaining walls including materials proposed
- 27. Fire access and routes
- 28. Location, dimensions and number of parking spaces (including visitor and accessible) and drive aisles
- 29. Location of mechanical room, and other building services (e.g. A/C, HRV)
- 30. Refuse disposal and storage areas including any related screening (if indoors, need notation on site plan)
- 31. Winter snow storage location



- 32. Landscape areas with dimensions
- 33. Natural features, watercourses and trees
- 34. Fire hydrants and utilities location
- 35. Fencing, screening and buffering size, type and location
- 36. All hard surface materials
- 37. Light standards and wall mounted lights (plus a note on the site plan that all outdoor lighting is to be dark sky compliant)
- 38. Business signs (make sure they are not in sight lines)
- 39. Sidewalks and walkways with dimensions
- 40. Pedestrian access routes into site and around site
- 41. Bicycle parking
- 42. Architectural elevations of all building sides
- 43. All other requirements as per the pre-consultation meeting

addition, the following additional plans, studies and reports, including but not limited may also be required as part of the complete application submission:
Zoning Deficiency Form
On-Site Sewage Disposal System Evaluation Form (to verify location and condition)
Architectural Plan
Buildings Elevation Plan
Cut and Fill Plan
Erosion and Sediment Control Plan
Grading and Drainage Control Plan (around perimeter and within site) (existing and proposed)
Landscape Plan
Photometric (Lighting) Plan
Plan and Profile Drawings
Site Servicing Plan
Storm water Management Plan
Street Sign and Traffic Plan
Street Tree Planting Plan
Tree Preservation Plan
Archaeological Assessment
Environmental Impact Study



■ Functional Servicing Report
☐ Geotechnical Study / Hydrogeological Review
☐ Minimum Distance Separation Schedule
□ Noise or Vibration Study
☐ Record of Site Condition
□ Storm water Management Report
■ Traffic Impact Study – please contact the Planner to verify the scope required
 Site Plan applications will require the following supporting materials: Two (2) complete sets of the site plan drawings folded to 8½ x 11 and an electronic version in PDF format Letter requesting that the Holding be removed (if applicable) A cost estimate prepared by the applicant's engineer An estimate for Parkland dedication by a certified land appraiser Property Identification Number (PIN) printout
Standard condominium exemptions will require the following supporting materials:
☐ Plan of standard condominium (2 paper copies and 1 electronic copy)
☐ Draft condominium declaration
□ Property Identification Number (PIN) printout

Your development approval might also be dependent on other relevant federal or provincial legislation, municipal by-laws or other agency approvals.

All final plans must include the owner's signature as well as the engineer's signature and seal.

I. Development Agreements

A development agreement may be required prior to site plan approval, subdivision and condominium applications. Should this be necessary for your development, you will be contacted by the agreement administrator with further details of the requirements including but not limited to insurance coverage, professional liability for your engineer, additional fees and securities.



J. Transfers, Easements and Postponement of Interest

The owner acknowledges and agrees that if required, it is their solicitor's responsibility on behalf of the owner, to disclose the registration of all transfer(s) of land and/or easement in favour of the County and/or utilities. Also, the owner further acknowledges and agrees that it is their solicitor's responsibility on behalf of the owner for the registration of postponements of any charges in favour of the County.

K. Permission to Enter Subject Lands

Permission is hereby granted to Norfolk County officers, employees or agents, to enter the premises subject to this application for the purposes of making inspections associated with this application, during normal and reasonable working hours.

L. Freedom of Information

For the purposes of the Municipal Freedom of Information and Protection of Privacy			
Act, I authorize and consent to the use by or the disclosure to any person or public			
body any information that is collected under the auth	nority of the <i>Planning Act, R.S.O.</i>		
1990, c. P. 13 for the purposes of processing this ap	oplication.		
V+ VK	30 OCT 2024		
Owner/Applicant Signature	Date		
M. Owner's Authorization			
If the applicant/agent is not the registered owner of t	the lands that is the subject of this		
application, the owner(s) must complete the authoriz	zation set out below.		
I/We Rob Bilger am/a	re the registered owner(s) of the		
lands that is the subject of this application.			
I/We authorize Peter Penner	to make this application on		
my/our behalf and to provide any of my/our personal information necessary for the			
processing of this application. Moreover, this shall be your good and sufficient			
authorization for so doing.			
	30 OCT 2024		
Owner	Date		



Owner

Date

N. Declaration PETER J. PENNER OF NORFOLK COUNTY
solemnly declare that:
all of the above statements and the statements contained in all of the exhibits ransmitted herewith are true and I make this solemn declaration conscientiously believing it to be true and knowing that it is of the same force and effect as if made under oath and by virtue of <i>The Canada Evidence Act</i>
Declared before me at:
own of Tillsonburg
n County of Oxford Owner/Applicant Signature
This 30th day of October
A.D., 20 <u>24</u>
Karla Cyr.
A Commissioner, etc.
Karla Suzanne Cyr, a Commissioner, etc.,



Province of Ontario, for Cyril J. Demeyere Limited. Expires March 21, 2025.



Pre-Consultation Meeting Notes

Date: June 7, 2023

Description of Proposal: The applicant proposes a Site Plan Amendment that would facilitate

the development of a 1161 m2 warehouse that would replace a 456 m2 warehouse.

Property Location: 494 Hwy 3, Courtland

Roll Number: 541002007600

Please read all the information contained in this document, as it pertains to the requirements for future development planning applications. As a result of the information shared at the pre-consultation meeting dated June 7, 2023, the following applications and qualified professional documents / reports are required as part of a complete application. Failure to include all listed items with the application will deem the application incomplete. The County reserves the right to change, reduce or add requirements for a complete application, particularly if the submission does not match the proposal as reviewed during the pre-submission consultation meeting.

Please note that various fees are associated with each application and there are also costs for qualified professionals retained to complete various documents / reports. All requirements identified are minimum and determined as of the date of the preconsultation meeting with the information available at that time. As the proposal proceeds and more information is made available, additional applications, studies, reports, etc. may be required.

The information contained in this document is applicable for a maximum of one (1) year from the date of meeting. If an application is not received within that time frame or any component of the proposal changes, a new pre-submission consultation meeting is required.

All applications are required to include information outlined in the Presubmission consultation meeting notes; failure to include all items with the application submission without prior approval will necessitate a notice of incomplete application response by the County.

Before you submit your application, please contact the assigned Planner to arrange a pre-submission meeting to review the submission and confirm applicable fee.

As part of a complete application, a signed version of these meeting notes is required.

Proponent / Agent Name	Signature	Date

Attendance List

Proponent	Greg Eyre c/o Reid & Deleye
Community Development – Planning and Agreement	Mohammad Alam, Supervisor Development Lindsay King, Junior Planner Annette Helmig, Agreement and Development Coordinator
Building Department	Jonathan Weir, Building Inspector III Roxanne Lambrecht, Zoning Administrator
Reality Services	Karen Lambrecht, Corporate Services Generalist
Environment & Infrastructure Services – Development Engineering	Stephen Gradish, Development Technologist
Community Services – Fire	Katie Ballantyne, Community Safety Officer

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List of Application Requirements* and General Comments

Planning Department

Planning application(s) required to proceed	Required
Official Plan Amendment Application Choose an item.	
Zoning By-law Amendment Application Choose an item.	
Site Plan Application Choose an item.	X
Draft Plan of Subdivision Application	
Draft Plan of Condominium Application	
Part Lot Control Application	
Consent / Severance Application	
Minor Variance Application	

Removal of Holding Application		
Temporary Use By-Law Application		
Other - Click here to enter text.		
Planning requirements for a complete application The items below are to be submitted as part of the identified Planning Application(s). ** electronic/PDF copies of all plans, studies and reports are required**	Required at OPA/ Zoning Stage	Required at Site Plan Stage
Proposed Site Plan / Drawing		X
Planning Impact Analysis Report / Justification Report		
Environmental Impact Study Choose an item.		
Neighbourhood Plan (TOR (Terms of Reference) must be approved by the County)		
Agricultural Impact Assessment Report		
Archaeological Assessment		
Heritage Impact Assessment		
Market Impact Analysis		
Dust, Noise and/or Vibration Study		
MOE D-Series Guidelines Analysis		
Landscaping Plan		X
Elevation Plan		X
Photometrics (Lighting) Plan		X
Odour mitigation plan (in relation to Cannabis Production and Processing Facilities)		
Shadow Analysis Report		
Record of Site Condition		
Contaminated Site Study		
Minimum Distance Separation Schedule		
Parking Assessment		
Hydrogeological Study		
Restricted Land Use Screening Form		
Topographical Survey Drawing		X
Additional Planning requirement	ts	Required
Development Agreement		X
Parkland Dedication/Cash-in-lieu of Parkland		X(TBD)

^{*}The list of Planning Department requirements is based on the information submitted and as presented for this specific pre-consultation meeting. Norfolk County reserves the

right to adjust requirements including identifying additional requirements or reducing requirements. Any changes to a proposal may necessitate changes to Planning Department submission requirements. Furthermore, reports and studies are subject to peer review.

Community Development fees, applications, and helpful resources can be found can be found by visiting https://www.norfolkcounty.ca/government/planning/

Planning Comments

All comments are general and nature and subject to change pending modifications to the proposal and full analysis at the application submission stage.

Official Plan: The subject Lands are designated as 'Agricultural' in the Official Plan. The policies of the Agricultural Designation are also intended to provide the opportunity for businesses that support agricultural operations to locate on farms or in close proximity to farms. Although the proposed contractor supply and service shop may not directly related to agricultural operations, OP policies do not dictate to what is permitted in the Current Zoning.

Zoning By-Law: The subject lands are zoned as Rural Commercial (CR). Contractor supply and service shops are permitted in the CR zone.

Site Plan Control: The site is under Site Plan control and a site plan application is required under Site Plan Control By-law 2014-97.

Standard site plan components are required to be included with the application, which include:

- Key map
- Site Plan
- o Development name
- o All measurements in metric
- o All dimensions of the subject lands
- o Dimensions and setbacks of all buildings and structures
- o Gross, ground and useable floor area
- o Scale, legend and north arrow
- o Legal description and municipal address
- o Drawing title, number, original date and revision dates
- o Owner's name, address and telephone number and signature
- o Engineer's name, address and telephone number
- o Professional engineer's stamp
- o Any existing and proposed easements and right of ways
- o Zoning compliance table required versus proposed
- o Parking space totals required and proposed

- o All entrances to parking areas marked with directional arrows
- o Loading spaces, facilities and routes (for commercial & Industrial developments)
- o Building entrances, building type, height, grades and extent of overhangs
- o All exterior stairways and ramps with dimensions and setbacks
- o Names, dimensions and location of adjacent streets including daylighting triangles
- o Driveways, curbs, drop curbs, pavement markings, widths, radii and traffic directional signs
- o Retaining walls including materials proposed
- o Fire access and routes, including fire route signs
- o Fire hydrants and utilities location
- o Location, dimensions and number of parking spaces (including visitor and accessible) and drive aisles
- o Location of mechanical room, and other building services (eg. A/C, HRV)
- o Refuse disposal and storage areas including any related screening (if indoors, need notation on site plan and route used for truck pick-up)
- o Winter snow storage location
- o All hard surface materials
- o Light standards and wall mounted lights (plus a note on the site plan that all outdoor lighting is to be dark sky compliant)
- o Business signs (make sure they are not in sight-lines)
- o Pedestrian access routes into site and around site
- o Bicycle parking
- Other Features
- o Landscape areas with dimensions
- o Natural features and trees
- o Fencing, screening and buffering size, type and location
- o All hard surface materials
- o Light standards and wall mounted lights (plus a note on the site plan that all outdoor lighting is to be dark sky compliant)
- o Business signs (make sure they are not outside of the property line)
- o Sidewalks and/or walkways with dimensions

Site Plan/Development considerations:

- Please clearly show the vehicular path of travel and parking spaces, and their construction materials; Type A Barrier free parking with appropriate signs will be required;
- 2. A barrier free path of travel from the accessible parking to the building will be required:
- 3. Please include any loading areas as per the zoning by-law if proposed;
- 4. Please provide privacy fencing at the property lines adjacent to existing residential developments to the east;
- 5. Please note that CN Rail has a development guideline that you must adhere to. Please review the CN Rail Development guidelines and any required buffer.

Endangered and threatened species and their habitat are protected under the provinces Endangered Species Act, 2007 (ESA), O. Reg. 242/08 & O. Reg. 830/21. The Act prohibits development or site alteration within areas of significant habitat for endangered or threatened species without demonstrating that no negative impacts will occur. The Ministry of Environment, Conservation and Parks provides the service of responding to species at risk information requests and project screenings. The proponent is responsible for discussing the proposed activity and having their project screened with MECP (Ministry of Environment, Conservation and Parks).

Please be advised that it is the owner's responsibility to be aware of and comply with all relevant federal or provincial legislation, municipal by-laws, or other agency approvals.

[see Appendix A for additional comments]

Assigned Planner:

Mohammad Alam Supervisor Development Planning Extension 1828 Mohammad.Alam@norfolkcounty.ca

Development Engineering

Development Engineering – 494 Hwy #3, Courtland – SPA for Contractor Shop

Development Engineering requirements to proceed The below requirements are to be submitted as part of the Formal Development Planning application.	Required at OPA/ Zoning Stage	Required at Site Plan Stage	Potentially Required (See Notes Section)
General Requirements			
Concept Plan		X	
Lot Grading Plan		X ²	
Siltation and Erosion Control Plan		X ²	
General Plan of Services		X ²	
Geotechnical Report			X ¹²
Functional Servicing Report		X ³	

Storm Water Servicing Requirements – Storm Criteria and ISMP Section 4.0	Section 7.0 and Section 8 Norfolk County
Storm Water Management Design Report (including calculations)	X ⁴
Storm Water Drainage Plan	X ⁵
Establish/Confirm Legal and Adequate Outlet	X ₆
Anticipated Flow/Analysis to Receiving Collection System	X ⁷
Municipal Drainage	X8,9
Transportation Requirements – Section Section 5.0, Section 6.0 and Appendix J	6.0 Norfolk County Design Criteria, ISMP
Traffic Impact Study	X ¹⁰
Improvements to Existing Roads & Sidewalk (urbanization, pavement structure, widening sidewalk replacement, upgrades, extension and accessibility)	X ¹¹

General Notes:

- Securities will be required in the form of a schedule. Any works completed within the Municipal Right-of-Way (R.O.W.) is to be shown as 100% security. Any works completed within private property are to be shown as 10% security. This can be submitted at time of Site Plan.
- All reports and plans are to be signed and stamped by a Professional Engineer (P.Eng.).
- All reports are to be completed in reference to Norfolk County's Design Criteria and Integrated Sustainable Master Plan (ISMP).
- Recommendations from all reports must be incorporated into the engineering design.
- Norfolk County's Design Criteria can be provided.

Required at Zoning Amendment Stage:

In Development Engineering review of this submission for Pre consultation, Development Engineering has assumed the current Zoning is adequate and have no comments specific to a Zoning Amendment. If a Zone change is ever required Development Engineering would have additional comments.

Required at Site Plan Amendment Stage Notes:

1. A Concept Plan

- 2. Lot Grading Plan, Siltation and Erosion Control Plan, and General Plan of Services drawing (showing Domestic Well and Septic system locations) can be shown on one engineering plan as long as it's legible for review.
- 3. A Functional Servicing Brief (FSB) will be required. The FSB shall explain the proposed water and wastewater needs of the site and confirm that the future Well and septic are adequate to support the development.
- 4. A Stormwater Management Report is to be completed as per Norfolk County Design Criteria Section 7.
- 5. A Storm water Drainage Plan will be required to identify any external Storm flows to the Site and also identify all areas of this site that discharge Storm water to neighboring properties.
- 6. The developer is responsible to establish and confirm a legal and adequate Storm water outlet. At the Pre-consultation meeting it was mentioned this property is part of the Ronson A and Ronson B Municipal Drains.
- 7. Anticipated Flow/Analysis to Receiving Collection System (Ronson A and Ronson B Municipal Drain) will be required.
- 8. The subject lands are within the Ronson A and Ronson B Municipal Drain Watershed. Prior to approval of site plan, the site must comply with all conditions of the Drainage Act.

During detailed engineering review of the site plan the overall development will be reviewed for changes to the Watershed area. Any changes to the Municipal Drain or drainage area changes due to the Intensification or increased flows will require Drainage Act compliance. For questions or concerns pertaining to the Drainage Act please contact Bill Mayes, Drainage Superintendent, Norfolk County, bill.mayes@norfolkcounty.ca.

9. A Drain report may need to be amended to address the changes as a result of this development. The extent of which will be highly dependent on the final development servicing and where overland flows are being discharged. Any or all changes to the drain or drain report will be the requirement of the proponent.

The drain will accommodate drainage from the development but to the extent of the existing conditions which would be considered a limited outlet. The drain is designed to an agricultural drain design standard which may have considered this property as farmland. Any additional runoff will need to be managed on-site or alterations to the drain through an engineer report under the *Drainage Act*.

10. As per Norfolk County's Integrated Sustainable Master Plan (ISMP) – Appendix J: Traffic Impact Study (TIS) Guidelines, a Traffic Impact Study should be required with every planning application. As this site fronts onto HWY #3 all requirements for a TIS will be as per the MTO standards. Please ensure Norfolk County receives a copy of the TIS prepared for MTO. This report will be accepted as information.

11. All requirements for any proposed upgrades to the Entrances will be determined by MTO. All permits are to be applied for with MTO. Norfolk County standards are not required for Width and Surface upgrades as Norfolk County is not the Road Authority at this location.

Potentially Required Notes:

12. A Geotechnical report must be submitted if Storm water management practices involving infiltration are proposed.

Stephen Gradish
Development Technologist
Extension 8015
Stephen.Gradish@norfolkcounty.ca

Agreements

A recommended condition of your planning application approval could be to enter into a development agreement with the County that will be registered on title to the subject lands, at the Owner's expense. The additional requirements for a development agreement could include, but are not limited to the following:

- Engineering drawing review
- Engineer's schedule of costs for the works
- Clearance letter and supporting documentation to support condition clearance
- User fees and performance securities
- Current property identification number (PIN printout) (can be obtained by visiting https://help.onland.ca/en/home/)
- Owner's commercial general liability insurance to be obtained and kept in force during the terms of the agreement
- Postponement of interest. If there are mortgages / charges on your property identifier, your legal representative will be required to obtain a postponement from your bank or financial institution to the terms outlined in your development agreement
- Transfers and / or transfer easements along with registered reference plan

Annette Helmig
Agreement and Development Coordinator
Extension 8053
Annette.Helmig@norfolkcounty.ca

Building

Zoning Administrator:

Zoned CR (Rural Commercial)

- -proposed replacement of existing 456sqm structure with a new 1161sqm contractor supply and service shop
- -needs to meet setbacks to property lines
- -13m front
- -3m interior side
- -9m rear
- -needs to be less than the maximum height of 11m
- -requires minimum 33 parking spaces (1 per every 35sqm of usable floor)
 - -parking spaces should be 3m x 5.8 m
- -also requires at least 1 type A and 1 type B accessible parking spaces
- -type A accessible space needs to be at least 3.4m x 5.8m with an access isle
- -note that special provision 14.353 permits an accessory showroom (restricted in size) in the other building on the property, does not apply to this proposed building
- 14.353 In addition to the uses permitted in the CR Zone, a metal working shop and welding shop for steel and accessory showroom shall also be permitted provided the maximum usable floor area of the building be restricted to what was existing on December 10, 1994. Further, the sale of used vehicles for the carrying of passengers or the transport of goods, and the sale of used mobile construction equipment shall also be permitted.
- -include a zoning table on the site sketch that lists the requirements and the proposed parking, setbacks and height
- -also include the parking spaces on the site sketch with dimensions, including labelling accessible spots
- **3.5 Building Separation from Railways** The pertinent side yard and rear yard provisions contained herein shall not apply where an interior lot line is in common with a railway right-of-way boundary in which case the minimum separation distance between any building or structure and the railway rightof-way boundary shall be as follows:
- a) for any dwelling unit, institutional use, hotel, motel, motor hotel, hospital, school 15 metres;
- b) for any industrial, commercial or agricultural building requiring direct rail loading facilities, or for any building or structure accessory to any permitted use 0 metres;
- c) all other buildings 6 metres.

Roxanne Lambrecht Zoning Administrator Extension 1839 Roxanne.Lambrecht@norfolkcounty.ca

Building Inspector:

The proposed construction is considered an F2-Industrial occupancy with possible E-Retail and D-Office type occupancies as defined by the Ontario Building Code (OBC). You will need to retain the services of an Architect or a Professional Engineer to complete the design documentation for this application.

The Designer will need to provide a Part 3 Building Code matrix. This matrix represents selected elements from your detailed code analysis and presents a quick overview to the municipal building official of the key OBC factors concerning your design. The matrix will identify OBC review items such as occupant loads, fire separations, project description, building size, building classification, fire alarms, type of construction, barrier free requirements, plumbing fixture requirements and spatial separations.

The Designer will need to view all aspects of OBC Sentence 3.2.5.7. Water Supply in regards to firefighting. This will include the expanded requirements and water supply tables from the OBC Appendix for Water Supply, where municipal water supply capacities are limited, it will be necessary to have on-site supplemental water supply. Greg has asked about using the pond from the West property for firefighting water supply, the OBC does not comment on this. I would recommend contacting the Planner in charge to discuss what may be required in terms of an easement to access the water from the neighboring property, this is due to the fact either property could be sold and the service/operation of the fire service main must be maintained.

A demolition permit will be required for any buildings being removed.

A qualified individual with a BCIN# for On-Site Sewage Systems would be required to complete the design for any proposed new septic system. The septic system must be sized for the proposed flow and all OBC Part 8 clearances will be required. Municipal septic permits/inspections are for the sum of all septic systems on the property up to a flow of 10,000L, above which requires MOE approvals.

Signs proposed due to development will require a permit as part of Norfolk's Sign Bylaw and may need a Building Permit according to the OBC.

Items for Site Plan

Site plan drawings need to have enough detail, to determine compliance with the code references listed.

- 1. Indicate location of access route and access route design [OBC 3.2.5.4 to 3.2.5.6]
- 2. Revised fire water pond design and calculations. [OBC 3.2.5.7]
- 3. Indicate location of existing and new fire department connections. Dimensions between hydrants and building entrances is required.

[OBC 3.2.5.16]

- 3. Location and specifications of exterior lighting. Lighting to be included in SB-10 report energy efficiency
- 4. Indicate barrier free path of travel from parking area to building entrance. Construction of curb cuts and location of tactile attention indicators is required. [OBC 3.8.1.3, & 3.8.3.2]
- 5. Location of revised septic system (if required)
- 6. Provide building elevations and cross section, showing building massing, location of proposed entrances and exits, barrier free controls, exterior lighting locations, and exterior signage. [Planning Act 41(4).2]

Items for Building Permit

"-Industrial Commercial Institutional (ICI)" & "Applicable Law Checklist" Step by Step Guide Building Permit Package has been attached to the minutes herein, this contains information on drawing requirements, designers, forms, contact information for Building Department etc.

If you have any questions on the building permit process or plans required, please contact permits@norfolkcounty.ca or 226-NORFOLK (226-667-3655) ext. 6016

Jonathan Weir Building Inspector III Extension 1832 jonathan.weir@norfolkcounty.ca

Corporate Support Services – Realty Services

Click here to enter text.

If a Development Agreement is required, then the County will require postponements of any charges/mortgages (if any) on title to the County's Development Agreement. We recommend that you contact your Lender(s) (if any) and/or your solicitor as early in the process as possible to avoid any delays.

If access to neighbouring property is being requested, an easement may be required to be registered on title to both the subject and neighbouring properties. We recommend that you contact your solicitor to discuss the process of preparing/registering such an easement on title.

Karen Lambrecht
Corporate Services Generalist, Realty Services
Extension 8140
karen.lambrecht@norfolkcounty.ca

Fire Department

Norfolk County Fire Department has the following comments for this proposal:

- Ensure adequate access for fire apparatus Katie Ballantyne Community Safety Officer Extension 2423 Katie.Ballantyne@norfolkcounty.ca

External Agencies Ministry of Transportation

The subject property is located adjacent to Highway 3, which is classified as a *Provincial Arterial* and designated as a *Controlled Access Highway* (CAH). As such, all requirements, guidelines and best practices in accordance with this classification and designation shall apply.

The owner should be aware that the property falls within MTO's Permit Control Area (PCA), and as such, MTO Permits are required before any demolition, grading, construction or alteration to the site commences. In accordance with the Ontario Building Code, municipal permits may not be issued until such time as all other applicable requirements (i.e.: MTO permits/approvals) are satisfied. As a condition of MTO permit(s) MTO will require the following for review approval:

Site Plans;

An MTO Building and Land Use Permit is required. As a condition of MTO permits, the following shall be provided:

• The Proponent shall submit an acceptable Site Plan, Grading Plan, Illumination Plan, Drainage Plan and Site Servicing Plan for MTO review and approval. These plans shall clearly identify all structures/works (existing and proposed). MTO requires all buildings, structures and features integral to the site to be located a minimum of 14 metres from the highway property limit (along Highway 3), inclusive of fire-lanes, parking and stormwater management facilities.

Traffic Impact Study

MTO will require a Traffic Impact Study (TIS). The TIS shall be completed by a Registry, Appraisal and Qualification System (RAQS) approved Transportation Engineer in accordance with MTO traffic guidelines. Should highway improvements be identified, it shall be the responsibility (financially and otherwise) of the owner to acquire any required property, and to design and construct the works in accordance with MTO standards, specifications, and criteria.

Additionally, it must be noted that if the TIS identifies that highway / intersection improvements are required, but our review indicates that the required improvements cannot be geometrically accommodated, then MTO may be unable to approve the application, and MTO permits would not be available.

Entrance/Access Management

- Interconnection to adjacent properties will not be permitted. As a condition of MTO permits, fencing, berms or any other kinds of physical barriers shall be constructed prohibiting vehicular traffic between this and adjacent properties.
- In accordance with MTO's Highway Access Management Manual guidelines, the property is limited to (1) one entrance only, and all other entrances shall be removed.
- Due to the intensification of the site, any required entrance modifications to the site must be supported by the TIS. Modifications to the entrance layout shall meet the width, shape and radii design standard of MTOD 305.140 and/or comply with the requirements identified in the TIS. Additionally, the new modified entrance is required to be located on the far east limits of the site in order to accomplish a more appropriate offset from Goshen Road.

Storm Water Management Report (SWMR);

As a condition of MTO permits, to ensure that stormwater runoff from this property does not adversely

affect our highway drainage system or highway corridor, MTO may require the owner to submit a Storm Water Management Report (SWMR) report along with the above-noted grading/drainage plans for the proposed development for our review and approval.

The drainage engineer/consultant should refer to the ministry website for applicable IDF curves, Highway Drainage Design Standards (2008) and the ministry's Stormwater Management Requirements for Land Development Proposals. https://www.ontario.ca/page/resources-transportation-planners#section-5

• The drainage engineer/consultant shall ensure that all return periods are assessed (2yr, 5yr, 10yr, 25yr, 50yr, 100yr and Regional).

Signs

Any/all signage visible from Highway 3 including temporary development signs, must be identified on the plans, must conform to MTO policies and guidelines, and will require a valid MTO Sign Permit before installation. The minimum Commercial Signage setback is 3.0m.

Encroachments

Any encroachments and proposed work within the Highway 3 property limits are subject to MTO conditions, approval and permits, prior to construction. All provincial highway property encroachments are strictly regulated and must meet all conditions set out by MTO. MTO Encroachment permit may be required to perform entrance modification/restoration works within the MTO property limits.

MTO looks forward to receiving additional materials for assessment as the project advances. Please feel free to contact me directly should you have any questions or concerns.

Michael Kilgore - Corridor Management Planner (A)
Ministry of Transportation
226-973-7021
Michael.kilgore@ontario.ca

Appendix A: Planning Reference Materials

Following is a summary of some land use planning reference materials. It is the requirement of the applicant to ensure compliance with applicable legislation, policies and regulations.

Provincial Policy Statement, 2020

https://www.ontario.ca/page/provincial-policy-statement-2020

Norfolk County Official Plan

https://www.norfolkcounty.ca/government/planning/official-plan/

Section 9.6.1 outlines requirements in relation to requests to amend the Official Plan.

Section 9.6.2 outlines requirements in relation to requests to amend the Zoning By-law.

It is the responsibility of the proponent to review and ensure relevant Official Plan policies are addressed in any future development application.

Norfolk County Zoning By-Law 1-Z-2014

https://www.norfolkcounty.ca/government/planning/new-zoning-by-law/

The provisions of the Norfolk County Zoning By-Law shall apply to all lands within the boundaries of Norfolk County. No land, building or structure shall be used, erected, or altered in whole or in part except in conformity with the provisions of this By-Law. No land, building or structure shall be used or occupied except for uses that are specifically identified in the By-Law as permitted uses by the relevant zoning category.

It is the responsibility of the proponent to review and ensure relevant Zoning Bylaw provisions are addressed in any future development application





REQUIRED INFORMATION

Name of Owner			
Property Legal Description			
Roll Number			
PIN Number			
Type and Number of Units			
Single Detached			
Semi-Detached			
Duplex			
Triplex			
Four-plex			
Street Townhouse			
Stacked Townhouse			
Apartment_			
Transfer Easements Block Number and Purpose			
Transfer Block Number and Purpose			
Geotechnical Report prepared for Lands	O YES	O NO	O UNKNOWN
Lands are Within the Source Water Protection Area	O YES	\bigcirc NO	O NNKNOMN
Lands Contain any Contaminated or Impacted Soil	O YES	O NO	O NNKNOMN
Lands Contain any Natural Watercourse	O YES	ON O	O NNKNOMN
Lands Contain any Wetlands	O YES	O NO	O NNKNOMN
Lands Contain any Archaeological Sites	YES	O NO	O NNKNOWN
Lands Contain an Existing Well and or Septic Field	YES	\bigcirc NO	O UNKNOWN
Species at Risk Branch MECP Screening	YES	O_{NO}	OUNKNOWN
Lands Contain any Endangered Species	O YES	\bigcirc NO	O UNKNOWN
OWNER INFORMATION			
NAME AND CONTACT			
ADDRESS WITH POSTAL CODE			
PHONE NUMBER			
EMAIL			
AGENT INFORMATION			
NAME AND CONTACT			
ADDRESS WITH POSTAL CODE			
PHONE NUMBER			
EMAIL			

AGREEMENT SERVICES

SITE PLAN



ENGINEER INFORMATION	
NAME AND CONTACT	
ADDRESS WITH POSTAL CODE	
PHONE NUMBER	
EMAIL	<u>—</u>
LAWYER INFORMATION	
NAME AND CONTACT	
ADDRESS WITH POSTAL CODE	
PHONE NUMBER	
EMAIL	
INSURANCE PROVIDER INFORMATION	
NAME AND CONTACT	
ADDRESS WITH POSTAL CODE	
PHONE NUMBER	
EMAIL	
FINANCIAL INSTITUTION INFORMATION (IF APPLICABLE)	
NAME AND CONTACT	
ADDRESS WITH POSTAL CODE	
PHONE NUMBER	<u> </u>
EMAIL	<u> </u>
MORTGAGEE INFORMATION (IF APPLICABLE)	
NAME AND CONTACT	
ADDRESS WITH POSTAL CODE	

SPECIES AT RISK SCREENING

PHONE NUMBER_____

The Ontario Endangered Species Act inquiries and Species at Risk screening are now handled by the Ministry of the Environment, Conservation and Parks, specifically the "Species at Risk Branch" and the new e-mail address for handling these inquiries is now SAROntario@ontario.ca.

TRANSFERS, EASEMENTS AND POSTPONEMENT OF INTEREST

The owner acknowledges and agrees that, it is their solicitor's responsibility on behalf of the owner for the registration of all transfer(s) of land to the County, free and clear of any charges or encumbrances, and/or transfer(s) of easement in favour of the County and/or utilities at no cost to the County. In addition, the owner further acknowledges and agrees that it is their solicitor's responsibility on behalf of the owner for the registration of postponements of any charges to the County's agreements.

AGREEMENT SERVICES

SITE PLAN



INSURANCE CERTIFICATES

OWNER'S AUTHORIZATION

Prior to the execution of the development agreement, the owner shall at their expense obtain and keep in force, during the term of this development agreement, commercial general liability insurance coverage satisfactory to the County. The owner further acknowledges and agrees that he/she has authorized the County to discuss with their insurance provider the specific insurance requirements of the County for agreement purposes. In addition, the County will require any professionals hired to carry professional liability insurance to provide coverage for acts, errors and omissions arising from their professional services performed.

I/Wesubject of this site plan agreement.	am/are the registered owner(s) of the lands that is the
	to provide information on my/our behalf on necessary for the processing of this site plan agreement. uthorization for so doing.
<u> </u>	provide and receive information on my/our behalf in redit and agreement registration of my/our development.
,	arges or mortgage holders on the property they will be added ired to postpone their interest on the property to the County's
Owner Signature	Date

To start your agreement, please return the required supporting information and fees along with the first three pages of this document completed and signed. Provide your payments by the mail or courier to the address below or drop off at ServiceNorfolk customer service desk on the first floor 185 Robinson Street, Simcoe ON N3Y 5L6 Monday to Friday from 9 am to 4 pm. Please make your cheque payable to the Corporation of Norfolk County. If paying by credit card please contact ServiceNorfolk at 519 426-5870 Ext. 4636.

CONTACT FOR FURTHER INFORMATION AND QUESTIONS

Annette Helmig, Agreement and Development Coordinator Norfolk County, Community Development Division, Planning Department, Agreement Services 185 Robinson Street Suite 200, Simcoe ON N3Y 5L6 226.777.1445

annette.helmig@norfolkcounty.ca

The information submitted on this form is collected under the authority of the Freedom of Information and Protection of Privacy Act (FIPPA) and Municipal Freedom of Information and Protection of Privacy Act (MFIPPA) for Norfolk County employees to use for the purpose of preparing and registering a development agreement. Questions about the collection of personal information through this form may be directed to the Agreement and Development Coordinator or Information and Privacy Coordinator, Corporation of Norfolk County, 50 Colborne Street South, Simcoe ON N3Y 4H3.



AGREEMENT SERVICES SITE PLAN

DOCUMENTATION AND FEES REQUIRED

	Owner's agreement authorization
	Postponement of interest from mortgagee / chargee (if applicable)
	Current parcel register (property identifier or PIN printout)
	Owner's commercial general liability certificate of insurance
	Construction estimates (100% for external works and 10% of internal works)
	Professional liability insurance for surveyor and / or engineer
	Final reference plan for any easements and lands to be conveyed
	Letter from owner requesting holding (H) symbol be removed from the subject lands
	Letter of credit or certified cheque for performance securities
\Box	Current property taxes paid
essence	User fees (according to the By-Law in effect at the time that payment is made). If time is of the se, a certified cheque is requested otherwise it will take three weeks for the cheque to clear our financial
instituti	
	\$2,780 for preparation of the site plan agreement
	\$924 to remove the holding from the zoning on the property (if applicable)
	\$447 for financial administration of this agreement
	\$542 per tree cash-in-lieu of trees (if applicable)

2% or 5% land appraisal cash-in-lieu of parkland as per consolidated by-law 2016-126 (if applicable)



APPLICABLE LAW CHECKLIST

The Building Code Act prohibits the issuance of a building permit if the proposed construction or demolition will contravene an applicable law as defined by the Building Code. The questions below will help you to determine if an applicable law applies to your project. No timeframe for building permit review can be established until all required applicable law approvals are completed and the approval documents are submitted to the Building Division.

If the answer is **YES** to any question, the relevant approval documents must be submitted with this permit application. Where any required approval has **NOT** been obtained, the agencies listed on the back of this form must be contacted to obtain approval, and the declaration on the bottom of this form must state accordingly.

Property Address: Permit Number (office use)		
Zoning By-Laws – Norfolk County Planning Department	YES	NO
Is/was relief required to permit a minor zoning variance in your proposal?		
Is/was rezoning required to permit the proposed building or land use?		
Is a land division or subdivision required and not yet fully completed?		
Are municipal services required but not yet completed or available?		
Planning Approval - Norfolk County Planning Department		
	YES	NO
Is this property regulated by Site Plan Control under Section 41 of the Planning Act?		
Heritage - Norfolk County Heritage and Culture Department		
	YES	NO
Are you demolishing a building that is listed on the County's heritage inventory?		
Is the building designated or in the process of being designated?		
Is the property located in a heritage district or study area?		
	1	I
Construction and Fill Permits – Long Point Regional or Grand River Conservation Authority	YES	NO
Is the property located within a regulated area (i.e. abutting a ravine, watercourse, wetland, or shoreline)?		
Duilding and Land Has Daggite Outside Ministry of Transportation		
Building and Land Use Permits - Ontario Ministry of Transportation	YES	NO
Is the property within 45m of a highway or 180 m from any highway intersection?		
Is the property within 395m of a controlled highway intersection? (applies to Sign Permits)		
Is this a major traffic generating project located within 800m of a highway?		
Clean Water Act – Public Works	YES	NO
Is the property located within a Source Water Protection regulated area?		
If yes: does a Water Source Protection Plan restrict the land use you are proposing? (s.59 screening form may be required)		

Agri	culture and Farms - Ontario Ministry of Agriculture and Food	YES	NO
Is thi	s a farm building that will house animals or manure?		
Is thi	is a milk processing plant?		
Crov	wn Lands Work Permit – <i>Ministry of Natural Resources</i>	YES	NO
	you proposing to construct or place a structure or combination of structures that are in physical		
	act with more than 15 square meters of shore lands?		
Are y	you proposing to build on Crown Land?		
Elec	trical Conductor Clearances - Electrical Safety Authority	YES	NO
Are a	any overhead power lines located above or within 5.5 metres of the proposed building?		
Envi	ronmental Approvals - Ministry of Environment, Conservation, Parks	YES	NO
	Record of Site Condition required to be filed because of a change to more sensitive land use? Is property a former waste disposal site?		
	s project a major industrial, commercial, or government project?		
Is thi	s a renewable energy project?		
Does	s this property have a Certificate of Property Use under the Environmental Protection Act?		
Ob II	d Company of State of	\/F0	NO
	d Care Centres - Ministry of Education	YES	NO
Is a c	daycare proposed in any part of the building?		
Seni	ors Centres - Ministry of Children, Community and Social Services	YES	NO
Is thi	s a seniors project where Ontario Government funding is being sought?		
Long	g Term Care Centres – Ministry of Health & Long Term Care	YES	NO
Cons	struction, alteration or conversion of building used for a nursing home?		
Educ	cation Act - Ministry of Education	YES	NO
Is the	e project being carried out on the property of an educational facility?		
If so,	, is any or all building on the property being fully or partially demolished?		
	ARATION – I have considered the list of applicable laws in the Ontario Building Code as described aby declare that:	ove, and	do
	None of these applicable law approvals apply to this project		
	Applicable laws check 'yes' apply to this project, and approval documents are submitted with this	applicati	on.
	Applicable laws checked 'yes' apply to this project; however, all approval documents have not yet	been ob	tained
	Iformation provided on this form is true to the best of my knowledge. I have authority to act on beh r, corporation, or partnership with respect to this application (if applicable).	nalf of the	е
Nam	e: Signature: Date:		

Approvals from other agencies are required in many instances before a building permit can be processed and issued. These approvals are **NOT** administered by the Building Department. The fastest way to obtain a building permit is to ensure that all other required approvals are completed prior to permit application. The Building Department is required by law to prioritize applications that are fully complete in terms of applicable law approvals and document submissions. Building permit documents must be consistent with applicable law approvals. If you answer yes to any of the following question please reach out to these agencies for approvals.

Zoning and Planning - Community Services Division - Norfolk County

Zoning 519-426-5870 ext. 6064 or <u>zoning@norfolkcounty.ca</u> **Planning** 519-426-5870 ext. 1842 or planning@norfolkcounty.ca

Planning Act, s.34, 34(5), 45, and Part VI

Zoning By-laws restrict such things as land use, lot size, building size, and setbacks. If your project does not comply with any part of the Zoning By-law, a minor variance or rezoning must be obtained before any building permit can be issued. Zoning By-laws also restrict the issuance of permits until any associated land division, subdivision, or municipal servicing is complete.

Planning Act, s.41

Site Plan Approval applies to commercial, industrial, institutional, multi-residential and intensive livestock site plans. The site plan agreement must be registered before site plans will be approved.

Conservation Authority Permits

Grand River Conservation Authority (GRCA)1-866-900-4722 or grca@grandriver.ca
Long Point Regional Conservation Authority (LPRCA) 1-888-231-5408 or conservation@lprca.on.ca

Conservation Authorities Act s. 28 (1)(c), regulation 166/06

Development within certain conservation regulated areas requires a construction and fill permit from the conservation authority before any building permit can be issued. GRCA or LPRCA will confirm if your property falls within their jurisdiction.

Highway Corridor Building & Land Use Permits

Ministry of Transportation (MTO) 1-800-268-4686 or

www.mto.gov.on.ca/english/highway-bridges/highway-corridor-management/index.shtml

Public Transportation and Highway Improvement Act, s.34, 38

Ministry authorization is required for construction of all buildings within certain distances of a highway or intersection. The requirement for Ministry authorization extents to 800m from a highway where development will generate major traffic, such as a shopping centre.

Environmental Approvals

Ministry of the Environment, Conservation and Parks (MECP)1-800-461-6290 or www.ontario.ca

Environmental Protection Act s. 46, 47.3, 168 and the Environmental Assessment Act s 5.

Ministry of Environment approvals are required where a property of industrial or commercial use is changed to more sensitive residential or parkland use, for major government, industrial and commercial projects where defined by regulation, properties formerly used for landfill or waste disposal, or renewable energy projects.

Electrical Conductor Clearances

Electrical Safety Authority 1-877-372-7233 or www.esasafe.com

Subsection 3.1.19. of the Ontario Building Code prohibits buildings being located beneath or within a certain minimum distances of overhead electrical conductor wires, other than the power feed to the building.

Source Water Protection - Environmental and Infrastructure Services - Norfolk County

Environmental Services – Stephanie Davis- Manager, Water & Wastewater Compliance- 519-426-5870 ext. 8037 or Stephanie.Davis@norfolkcounty.ca

Cambium Inc. Racheal Doyle – <u>sourcewaterprotection@cambium-inc.com</u>

Clean Water Act s. 59

Special land use restrictions may apply if a water source protection plan is in effect in the area where the building is located. Uses affected by these restrictions require the approval of the designated Risk Management Official

Agriculture and Farms

Ministry of Agriculture Food and Rural Affairs 1-877-424-1300 or www.omafra.gov.on.ca

Nutrient Management Act 2002 s.11 reg 267/03, Milk Act s.14

Buildings or structures that house animals or store manure may trigger a requirement for a nutrient management strategy approved by the Ministry. The Ministry must determine that a milk processing plant is necessary and authorize it before a building permit can be issued.

Child Care Centres

Ministry of Education (905) 895-9192 or www.ontario.ca

Child Care and Early Years Act, s. 14 reg 137/15

Ministry plan approval is required if a new building is proposed to be used as a day nursery, an existing building is proposed to be used, altered or renovated for a day nursery, or if an existing day nursery is altered or renovated.

Seniors Centres

Ministry of Children, Community and Social Services 1-888-789-4199 or www.mcss.gov.on.ca

Elderly Persons Centres Act s. 6 of reg 314

Reports must be submitted to the Minister and approval obtained for all seniors centres to which government funding applies.

Long Term Care Homes

Ministry of Health & Long Term Care 1-800-387-5559 or www.health.gov.on.ca

Nursing Home Act s. 4, 5 reg 832

Homes for the Aged & Rest Homes Act s. 14

The Long Term Heath Care Act is designed to help ensure that residents of long-term care homes receive safe, consistent, high-quality, resident-centred care.

Education

Ministry of Education (905) 895-9192 or www.ontario.ca

Education Act s. 194

The board shall obtain approval from the Minister for the demolition of any buildings located on a school site regulated by the Education Act. App

Crown Lands Works Permits

Ministry of Natural Resources www.ontario.ca/page/crown-land-work-permits

Ontario Regulation 239/13 s. 2, s. 5

Ministry approval is required to construct a building on crown lands or to construct or place a structure along shorelines.

INDUSTRIAL COMMERCIAL INSTITUTIONAL

Building Permit Package

A step by step guide for making a building permit application





Norfolk County Building Department Community Development Division 185 Robinson Street, Suite 200 Simcoe, Ontario, N3Y 5L6 norfolkcounty.ca



Industrial, Commercial, Institutional Large Residential Permit Package

Building permits help protect you, your home, and the interests of your community by making sure the project is structurally sound and follows the Ontario Building Code, municipal zoning and other applicable laws.



There are multiple steps to the building permit process. The purpose of this permit package is to highlight these steps and provide guidance to the building permit process.

STEP 1: Applicable Law.

Approvals from other agencies are required in many instances before a building permit can be processed and issued. These approvals are **NOT** administered by the Building Department. The fastest way to obtain a building permit is to ensure that all other required approvals are completed prior to permit application.

An Applicable Law Checklist is required as part of a complete application. Agency contacts are attached with this form. Our community mapping has many of these layers mapped to help you determine if additional approvals are required for your application.

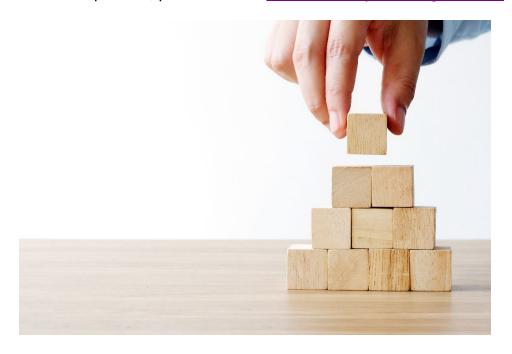
Pre-consultation meeting - Site plan approval.

Most industrial, commercial and institutional buildings are located on properties where a site plan approval is required through the Planning Act. Before you submit a planning application, please contact our office about the necessity of a pre-consultation meeting.

These confidential meetings are hosted by the Planning Department with staff from various departments (and agencies, as applicable) who will provide valuable feedback on your proposal and outline what will be required as part of a complete planning application. After the meeting, you will receive a detailed summary of the meeting which will summarize the feedback from each department. The document will also include a clear list of what you need to submit as part of a complete planning application form.

To request a pre-consultation meeting, please email the Planning Department at precon@norfolkcounty.ca. You can also call us at (519) 426-5870 ext. 1842.

For more information on the process, please see the Norfolk County Planning Website.



Site Plan Control.

Site Plan Control is a tool utilized by the County to ensure that specific development proposals meet the Official Plan and Zoning By-law objectives. Essentially, site plans approved under this process are very detailed and outline precisely how a particular property will be developed. Site Plan Approval is typically the last planning approval necessary before the Building Permit process.

A Site Plan Agreement is sometimes required for more complicated developments. Where a property has site plan control designation, but the proposed construction is deemed minor in nature, the applicant will need to provided a site plan waiver from the Planning Department as part of an Applicable Law review.

Zoning Requirements.

Finding the zoning associated with your property is easy with our <u>GIS Community Web Map</u>, position over your property and turn on the zoning layer by clicking layer list, planning, zoning. <u>Norfolk County Zoning Bylaw</u> is available online.



To confirm your project conforms to the Zoning By-law you will need to provide a plot plan indicating:

- Property lines and lot dimension,
- Location of building and all other structures on the lot,
- Location of all steps and landings,
- □ Distance from dwelling to property lines,
- Parking spots with dimensions,
- Location of septic system.

If your proposed building / structure does not comply with the zoning requirements, a planning application will be required. Zoning and Planning approval is required as part of a complete permit application.

Planning Department: planning@norfolkcounty.ca or 519-426-5870 ext. 1842. Zoning: zoning@norfolkcounty.ca or 519-426-5870 ext. 1000.



Lot Grading.

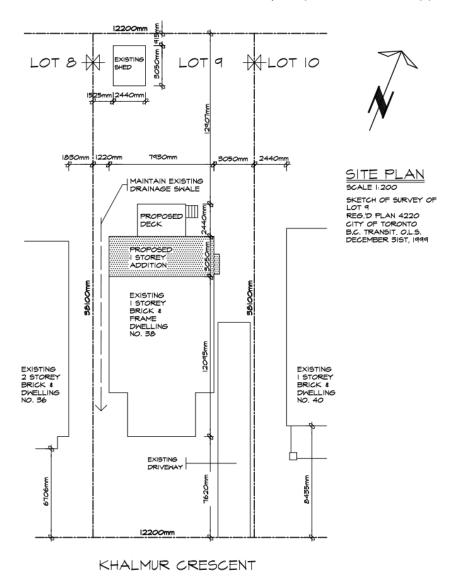
Where a property has been issued a site plan waiver, and the footprint of the building is increasing in size, a lot grading is required.

Proposed grading plans and lot grading form shall be submitted with all building permit applications, under <u>Norfolk County Grading and Drainage By-law.</u>

Proposed grading plans needs to identify:

- all surface features;
- existing and proposed structures;
- changes in grade and slopes in percent between such changes; and
- include sufficient information regarding adjacent properties to confirm conformance with this By-Law with respect to drainage onto those properties.

An exemption may be considered for a lot in a rural area (complete form, fee applies)



STEP 2: Preparing your application.

A building permit application consists of many documents. The forms attached are to be completed, signed, and dated.

Who can design the building?

There are many factors to determine who can complete drawings and design documents for these types of buildings. Your design team can be made up of qualified individuals with a Building Code Identification number (BCIN), an architect, and/or Professional Engineers. If you are unsure what qualifications a designer needs for your specific project, reach out to a building inspector for more information.

Building Department staff cannot make recommendation on a specific designer or design company.

Drawings and Documents.

Drawings are to be legible and to scale. Use a ruler or computer aided drafting (CAD) software to complete your drawings. Provide enough information and detail to ensure compliance with the Ontario Building Code.

The Ontario Building Code is available online under the 'regulations under this act' tab.

Building Department staff are not permitted by law to provide design advice. It is the responsibility of the property owner or authorized agent to complete a design that meets the requirements of the Ontario Building Code (OBC) and the Building Code Act (BCA).

Building Permits – Application Checklist.

Completed Forms.

- Building Permit Application Form.
- Signed Commitment to General Review.
- Property Owner Consent Form, if application is not completed by the property owner,
- Applicable Law Checklist and supporting documents.
- □ Lot grading form (projects with a site plan waiver).

Required Documents.

- Approved Site Plan approval plot plan.
- Drawings of the building.
 - Architectural,
 - Structural,
 - o Electrical,
 - Mechanical,
 - o Plumbing.
- Building Code Matrix.
- Completed SB-10 report (energy efficiency).

Community Development Division - Building Department

Fees.	 Septic System or Sewage Works. Sewage system is under 10,000 litres/day daily design flow for the whole site. (separate application through Norfolk County) Sewage Works is over 10,000 litres/day daily design flow for the whole site. ECA to be obtained from The Ministry of Environment, Conservation, Parks. Contact: Christopher O'Connor. Phone: 1-800-668-4557, Cell: 905-515-9618 Email: Chris.O'Connor2@ontario.ca
	Building Permit fee. Plumbing fee. Occupancy fee. Civic address (where applicable). Water / Sanitary / Storm Connection Permit (where applicable). Development charges (if applicable).
Septio	Permits - Application Checklist.
Comp	leted Forms.
	Building Permit Application Form. Schedule 1: Designer Information. Schedule 2: Sewage System Installer Information.
Requi	red Documents. Septic System Permit Application Information Package / Worksheets. Percolation time ('T' time) report from a licensed testing agency.

□ Septic Permit fee.

STEP 3: Applying.

Online Portal: Visit Norfolk Permits Portal and make your application online.



Building Department

Apply for a Building Permit
Status and Fees

In Person: Visit our service counter located at 185 Robinson Street, Suite 200 Simcoe Ontario.

Our Permit Coordinators will review your application and provide in writing any item which may be missing from the application and a cost break down for the permit fees and payment options.

Step 4: Plans Review.

A Building Inspector will contact you in writing if there are building code concerns or missing information from your application.

A building permit is issued once all documentation has been received, fees are paid in full, and your plans are check for compliance with zoning by-law and the building code.

Step 5: Inspections

Once you have obtained a building permit, a building inspector needs to attend your site at several milestones in the construction process. For more information, please check the inspection section of Norfolk County's Building Department website. Once all inspections are complete and passed your permit is closed.

Need Help? If you have any question on the building permit process or plans required, please contact permits@norfolkcounty.ca or 519-426-5870 ext. 6016

Updated October 2022

BILGER BAY SHOP - SITE PLAN

NORFOLK COUNTY - COURTLAND SECURITIES AND CONSTRUCTION ESTIMATES

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REVISION
31 OCTOBER 2024 - PRELIMINARY FOR APPROVAL
DATE - COLLECTED AT REGISTRATION
DATE - HELD AFTER ACCEPTANCE

ITEM	DESCRIPTION	UNIT	QTY.	UNIT PRICE	TOTAL COST	Secu	rities
						10%	100%
	OW GROUND						
WA.	TERMAIN						
	Water						
	a) Dry Hydrant b) Conncrete Chamber for Well	L/S L/S	1 1	\$7,000 \$3,000	\$7,000 \$3,000	\$700 \$300	
	TOTAL WATERMAIN			-	\$10,000	\$1,000	\$0
STO	RM SEWERS						
	Storm Sewer						
	a) 200mm Diameter HDPE	М	129	\$120	\$15,432	\$1,543	
	b) 300mm Diameter	M	127	\$220	\$28,006	\$2,801	
	c) 375mm Diameter	М	103	\$260	\$26,780	\$2,678	
	1200mm Diameter Manholes	EA	1	\$6,000	\$6,000	\$600	
	Video Inspection and Report	L.S.	1	\$1,500	\$1,500		\$1,500
	TOTAL BELOW STORM SEWER			-	\$77,718	\$7,622	\$1,500
					\$87,718	\$8,622	\$1,500
4 D	OVE CROUND						
	OVE GROUND						
210	RM SEWERS						
	Catchbasins	EA	7	\$2,500	\$17,500	\$1,750	
	Ditch Inlet	EA	1	\$3,000	\$3,000	\$300	
	Rip Rap R50	M^2	55	\$50	\$2,750	\$275	
	TOTAL ABOVE STORM SEWER			-	\$23,250	\$2,325	
TRU	CK YARD CONSTRUCTION						
	Granular 'A'	M_3	900	\$60	\$54,000	\$5,400	
	Granular 'B'	M³	3880	\$50	\$194,000	\$19,400	
				,	, , , , , , , , , , , , , , , , , , , ,	, ,, ,,	
	TOTAL TRUCK YARD CONSTRUCTION			-	\$248,000	\$24,800	
				_			
				_	\$271,250	\$27,125	
FIN	ISHING WORKS						
	Top Soil Grading and Seeding	M^2	6250	\$3	\$18,750		\$18,750
	Top son Grading and securing						
	Driveway Apron	L/S	1	\$10,000	\$10,000		\$10,000

						10%	100%
BELOW GR							
STORM WA	ATER MANAGEMENT PO	ND					
	on and Grading of SWM Pond	L/S	1	\$30,000	\$30,000		\$30,000
(6000m³)					\$30,000		\$30,000
				_			
LANDSCA	PING AND ON SITE WOI	RKS					
1.8m High	n Chainlink Fence	Μ	318	\$75	\$23,850		\$23,850
Wall Mou	unted Exterior Lighting	EACH	9	\$500	\$4,500		\$4,500
					\$28,350		\$28,350
				_			
SUMMARY	,						
BELOW G	GROUND			_	\$87,718	\$8,622	\$1,500
ABOVE G	GROUND			-	\$271,250	\$27,125	\$0
FINISHING	G WORKS			_	\$28,750	\$0	\$28,750
STORM WATER MANAGEMENT POND \$30.					\$30,000	\$0	\$30,000
LANDSCA	APING AND ON SITE WORKS			_	\$28,350	\$0	\$28,350
TOTAL SEC	HIDITIES DECILIDED AT DI	ECISTD.	ATION	ı	-		\$124,347
IOIAL SEC	TOTAL SECURITIES REQUIRED AT REGISTRATION						۶۱ <u>۷</u> 4,34/



494 HIGHWAY 3
PROPOSED SITE IMPROVEMENTS
BILGER SLINGER SERVICES
COURTLAND, ONTARIO

FUNCTIONAL SERVICING REPORT



494 HIGHWAY 3

PROPOSED SITE IMPROVEMENTS

BILGER SLINGER SERVICES

FUNCTIONAL SERVICING REPORT

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APPENDIX 'A' - SWM CALCULATONS

Pre-Development Flows
2-Year Post Development Storage Requirements
5-Year Post Development Storage Requirements
100-Year Post Development Storage Requirements
Overall Site Storage Summary

<u>APPENDIX 'B' – WATER SERVICING</u>

OBC Fire Flow Calculations Ontario Well Record (910370347)

<u>APPENDIX 'C' – TRAFFIC IMPACT STUDY</u>

Traffic Impact Study (Paradigm Transportation Solutions)





Alex Muirhead, P. Eng.

Peter J. Penner, P. Eng. | Deren Lyle, P. Eng.

261 Broadway, P.O. Box 460, Tillsonburg, ON N4G 4H8 T: 519-688-1000

www.cjdleng.com

23044 30 October 2024

494 HIGHWAY 3

PROPOSED SITE IMPROVEMENTS

BILGER SLINGER SERVICES

FUNCTIONAL SERVICING REPORT

1.0 INTRODUCTION

The subject site, 494 Highway 3, near the village of Courtland, Norfolk County, is located on the south side of Highway 3 between Goshen Road and Plowman's Line. The site is currently zoned rural commercial (CR). The proposed use complies with the current zoning and therefore there is no zoning amendment required for this property.

Site Grading and Servicing Plans have been prepared by Cyril J. Demeyere Limited (CJDL) in support of the proposed Bay Shop, Storage Building and Covered Truck Parking for Bilger Slinger Services. The site is currently an equipment dealership with a paved parking lot and a partial gravel storage yard with an existing storage building. Blue West will continue to lease the front portion of the existing building operating as a separate business containing a retail show room and shop. Access to the site will be provided through two existing 8.0m± wide driveways from Highway 3 as shown on the Site Plan. The western access is currently paved and will remain unchanged as a result of this development. The eastern entrance is currently gravel and will be upgraded to a 9.0m wide heavy duty asphalt. No new entrances onto Highway 3 are proposed as a part of this development. A Traffic Impact Study was conducted by Paradigm Transportation Solutions in support of this application.

2.0 TRAFFIC IMPACT STUDY

A Traffic Impact Study was completed by Paradigm Transportation Solutions Limited is attached in Appendix 'C'. The study examined existing and future traffic conditions under a 5-yr and 10-yr horizon and provided recommendations on Site access upon completion of the expansion. The study assumed that the location of the 2 current entrances to the site would be maintained and remain stop sign controlled at the exits only. The study was completed in accordance with both the requirements detailed by the MTO TIS Guidelines and the Norfolk County TIS Guidelines. It was concluded based on the findings of the study that maintaining 2 driveway accesses onto Highway 3 be considered for approval as proposed. No improvements to Hwy No. 3 are required as a result of the proposed Site Plan. Both adjacent intersections on Hwy No. 3 will continue to operate at acceptable levels of service past the 2034 horizon.



3.0 SANITARY SERVICING

The existing building located on the site is serviced by a septic system located within the existing front yard of the site. This system is still functional and is intended to remain in use for the proposed development. The proposed buildings will not require sanitary service as they will not have washrooms or process water. There is not anticipated to be an increase in sanitary usage on site as a result of this development and therefore no septic system upgrades are proposed.

4.0 WATER SUPPLY

Currently there is an existing well on site that is used to service the existing building. There will be no process water or bathrooms within the proposed buildings therefore there is not expected to be any significant increase in water usage on site. The existing well will continue to provide service to the existing building after this development takes place. The well record is attached in appendix 'B' of this report. A pumping rate of 5.8 GPM was recommended on the well record and it is anticipated that this rate will continue to be sufficient for the proposed site.

Firefighting flows will be provided by an on-site stormwater management/fire pond. There is no municipal pressurized water source available at this property, therefore a dry hydrant and fire pond will be utilized to satisfy the fire protection requirements of the property. Firefighting flow requirements were determined in accordance to the Ontario Building Code. The volume of water required for each of the buildings on the property was calculated, with the highest required value being the value required to be stored within the fire pond. In accordance with the OBC, the bottom 0.60m of the pond was not used in this calculation to allow for sediment to accumulate at the bottom of the pond. The top 0.6m was also taken out of the volume to account for the potential ice that could form on the pond in the winter months.

As this pond will act as both a fire protection pond and stormwater management pond, it should be noted that the volumes were calculated from the permanent water level and therefore there is a potential for more fire storage within the pond during large storm events. It should be noted that the permanent water level will be located in native silty clay and therefore a clay liner will not be required for the pond to maintain the required water levels. The inlet for the dry hydrant will be located 0.45m above the bottom of the pond to minimize the amount of sediment drawn though the hydrant during firefighting operations. The dry hydrant will be located adjacent to the gravel parking lot with a clear path to all of the buildings on site. The dry hydrant will be located approximately 0.60m above the ground and approximately 3.1m above the permanent water level. This should allow for effective drafting operations to take place should it be required. Table 1 below summarizes the required volume of water for firefighting for each building on site as well as the amount provided within the pond.

Table 1: Required Fire Flow Volumes

Required Volume	Prop. Bay Shop	Prop. Covered Truck Parking	Prop. Storage Building	Existing Building	Provided Fire Pond Storage
Volume (L)	191,266	181,546	27,516	79,589	223,920

As is shown above, the fire pond will contain 223,920 L of fire storage to provided which is above the required 191,266 L of fire storage to comply with the OBC. Detailed fire flow calculations are included in Appendix B of this report.



5.0 STORMWATER MANAGEMENT

5.1 Rainfall information

IDF curves for this site were determined using the Norfolk County Design Criteria (2019). The equation and parameters can be found below:

Intensity (i) = $a/(t+b)^c$

Where: I = rainfall intensity (mm/h)

a,b,c = rainfall parameters listed below

t = inlet time based off time of concentration (hours)

The inlet time (t) was calculated using the Airport Formula for areas with a C value less than 0.4 and the Bransby Williams Method for areas with a C value greater than 0.4 as recommended by the MECP.

Table 2: Rainfall Parameters (Norfolk County Design Criteria (2019))

Parameters	2 Year	5 Year	100 Year
а	529.711	583.017	801.041
b	4.501	3.007	1.501
С	0.745	0.703	0.657

5.2 Pre-Development Conditions

The entire property is currently tributary to the Ronson Drain located at the southeast corner of the property. The existing topography is generally flat with the majority of the property draining to the southeast of the property. The site generally flows overland to this drain with 2 catchbasins that outlet to the open ditch potion of the drain located along the west side of Plowmans Line. The municipal drain at the outlet consists of an open ditch with a 900mm diameter culvert directly downstream that crosses under the existing railroad tracks. Norfolk County typically requires all sites to control post development outflow back to predevelopment levels. For this property, the drain was not sized to accommodate the current pre development flow rates and therefore post development flows will be restricted back to the capacity of the drain estimated to be 1"/24hrs. This equates to an outlet rate of approximately 9.2 l/s. Detailed pre-development calculations are included in Appendix A of this report.

A test pit was conducted to determine the soil type in the location of the proposed pond. It was concluded that there is approximately 0.5m of silty sand located below the topsoil. Below that the material appeared to be all silty clay in nature and will therefore be suitable to line the pond without the need to import a clay liner. This matches the material shown in the well log completed on site in 1961.



5.3 Post-Development Conditions

In post development conditions, SWM quantity controls will be implemented to control post-development storm run-off from the site back to the capacity of the drain for the 2 to 100-Year storms.

The following will be used for all post development runoff calculations:

Building C = 0.95
 Gravel C = 0.70
 Grass C = 0.15
 Asph./Conc. C = 0.95
 Pond C = 0.95

- Q, Rational Method Q = 0.0028*C*I*A- Intensity: I = $a/(t+b)^c$

- a,b,c Factors Norfolk County Design Criteria (2019)

- Tc, Airport Method $T_c = \frac{3.26*(1.1-C)*L^{0.5}}{\frac{S_w^{0.33}}{0.057*L}}$ - Tc, Bransby Williams $T_c = \frac{0.057*L}{\frac{S_w^{0.2}*A^{0.1}}{0.057*L}}$

The site is generally graded to contain all of the surface runoff within the site before outletting at the southeast corner of the site to the Ronson Municipal Drain through a proposed SWM pond. A small portion of the western edge of the site currently flows overland towards an existing roadside ditch along Highway 3. This area will not be altered as part of this development and therefore the flow will remain tributary to this existing ditch.

All controlled flow on site will flow through a proposed wet pond to be located in the southeast corner of this site. This wet pond will contain a 1.5m deep permanent pool which will also contain enough storage for firefighting purposes. The pond been designed following the Norfolk County Design Criteria (2019) & MOE 2003 SWM Manual for wet ponds. The surface of the permanent pool will be located at an elevation of 237.70m which is located within the clay soil layer. A test pit was conducted on September 12, 2023 at this location to confirm the soil type. Soil depths were measured on site and the soil was determined to be clay below an elevation of 238.74m (0.8m below surface). A table including the test results are included on the site plan drawings. A 150mm outlet pipe at 0.36% will restrict flows from the pond down to 9.1 l/s which is within the design capacity municipal drain's 2.8ha allocated to this site. This flow rate was calculated using the pipes capacity. It was assumed that as the water level increases in the pond, the water level in the outlet ditch will rise at a similar rate. An increase in the hydraulic grade line is not anticipated as the pond fills up. For this reason it is assumed that the slope and diameter of the pipe are the only limiting factors in the outflow rate of the pond. This outlet will be through a 600 x 600 ditch inlet complete with a 2:1 sloping grate. Detailed calculation relating to outflow rates is included in Appendix A of this report.

The wet pond will pond to an elevation pond 238.68m during the 100-year flood which will allow for 0.32m of freeboard to the top of pond elevation of 239.00m. This correlates to an active storage depth of 0.48m. Table 3 below illustrates the total volume stored during each storm event as well as the corresponding elevation. Detailed post development calculations and storage tables are included in Appendix A of this report.



Table 3: Overall Site Outflow and Storage Summary

Ponding	Total Storage	Storage I (m³)	Storage I (m³)		Peak Discharge			
Elev. (m)	Total Active Storage Provided (m³) Total Active Storage	Total Active Required	Controlled (I/s)	Notes				
237.20	N/A	N/A	N/A	N/A	N/A	N/A	Outlet	
238.02	1728	497	490	9.1	81	9.2	2-YEAR	
238.20	2050	818	811	9.1	107	9.2	5-YEAR	
239.12	4193	2960	2943	9.1	180	9.2	100-YEAR	

An overflow swale will be provided to direct emergency overflow from the pond overland to the Ronson Municipal Drain ditch. This swale will ensure the surrounding properties do not experience flooding should the pond outlet fail. This swale will also be used to drain the undeveloped portion of the southeast of the property to the municipal drain via overland flow. This area is currently relatively flat manicured lawn and there will be no change to this area as a result of this development.

6.0 EROSION AND SEDIMENTATION CONTROL/CONSTRUCTION PRACTICES

Prior to topsoil stripping and bulk grading being completed, silt fence will be placed at drainage outlets around the perimeter of the site adjacent to existing developments. As servicing progresses, silt fence (with straw bales, if required) will be reinstated.

Catch basins will contain 300mm minimum deep sumps which will collect sediment.

In addition to the silt fence, the Contractor will place geotextile under all catch basin and manhole castings to prevent the flow of construction silt into the storm sewers and to the receiving water courses.

All silt will be removed as accumulated and/or as directed by the Engineer on site. Catch basins will be cleaned by the Contractor during construction to remove any silt which may accumulate.

All finished earth surfaces will be topsoiled and sodded. The Contractor will be required to return within the guaranteed maintenance period to remedy any areas of erosion which develop.



7.0 ELECTRICAL AND UTILITIES

It is anticipated that the existing hydro service will be sufficient to feed the new building as a single metered service. The current Enbridge Gas service provided to the property will also be maintained as a part of this development. Bell Canada and Execulink have existing services available to this property for communication and internet. Contact with Utility companies will continue throughout the Site Plan Application Process.

** ** **

If there are any questions, please do not hesitate to contact this office.

All of which is respectfully submitted,

Zachary Rachar, EIT

Zechoof Rech

Peter Penner, P. Eng.

ZDR/kc



APPENDIX 'A' – SWM CALCULATIONS

Pre-Development Flows

2-Year Post Development Storage Requirements

5-Year Post Development Storage Requirements

100-Year Post Development Storage Requirements

Overall Site Storage Summary



23044

30-Oct-24

PROJECT:

DATE:

Bilger Slinger Services

Proposed Bilger Warehouse - Functional Servicing Report

Norfolk County - Courtland DESIGN BY: ZDR

PRE-DEVELOPMENT FLOW

RUNOFF COEF.

Catchment	Grass	Building Roof	Conc. Asph	Open Space (Gravel)	Total
C=	0.15	0.95	0.95	0.7	0.36
Area	1.84	0.13	0.17	0.63	2.77

IDF PARAMETERS (NORFOLK COUNTY DESIGN CRITERIA, 2019)

Parameters	2-Year	5-Year	100-Year
а	529.711	583.017	801.041
b	4.501	3.007	1.501
С	0.745	0.703	0.657

I= rainfall intensity (mm/hr), a,b= paramenters, t= time of concentration (hours)

TIME OF CONCENTRATION (MINUTES)

Catchment	Method	Flow Length (L)	Slope (S _w)	Area (ha)	T_c (min)
Pre Dev	AIRPORT (C<0.4)	322	0.90	2.77	44.72

RAINFALL INTENSITY (mm/h)

Catchment	tchment 2-Year		100-Year	
Pre Dev	29.07	38.51	64.55	

I = a/(t+b)^c: I= rainfall intensity (mm/hr), a,b,c= paramenters, t= time of concentration (hours)

DISCHARGE (L/s)

(Q = 2.778 * C * I * A)

Catchment	2-Year	5-Year	100-Year	
Pre Dev	80.9	107.2	179.6	

^{* 2} to 10 year storm: Q X 1.0, * 25 year storm: Q X 1.1, * 50 year storm: Q x 1.2, 100 year storm: Q x 1.25

MUNICIPAL DRAIN OUTLET DESIGN RESTRICTION (L/s):

2.8ha of 171Ha

1.637% of culvert capacity of 563.3l/s (900mm steel culvert @ 0.33%)

0.01637 * 563.3 =

9.2 l/s

No.	REVISION	ВҮ	DATE



23044

30-Oct-24

Bilger Slinger Services PROJECT: DATE:

Proposed Bilger Warehouse - Functional Servicing Report

Norfolk County - Courtland DESIGN BY: ZDR

POST-DEVELOPMENT STORAGE VOLUME CALCULATIONS - 2-Year Storm **RUNOFF COEF.**

Catchment	Grass	Conc / Asph	Open Space (Gravel)	Building Roof	Pond	Total Area	Runoff Coefficient
C=	0.15	0.95	0.7	0.95	0.95		
Area	0.65	0.17	1.39	0.34	0.22	2.77	0.64

IDF PARAMETERS (NORFOLK COUNTY DESIGN CRITERIA, 2019)

Parameters	2-Year
а	529.711
b	4.501
С	0.745

I= rainfall intensity (mm/hr), a,b,c= paramenters, t= time of concentration (hours)

Rate of Release 9.2 L/s $0.0092 \text{ m}^3/\text{s}$

_					<u>'</u>	0.0032	
	Volume Stored [m^3]	Volume Released [m^3]	Max. Rate of Release [m^3/s]	Inflow [m^3/s]	Total Volume [m^3]	Rainfall Intensity [mm/hr]	Time [hrs]
	142	3	0.0092	0.486	145	99.1	0.083
	207	6	0.0092	0.354	213	72.2	0.167
	276	11	0.0092	0.240	287	48.9	0.333
	317	17	0.0092	0.186	334	37.9	0.50
	346	22	0.0092	0.153	369	31.3	0.667
	368	28	0.0092	0.132	396	26.9	0.833
	386	33	0.0092	0.116	419	23.8	1
	423	50	0.0092	0.088	473	17.9	1.5
	447	66	0.0092	0.071	514	14.6	2
	475	99	0.0092	0.053	575	10.9	3
←Max Storage	489	132	0.0092	0.043	621	8.8	4
	482	265	0.0092	0.026	747	5.3	8
	432	397	0.0092	0.019	830	3.9	12
	325	596	0.0092	0.014	922	2.9	18
	198	795	0.0092	0.011	993	2.3	24

No.	REVISION		BY	DATE



23044

30-Oct-24

PROJECT:

DATE:

Bilger Slinger Services

Proposed Bilger Warehouse - Functional Servicing Report

Norfolk County - Courtland **DESIGN BY:** ZDR

POST-DEVELOPMENT STORAGE VOLUME CALCULATIONS - 5 Year Storm

RUNOFF COEF.

Catchment	Grass	Conc / Asph	Open Space (Gravel)	Building Roof	Pond	Total Area	Runoff Coefficient
C=	0.15	0.95	0.7	0.95	0.95		
Area	0.65	0.17	1.39	0.34	0.22	2.77	0.64

IDF PARAMETERS (NORFOLK COUNTY DESIGN CRITERIA, 2019)

Parameters	5 Year
а	583.017
b	3.007
С	0.703

I= rainfall intensity (mm/hr), a,b,c= paramenters, t= time of concentration (hours)

Rate of Release

9.2 L/s 0.0092 m³/s

0.0032 15							
	Volume Stored [m^3]	Volume Released [m^3]	Max. Rate of Release [m^3/s]	Inflow [m^3/s]	Total Volume [m^3]	Rainfall Intensity [mm/hr]	Time [hrs]
5	195	3	0.0092	0.663	198	135.3	0.083
7	277	6	0.0092	0.470	283	95.9	0.167
7	367	11	0.0092	0.315	378	64.4	0.333
4	424	17	0.0092	0.245	440	49.9	0.50
5	465	22	0.0092	0.203	487	41.4	0.667
8	498	28	0.0092	0.175	526	35.8	0.833
6	526	33	0.0092	0.155	559	31.7	1
8	588	50	0.0092	0.118	637	24.1	1.5
2	632	66	0.0092	0.097	698	19.8	2
3	693	99	0.0092	0.073	792	15.0	3
3	733	132	0.0092	0.060	865	12.3	4
3	803	265	0.0092	0.037	1068	7.6	8
9	809	397	0.0092	0.028	1206	5.7	12
6	766	596	0.0092	0.021	1362	4.3	18
9	689	795	0.0092	0.017	1484	3.5	24

age

No.	REVISION	BY	DATE



23044

Bilger Slinger Services PROJECT:

Proposed Bilger Warehouse - Functional Servicing Report DATE: 30-Oct-24

Norfolk County - Courtland DESIGN BY: ZDR

POST-DEVELOPMENT STORAGE VOLUME CALCULATIONS - 100 Year Storm RUNOFF COEF.

Catchment	Grass	Conc / Asph	Open Space (Gravel)	Building Roof	Pond	Total Area	Runoff Coefficient
C=	0.15	0.95	0.7	0.95	0.95		*1.25
Area	0.65	0.17	1.39	0.34	0.22	2.77	0.80

IDF PARAMETERS (NORFOLK COUNTY DESIGN CRITERIA, 2019)

Parameters	100 Year
а	801.041
b	1.501
С	0.657

I= rainfall intensity (mm/hr), a,b,c= paramenters, t= time of concentration (hours)

Rate of Release 9.2 L/s 0.0092 m³/s

		-				
Time	Rainfall	Total	ludla	Max. Rate of	Volume	Volume
Time	Intensity	Volume	Inflow	Release	Released	Stored
[hrs]	[mm/hr]	[m^3]	[m^3/s]	[m^3/s]	[m^3]	[m^3]
0.083	234.6	429	1.437	0.0092	3	427
0.167	160.8	592	0.985	0.0092	6	587
0.333	106.8	784	0.654	0.0092	11	773
0.50	83.0	915	0.509	0.0092	17	899
0.667	69.3	1019	0.424	0.0092	22	996
0.833	60.1	1104	0.368	0.0092	28	1077
1	53.5	1180	0.328	0.0092	33	1147
1.5	41.2	1363	0.252	0.0092	50	1313
2	34.2	1508	0.210	0.0092	66	1442
3	26.3	1738	0.161	0.0092	99	1639
4	21.8	1921	0.133	0.0092	132	1789
8	13.8	2442	0.085	0.0092	265	2177
12	10.6	2808	0.065	0.0092	397	2410
18	8.1	3228	0.050	0.0092	596	2632
46	4.4	4456	0.027	0.0092	1524	2933

←Max Storage

No.	REVISION	ВҮ	DATE



Bilger Slinger Services PROJECT: 23044

Proposed Bilger Warehouse - Functional Servicing Report DATE: 30-Oct-24

Norfolk County - Courtland DESIGN BY: ZDR

OVERALL SITE STORAGE TABLE

Ponding	Total Storage Pond Volume	rtal Active Storage vided (m³)	Active rage ed (m³)		Peak Disch	arge	- Notes
Elev. (m)	(m³)	Total Ac Storag Provided	Total Ac Storag Required	Controlled (l/s)	Pre Dev. (l/s)	Outlet Culvert Capacity (I/s)	
237.70	N/A	N/A	N/A	N/A	N/A	N/A	Outlet
238.02	1728	497	489	9.2	81	9.2	2-YEAR
238.20	2050	818	809	9.2	107	9.2	5-YEAR
239.12	4193	2960	2933	9.2	180	9.2	100-YEAR

STAGE STORAGE PONDING

Fire/SWM Pond

PONDING ELEV. (m)	TOTAL VOLUME (m³)	AVTIVE STORAGE VOLUME (m³)	DЕРТН (m³)
237.70	1232	0	0.00
237.75	1303	72	0.05
237.80	1377	145	0.10
237.85	1453	221	0.15
237.90	1531	300	0.20
237.95	1612	380	0.25
238.00	1695	463	0.30
238.05	1780	548	0.35
238.10	1867	636	0.40
238.15	1957	726	0.45
238.20	2050	818	0.50
238.25	2144	912	0.55
238.30	2241	1009	0.60
238.35	2341	1109	0.65
238.40	2442	1211	0.70
238.45	2547	1315	0.75
238.50	2654	1422	0.80
238.55	2763	1531	0.85
238.60	2875	1643	0.90
238.65	2989	1758	0.95

238.70	3106	1875	1.00
238.75	3226	1994	1.05
238.80	3348	2116	1.10
238.85	3473	2241	1.15
238.90	3600	2368	1.20
238.95	3730	2498	1.25
239.00	3863	2631	1.30
239.05	3998	2767	1.35
239.10	4137	2905	1.40
239.15	4277	3046	1.45
239.20	4421	3189	1.50

Release Rate of Outlet Pipe (Capacity)

$$Q = VA \qquad V = \frac{k}{n} \left(\frac{A}{P}\right)^{2/3} S^{1/2}$$

r = 0.200/2 = 0.1 m

k= 1

n= 0.013

A= Pi(r)^2

 $= 0.03142 \text{ m}^2$

P = 2Pi(r) = 0.62832 m

S= 0.0008

Q= 9.2 l/s

NO.	REVISION	BY	DATE

APPENIDX 'B' – WATER SERVICING

OBC Fire Flow Calculations
Ontario Water Well Record (910370347)



DATE: 30-Oct-24
DESIGN BY: GN/ZDR
CHECKED BY: PJP

Ontario Building Code Fire Demand Calculations

Average Floor Area 1161.3 No.Storeys Building Height 6.10 (Height to Underside of Roof Deck) (OBC Section 3) Sprinkler System? No Building Class F2 Fire Resistance/Separation 1.0 Construction Type Building is of noncombustible construction or of heavy timber construction conforming to Article 3.1.4.6. of the OBC. Floor assemblies, mezzanines, loadbearing walls, columns and arches do not have a fire-resistance rating. 27 **Building Volume** 7083.93 (Average Floor Area * Building Height) (Distance = 100.00 m) (OFM TG-03-1999 Section 6.3) 0.0 SERONT 0.0 (Distance = 77.00 m) (OFM TG-03-1999 Section 6.3) 0.0 (Distance = 30.00 m) (OFM TG-03-1999 Section 6.3) $\mathbf{S}_{\mathsf{RIGHT}}$ m) (OFM TG-03-1999 Section 6.3) 0.0 (Distance = 46.00 \mathbf{S}_{SUM} 0.0 $(S_{TOT} = 1 + S_{SUM})$ (OFM TG-03-1999 Section 6.3) 1.0 S_{TOT} Q **191266** L (Q = K*Volume*S_{TOT}) (OFM TG-03-1999 Section 6.3) (Table 2 - OFM TG-03-1999 Section 6.3) REQUIRED FIRE FLOW 6300 L/min REQUIRED FIRE FLOW L/s

02	Updated Building Size	PJP	1-Feb-24
01	Updated Per Municipal Comments	PJP	9-Nov-23
No.	REVISION	BY	DATE



DATE: 30-Oct-24
DESIGN BY: GN/ZDR
CHECKED BY: PJP

Ontario Building Code Fire Demand Calculations

Average Floor Area	1161.3	m²	
No.Storeys	1		
Building Height	5.79	m	(Height to Underside of Roof Deck) (OBC Section 3)
Sprinkler System?	No		
Building Class	F2		
Fire Resistance/Separation	1.0	h	
	_		onstruction or of heavy timber construction conforming to Article 3.1.4.6. of the OBC. Floor aring walls, columns and arches do not have a fire-resistance rating.
κ[27		
Building Volume	6723.93	m ³	(Average Floor Area * Building Height)
S _{FRONT}	0.0		(Distance = 46.00 m) (OFM TG-03-1999 Section 6.3)
S _{LEFT}	0.0		(Distance = 40.00 m) (OFM TG-03-1999 Section 6.3)
S _{RIGHT}	0.0		(Distance = 100.00 m) (OFM TG-03-1999 Section 6.3)
S _{REAR}	0.0		(Distance = 100.00 m) (OFM TG-03-1999 Section 6.3)
S _{SUM}	0.0		
S _{TOT}	1.0		(S _{TOT} = 1 + S _{SUM}) (OFM TG-03-1999 Section 6.3)
Q	181546.0	ι	(Q = K*Volume*S _{TOT}) (OFM TG-03-1999 Section 6.3)
REQUIRED FIRE FLOW	5400	L/min	(Table 2 - OFM TG-03-1999 Section 6.3)
REQUIRED FIRE FLOW	90	L/s	

02	Updated Building Size	PJP	1-Feb-24
01	Updated Per Municipal Comments	PJP	9-Nov-23
No.	REVISION	BY	DATE



DATE: 30-Oct-24
DESIGN BY: GN/ZDR
CHECKED BY: PJP

Ontario Building Code Fire Demand Calculations

Average Floor Area	223.0	m²	
No.Storeys	1		
Building Height	4.57	m	(Height to Underside of Roof Deck) (OBC Section 3)
Sprinkler System?	No		
Building Class	F2		
Fire Resistance/Separation	1.0	h	
	-		onstruction with fire separations and fire-resistance ratings provided in accordance with cluding loadbearing walls, columns and arches.
κ[27		
Building Volume	1019.11	m ³	(Average Floor Area * Building Height)
S _{FRONT}	0.0		(Distance = 15.37 m) (OFM TG-03-1999 Section 6.3)
S _{left}	0.0		(Distance = 100.00 m) (OFM TG-03-1999 Section 6.3)
S _{RIGHT}	0.0		(Distance = 33.00 m) (OFM TG-03-1999 Section 6.3)
S _{REAR}	0.0		(Distance = 100.00 m) (OFM TG-03-1999 Section 6.3)
S _{SUM}	0.0		
S _{TOT}	1.0		$(S_{TOT} = 1 + S_{SUM})$ (OFM TG-03-1999 Section 6.3)
Q	27516.0	L	(Q = K*Volume*S _{TOT}) (OFM TG-03-1999 Section 6.3)
REQUIRED FIRE FLOW	2700	L/min	(Table 2 - OFM TG-03-1999 Section 6.3)
REQUIRED FIRE FLOW	45	L/s	

02	Updated Building Size	PJP	1-Feb-24
01	Updated Per Municipal Comments	PJP	9-Nov-23
No.	REVISION	BY	DATE



DATE: 30-Oct-24
DESIGN BY: GN/ZDR
CHECKED BY: PJP

Ontario Building Code Fire Demand Calculations

Average Floor Area 865.1 No.Storeys Building Height 4.00 (Height to Underside of Roof Deck) (OBC Section 3) Sprinkler System? No Building Class D Fire Resistance/Separation 1.0 Construction Type Building is of noncombustible construction with fire separations and fire-resistance ratings provided in accordance with Subsection 3.2.2. of the OBC, including loadbearing walls, columns and arches. 23 **Building Volume** 3460.40 (Average Floor Area * Building Height) (Distance = 26.36 m) (OFM TG-03-1999 Section 6.3) 0.0 SERONT SLEFT 0.0 (Distance = 30.00 m) (OFM TG-03-1999 Section 6.3) 0.0 (Distance = 20.60 m) (OFM TG-03-1999 Section 6.3) $\mathbf{S}_{\mathsf{RIGHT}}$ m) (OFM TG-03-1999 Section 6.3) 0.0 955.00 (Distance = \mathbf{S}_{SUM} 0.0 $(S_{TOT} = 1 + S_{SUM})$ (OFM TG-03-1999 Section 6.3) 1.0 S_{TOT} Q 79589.2 L (Q = K*Volume*S_{TOT}) (OFM TG-03-1999 Section 6.3) (Table 2 - OFM TG-03-1999 Section 6.3) REQUIRED FIRE FLOW 2700 L/min REQUIRED FIRE FLOW L/s

02	Updated Building Size	PJP	1-Feb-24
01	Updated Per Municipal Comments	PJP	9-Nov-23
No.	REVISION	BY	DATE

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<u>APPENIDX 'C' – TRAFFIC IMPACT STUDY</u>

Traffic Impact Study (Paradigm Transportation Solutions, 2024)



494 Highway 3, Courtland, Norfolk County Transportation Impact Study

Paradigm Transportation Solutions Limited

2024-10 230661





Project Summary



Project Number:

230661

Date and Version:

2024**-**10 1.0.0

Client:

Reid & Deleye

Greg Eyre

Peter Penner Cyril J. Demeyere Limited Consulting Engineers 261 Broadway, P.O. Box 460 Tillsonburg ON N4G 4H8

Consultant Project Team

Rajan Philips, M.Sc. (PI), P.Eng. Patrick Neal, P.Eng.

Paradigm Transportation Solutions Limited

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494 Highway 3, Courtland, Norfolk County Transportation Impact Study

<< Original Signed By >>

Rajan Philips, P.Eng.

Disclaimer

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Executive Summary

Content

Paradigm Transportation Solutions Limited (Paradigm) has been retained to conduct this Transportation Impact Study (TIS) for the proposed changes to the property located at 494 Highway 3 in the community of Courtland, Norfolk County. The changes will provide for expanding an existing Equipment Warehouse and accommodate a new Truck Yard for two different businesses.

This TIS includes description and analysis of existing road and traffic conditions, a description of the proposed development, analysis of future traffic conditions, and assessment of the existing access arrangement to accommodate the proposed changes.

Proposed Site Changes

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

TIS Scope

The scope of the Transportation Impact Study for the proposed development includes:

Study Area Intersections:



- Highway 3 and Goshen Road; and
- Two site access intersections on Highway 3.
- Analysis Periods: Weekday AM and PM peak hours.
- ▶ **Traffic Conditions:** Existing (2023), expansion completion (2024), five years after completion (2029), and ten years after completion (2034).

Conclusions

Based on the investigations carried out, it is concluded that:

- Existing Traffic Conditions: The intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are operating at satisfactory levels of service (LOS A/B).
- ▶ **Site Traffic Volumes:** The existing site driveway volumes indicate that 5 AM peak hour trips and 4 PM peak hour generated by the current site operations.

With the proposed changes, the site traffic volumes are projected to increase to 7 AM peak hour trips and 15 PM peak hour trips.

Based on the current and future site traffic volumes, the site could be considered to be a low trip generator with minimal impact on driveway and road traffic operations.

- ▶ **Background Traffic Conditions:** The two intersections are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 background traffic conditions.
- ▶ Total Traffic Conditions: The intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours under 2024, 2029, and 2034 total traffic conditions, as under existing conditions.
- ▶ **Site Accesses:** The Westerly and Easterly Site Access intersections on Highway 3 are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 total traffic conditions.

Both access locations satisfy the sight distance requirements, and do not require an auxiliary westbound left-turn lane on Highway 3 to accommodate inbound left-turning traffic.

Recommendations

Based on the findings and conclusions of this study, it is recommended that the Site Changes with two driveways for the property at 494 Highway 3, Courtland be considered for approval as proposed.

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Table 2.3:	Existing Traffic Operations	
Table 3.1:	Trip Generation	
Table 3.2:	Estimated Trip Distribution	
Table 4.1:	2024 Background Traffic Operations	
Table 4.2:	2024 Total Traffic Operations	
Table 4.3:	2029 Background Traffic Operations	
Table 4.4:	2029 Total Traffic Operations	
Table 4.5 :	2034 Background Traffic Operations	
Table 4.6:	2034 Total Traffic Operations	
Table 4.7:	Sight Distance Measurements	



1 Introduction

1.1 Overview

Paradigm Transportation Solutions Limited (Paradigm) has been retained to conduct this Transportation Impact Study (TIS) for the proposed changes to the property located at 494 Highway 3 in the community of Courtland, Norfolk County. The changes will provide for expanding an existing Equipment Warehouse and accommodate a new Truck Yard for two different businesses.

Figure 1.1 details the subject development location.

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

1.2 Purpose and Scope

The purpose of this report is to assess the location and operation of the existing site driveways to accommodate the proposed site changes. The scope of the study, shared with Norfolk County and Ministry staff via e-mail in November 2023, includes:

 assessment of the current road, traffic and site conditions within the study area;



- estimates of background traffic growth for the year of the proposed site changes (2024), five years after changes (2029), and ten years after changes (2034);
- estimates of additional traffic generated by the proposed site changes;
- analyses of the impact of the future traffic on the surrounding road network, including the following study area intersections:
 - Highway 3 and Goshen Road; and
 - Two access intersections on Highway 3.
- Operational and safety assessment of the two site driveways in accommodating the proposed site changes.

Appendix A contains the pre-study consultation material.

This study has been prepared in accordance with the requirements detailed by the MTO TIS Guidelines¹ and Norfolk County TIS Guidelines².

Norfolk County Integrated Sustainable Master Plan (ISMP), Appendix J: TIS Guidelines, September 2016.



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General Guidelines for the Preparation of Traffic Impact Studies, Ministry of Transportation Ontario, March 2023.





Location of Subject Site

2 Existing Conditions

2.1 Existing Roadways

The main roadways near the subject development considered in assessing the traffic impacts of the development include:

- ▶ **Highway 3** is an east-west provincial highway³ with a two-lane cross section. The posted speed limit is 80 km/h.
- ▶ **Goshen Road** is a north-south local road with a two-lane cross section. The posted speed limit is 60 km/h.

Side-street stop control is provided at the intersection of Highway 3 and Goshen Road.

Figure 2.1 illustrates the traffic control and lane configuration at the intersections of Highway 3 and Goshen Road.

2.2 Existing Driveways on Highway 3

Highway 3 in the study area is identified as a 2B Arterial Road in MTO's Access Management Classification System.⁴

2B Arterial roads typically allows consideration of access connections based on spacing, density (number of accesses over a kilometre), property frontage, and safety requirements.

In the section of Highway 3 between Goshen Road and Plowman's Line, a distance of 625 metres, there are seven driveways on the north side of the highway and six on the south.

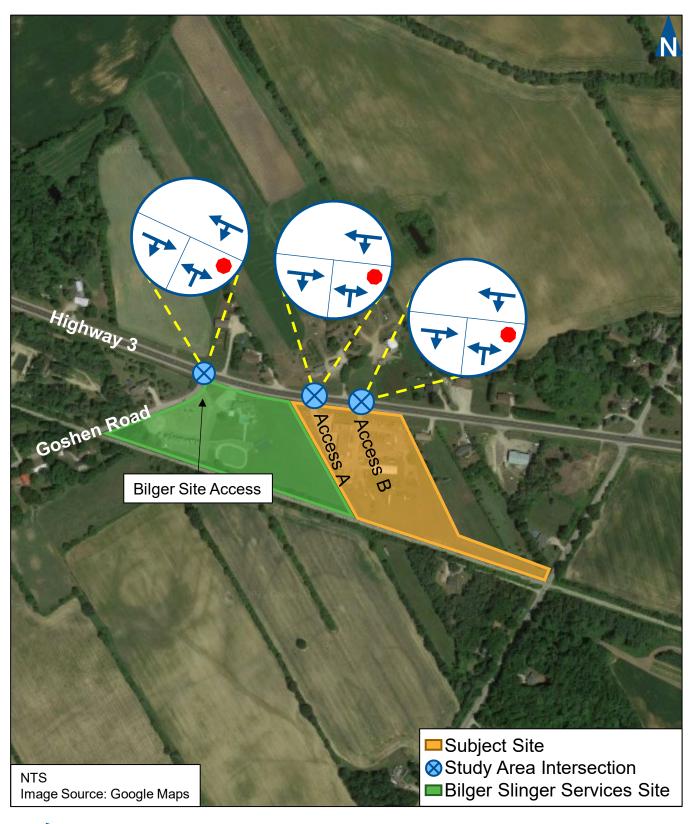
The driveways on the south side include the two driveways to the subject property. The property frontage on Highway 3 is 160 metres, and the two driveways are 70 metres apart, exceeding the minimum separation requirement of 30 metres. The westerly and easterly site driveways are located at 165 metres and 235 metres west of Goshen Road, respectively. The two driveways are under stop sign control as shown in **Figure 2.1**.

Ministry of Transportation. Highway Corridor Management Manual. April 2022.



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Norfolk County Official Plan, Schedule E-4: Delhi Transportation, 5 October 2018.





Existing Lane Configuration and Traffic Control

2.3 Traffic Volumes

Paradigm conducted turning movement counts at the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Blue West Equipment Access A on 28 November 2023. Paradigm also conducted turning movement counts at the Goshen Road and Bilger Access intersection on 28 November 2023 to determine the existing traffic in and out of the Bilger Slinger Services site.

Table 2.1 summarizes the traffic peak hours at each intersection.

 Intersection
 AM Peak Hour
 PM Peak Hour

 Highway 3 and Goshen Road
 7:45 - 8:45
 3:45 - 4:45

 Highway 3 and the Blue West Equipment Access A Goshen Road and Bilger Access
 7:45 - 8:45
 3:45 - 4:45

 Tool - 8:00
 4:00 - 5:00

TABLE 2.1: INTERSECTION PEAK HOURS

Appendix B contains the detailed traffic counts for the intersections.

Figure 2.2 illustrates the existing AM and PM weekday peak hour turning movement traffic volumes at the study area intersections.

Table 2.2 shows the 8-hour hourly breakdown of inbound and outbound traffic on Gosehn Road at the Bilger site access.

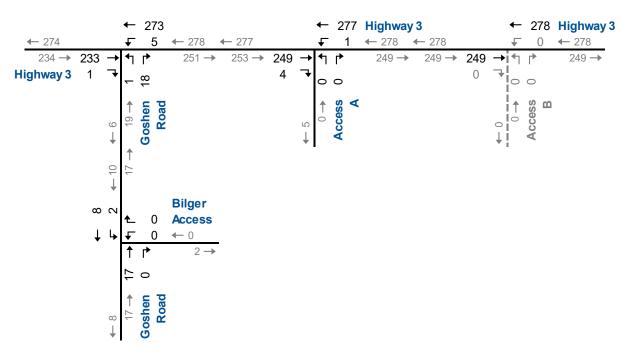
The study area roadway and driveway traffic volumes and turning movements are noted to be low to moderate:

- The peak directional traffic volume on Highway 3 is under 300 vph in the AM peak hour and under 450 vph in the PM peak hour.
- ► The traffic on Goshen Road is under 20 vph in either direction during either peak hour.
- ▶ The driveway turning movements are less than 5 vph.

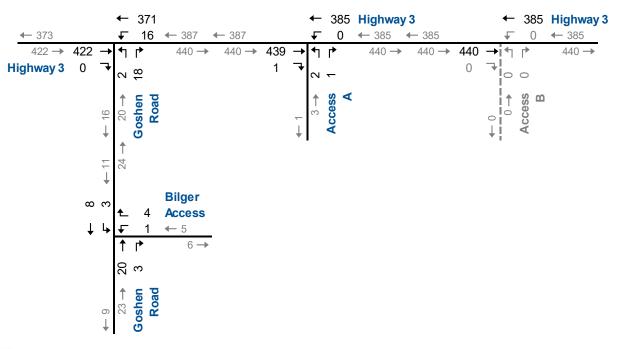
It is noted that with the proposed changes, the driveway volumes on Goshen Road for the property at 480 Highway 3 will be transferred to the subject site driveways on Highway 3 for the property at 494 Highway 3.

AM Peak Hour





PM Peak Hour





Existing Traffic Volumes

TABLE 2.2: BILGER SITE ACCESS 8-HOUR TRAFFIC VOLUMES

Time	Inbound	Outbound
7:00 – 8:00 AM	2	0
8:00 – 9:00 AM	2	1
9:00 – 10:00 AM	2	4
11:30 AM – 12:30 PM	2	2
12:30 – 1:30 PM	0	0
3:00 – 4:00 PM	4	2
4:00 – 5:00 PM	6	5
5:00 - 6:00 PM	5	4
Total	20	18

2.4 Traffic Operations

The level of service conditions at the Highway 3 and Goshen Road intersection, and at the Highway 3 and Access A intersection have been assessed through intersection operational analysis using Synchro 11.

Intersection level of service (LOS) is a recognized method of quantifying the average delay experienced by drivers at intersections. It is based on the delay experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles intending to make a particular movement, compared to the estimated capacity for that movement. The capacity is based on several criteria related to the opposing traffic flows and intersection geometry.

The highest possible rating is LOS A, under which the average total delay is equal or less than 10.0 seconds per vehicle. When the average delay exceeds 80 seconds for signalized intersections, 50 seconds for unsignalized intersections or when the volume to capacity (v/c) ratio is greater than 1.00, the movement is classed as LOS F and remedial measures are usually implemented if they are feasible. LOS E is usually used as a guideline for the determination of road improvement needs on through lanes, while LOS F may be acceptable for left-turn movements at peak times, depending on delays.

Movements are considered critical under the following conditions:

- Signalized Intersections:
 - v/c ratios for overall intersection operations, through movements or shared through/turning movements increased to 0.85 or above;
 - v/c ratios for exclusive movements increased to 0.95 or above; or



- queues for an individual movement are projected to exceed available turning lane storage.
- Unsignalized Intersections:
 - LOS based on average delay per vehicle, on individual movements exceeds LOS "E"; or
 - the estimated 95th percentile queue length for an individual movement exceeds the available queue storage.

Table 2.3 summarizes the results of the intersection operational analysis under existing conditions, including the AM and PM peak hour LOS, v/c ratios, and 95th percentile queues experienced.

The results indicate that the two intersections are operating at satisfactory levels of service (LOS A/B), and with no problem movements.

Appendix C contains the detailed Synchro 11 reports.

TABLE 2.3: EXISTING TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	h					
Period					Eastb	ound			Westl	ound		I	Northi	oounc	ı	;	South	bounc	I	
Analysis P	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
ınc	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Road		V/C Q		0.00	>		< <	0.00			0.03		>						
eal	Highway 3 & Access A		LOS		A	>	Α	<	A		Α	A		>	Α					
AM		TWSC	Delay		0	>	0	<	8		0	0		>	0					
٩	riigriway 3 & Access A	TWSC	V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
'n	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.04		>						
ak			Q		0	>	Α.	<	1			1		>	-					
			LOS		A	>	Α	<	A		A 0	В		>	В					
PM	Highway 3 & Access A	TWSC	Delay V/C		0	^	0	<	0		U	15		>	15					
	riigiway o a ricocco ri		Q Q		0.00	>		< <	0.00			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



3 Proposed Site Changes

3.1 Description of Site Changes

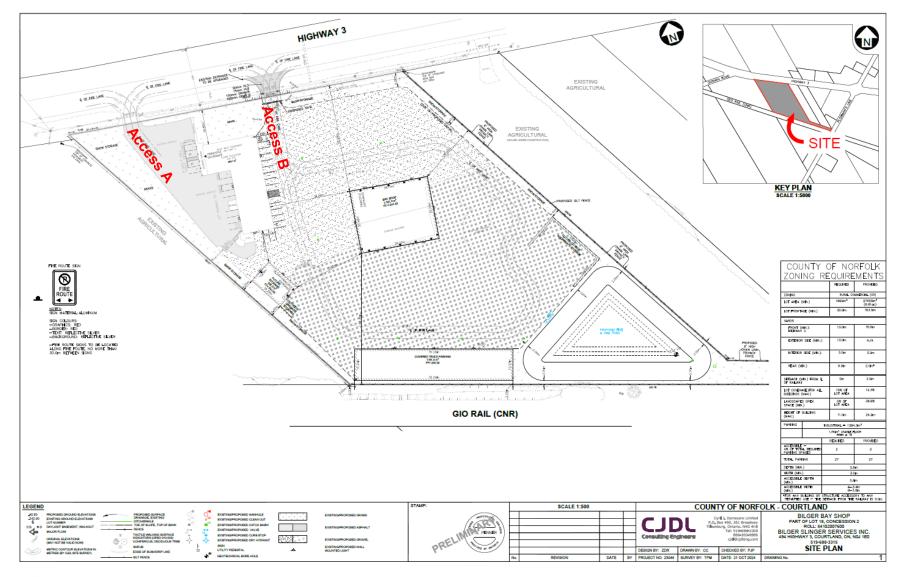
The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

Figure 3.1 includes the preliminary grading plan for the site illustrating the proposed changes including the access arrangement.





Proposed Site Changes

3.2 Site Trip Generation

The trip generation for each of the Blue West Equipment and Bilger Slinger Services sites are expected to be the same as under existing conditions.

Table 3.1 summarizes the number of trips generated by the proposed development.

TABLE 3.1: TRIP GENERATION

Site	AM	Peak H	lour	PM Peak Hour						
Site	In	Out	Total	In	Out	Total				
Blue West Equipment	5	0	5	1	3	4				
Bilger Slinger Services	2	0	2	6	5	11				
Total Trip Generation	7	0	7	7	8	15				

3.3 Development Trip Distribution and Assignment

The trip distribution for the Bilger Slinger Services site traffic was determined based on the existing assignment of traffic volumes at the site driveway and the existing travel patterns within the study area. **Table 3.2** displays the breakdown of trip distributions used in this study.

TABLE 3.2: ESTIMATED TRIP DISTRIBUTION

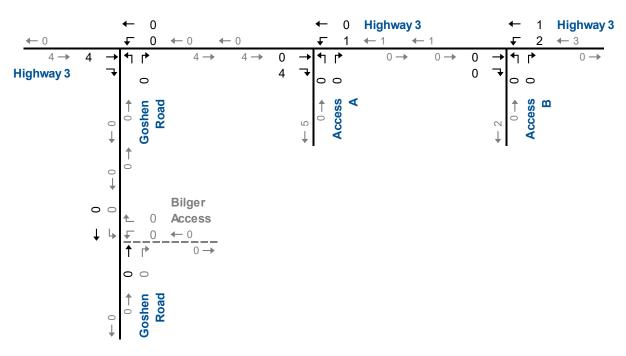
Origin/Destination	AM Pea	ak Hour	PM Peak Hour						
Origin/Destination	Inbound	Outbound	Inbound	Outbound					
East via Highway 3	83%	0%	50%	72%					
West via Highway 3	17%	0%	0%	8%					
South via Goshen Road	0%	0%	50%	20%					
Total	100%	0%	100%	100%					

Figure 3.2 illustrates the site-generated traffic volumes for the AM and PM peak hours. The site traffic for Blue West Equipment is unchanged from the existing traffic volumes.

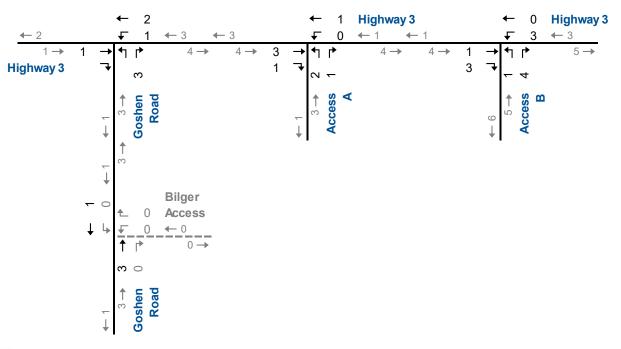
It is noted that as under existing traffic conditions the driveway volumes will remain minimal following the proposed site changes.

AM Peak Hour





PM Peak Hour





Site Generated Traffic Volumes

4 Access Assessment

The assessment of the subject driveways under future traffic conditions is based on future background and total traffic volumes, corresponding to the proposed timing of site changes (2024), five years after changes (2029), and ten years after changes (2034).

4.1 Background Traffic Forecasts

In order to derive the generalized background traffic volumes, a growth rate of 2.0% per annum was applied to the existing roadway traffic volumes.

No background developments were included in the future traffic volumes.

4.2 2024 Background Traffic Operations

Figure 4.1 illustrates the 2024 background traffic volumes, including road traffic growth.

The 2024 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

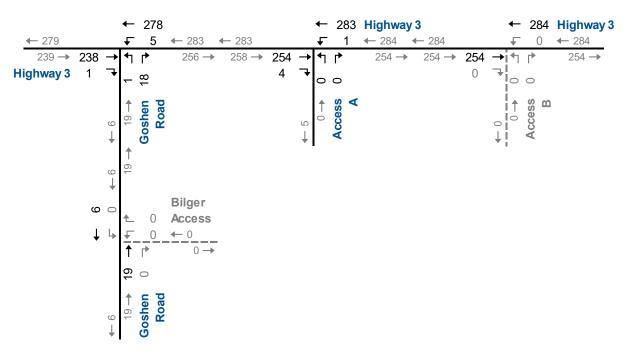
It is noted that no changes have been made to the turning movements at the intersection of Highway 3 and Goshen Road even they would likely be reduced following the proposed relocation of Bilger truck operations.

Table 4.1 summarizes the results of the 2024 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at satisfactory levels of service (LOS A/B), and with no problem movements.

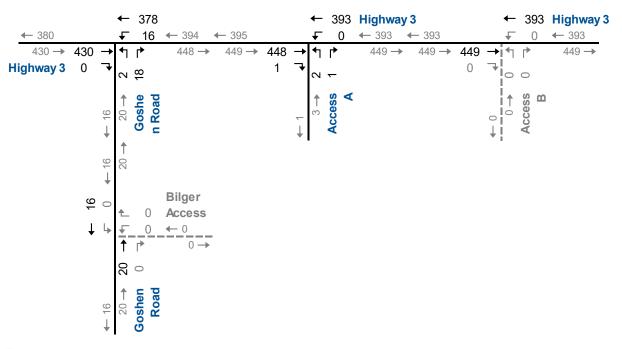
Appendix D contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2024 Background Traffic Volumes

TABLE 4.1: 2024 BACKGROUND TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	oroach	1					
erio					Eastb	ound			Westk	ound			Northl	bound	I	;	South	bounc	I	
Analysis Period	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
ın	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Road		V/C Q		0.00	>		<	0.01			0.03		>						
eak	Highway 3 & Access A		LOS		A	^	Α		A		Α	A			Α					
AM P		TA/0.0	Delay		0	>	0	<	8		0	0		>	0					
4		TWSC	V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
5	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.04		>						
ak			Q LOS		0 A	>	Α	<	1 A		Α	1 B		>	В					
			Delay		0	>	0	<	0		0	15		>	15					
PM	Highway 3 & Access A	TWSC	V/C		0.00	>			0.00		•	0.01		>	10					
			Q		0.00	>		<	0			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.3 2024 Total Traffic Operations

Figure 4.2 illustrates the 2024 total traffic volumes, including trips generated by the proposed development.

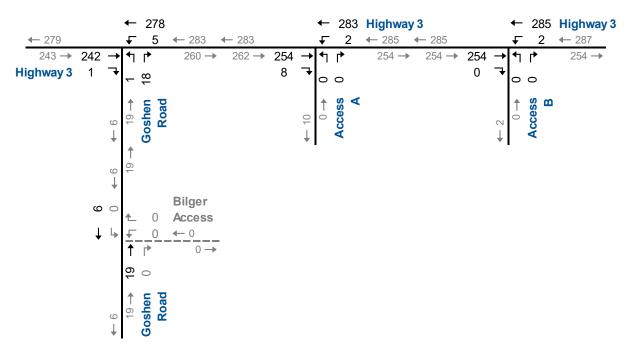
The 2024 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

Table 4.2 summarizes the results of the 2024 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

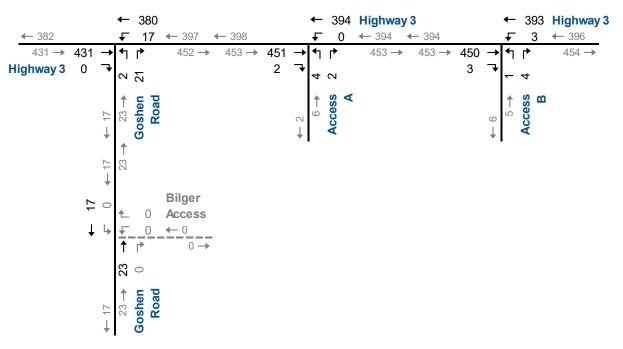
Appendix E contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2024 Total Traffic Volumes

TABLE 4.2: 2024 TOTAL TRAFFIC OPERATIONS

ъ										Directi	on/Mo	oveme	nt/App	roacl	1					
erio					Eastb	ound			Westk	ound		I	Northi	oound	I	;	South	bound	I	
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	ijeŢ	Through	Right	Approach	Пеff	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
r	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	0 >	v v v v	A 8 0.01 0		A 0	B 10 0.03 1		^ ^ ^	B 10					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^ ^	A 0	v v v	A 8 0.00		A 0	A 0 0.00 0		^ ^ ^	A 0					
A	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0		A 8 0.00		A 0	A 0 0.00		^	A 0					
_	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 9 0.02 1		A 0	B 12 0.04 1		^ ^ ^	B 12					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 15 0.02 1		^ ^ ^	C 15					
<u>a.</u>	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	A 0	v v v	A 8 0.00 0		A 0	B 12 0.01 0		^ ^ ^ ^	B 12					

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LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</> - Shared with through movement



4.4 2029 Background Traffic Operations

Figure 4.3 illustrates the 2029 background traffic volumes, including road traffic growth.

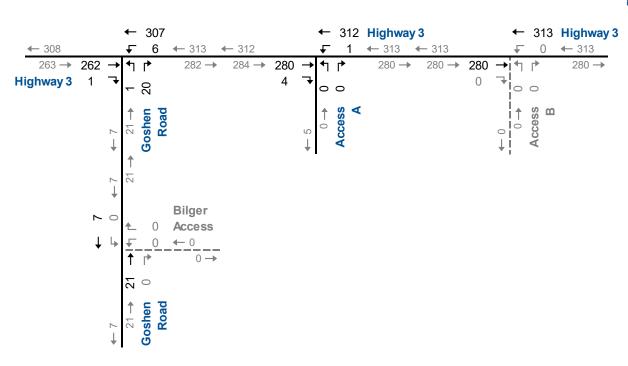
The 2029 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

Table 4.3 summarizes the results of the 2029 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at acceptable levels of service, and with no problem movements.

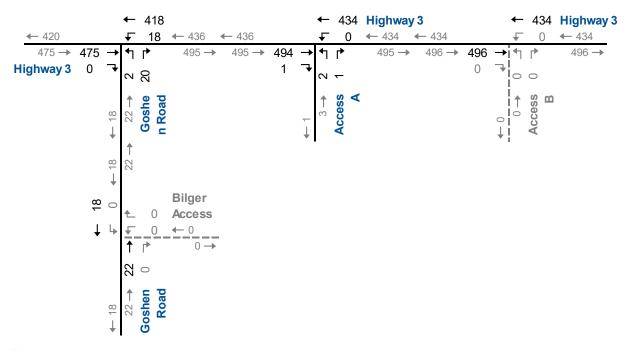
Appendix F contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2029 Background Traffic Volumes

TABLE 4.3: 2029 BACKGROUND TRAFFIC OPERATIONS

ठ										Directi	on/Mo	veme	nt/App	oroacl	1					
erio					Eastb	ound			Westk	ound			Northi	bound	I	;	South	bounc	I	
Analysis Period	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
nr	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Road		V/C Q		0.00	>		<	0.01			0.03		>						
ea	Highway 2.8 Access A	TWSC C	LOS		A	>	Α	<	A		Α	A		>	Α					
AM F			Delay		0	>	0	<	8		0	0		>	0					
⋖	Highway 3 & Access A		V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
늄	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.05		>						
ak			Q		0	>	^	<	1		^	1		>	С					
			LOS		A 0	>	A 0	<	A 0		A 0	C 16		>	16					
PM	Highway 3 & Access A	cess A TWSC	Delay V/C		0.00	>	U	<	0.00		U	0.01		>	10					
			Q		0.00	>		<	0.00			0.01		>						

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LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.5 2029 Total Traffic Operations

Figure 4.4 illustrates the 2029 total traffic volumes, including trips generated by the proposed development.

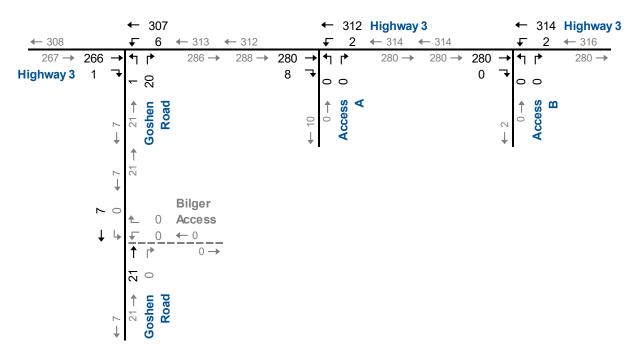
The 2029 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

Table 4.4 summarizes the results of the 2029 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

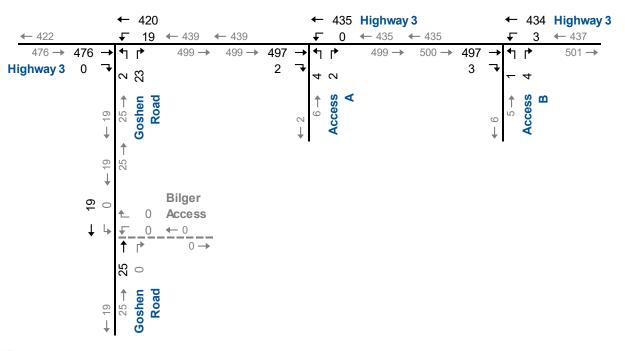
Appendix G contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2029 Total Traffic Volumes

TABLE 4.4: 2029 TOTAL TRAFFIC OPERATIONS

ъ										Directi	on/Mo	veme	nt/App	roacl	h					
erio					Eastb	ound			Westl	ound		ı	Northi	oound	i	,	South	bound	I	
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	IJeТ	Through	Right	Approach	IJeТ	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0		A 8 0.01 0		A 0	B 10 0.03 1		^ ^ ^	B 10					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	· · · · · ·	A 8 0.00		A 0	A 0 0.00		^ ^ ^	A 0					
AN	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	/	A 8 0.00		A 0	A 0 0.00		^ ^ ^	A 0					
r	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	<td>A 9 0.02 1</td> <td></td> <td>A 0</td> <td>B 12 0.05 2</td> <td></td> <td>^ ^ ^</td> <td>B 12</td> <td></td> <td></td> <td></td> <td></td> <td></td>	A 9 0.02 1		A 0	B 12 0.05 2		^ ^ ^	B 12					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 16 0.02 1		^ ^ ^	C 16					
ď	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 8 0.00 0		A 0	B 13 0.01 0		^ ^ ^	B 13					

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.6 2034 Background Traffic Operations

Figure 4.5 illustrates the 2034 background traffic volumes, including road traffic growth.

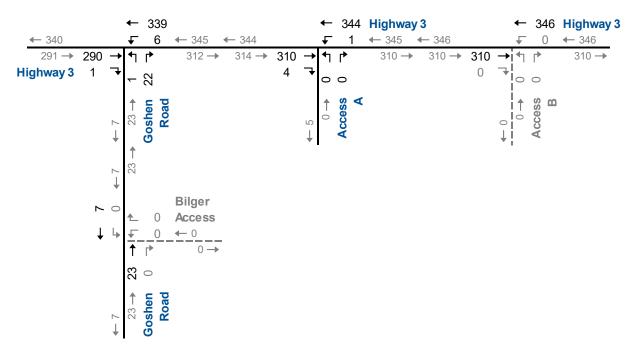
The 2034 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

Table 4.5 summarizes the results of the 2034 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at acceptable levels of service, and with no problem movements.

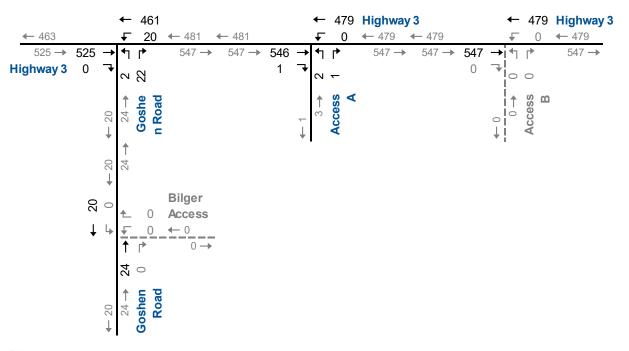
Appendix H contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2034 Background Traffic Volumes

TABLE 4.5: 2034 BACKGROUND TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	h					
Period					Eastb	ound			Westl	ound		ı	Northi	oounc	ı	;	South	bounc	i	
Analysis P	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
'n	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	11		>	11					
Peak Hour	Road		V/C Q		0.00	>		< <	0.01			0.04		>						
eal			LOS		A	>	Α	<	A		Α	A		>	Α					
AM F	Highway 3 & Access A	TWSC	Delay		0	>	0	<	8		0	0		>	0					
⋖	Highway 3 & Access A	TWSC	V/C		0.00	>		<	0.00			0.00		>						
	3 7 3 3		Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
5	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	13		>	13					
Peak Hour	Road		V/C		0.00	>		<	0.03			0.06		>						
ak			Q		0	>		<	1			2		>						
			LOS		A	>	A	<	Α		A	С		>	C					
P	Highway 3 & Access A	TWSC	Delay		0	>	0	<	0		0	18		>	18					
	Highway 3 & Access A		V/C Q		0.00	>		< <	0.00			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.7 2034 Total Traffic Operations

Figure 4.6 illustrates the 2034 total traffic volumes, including trips generated by the proposed development.

The 2034 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

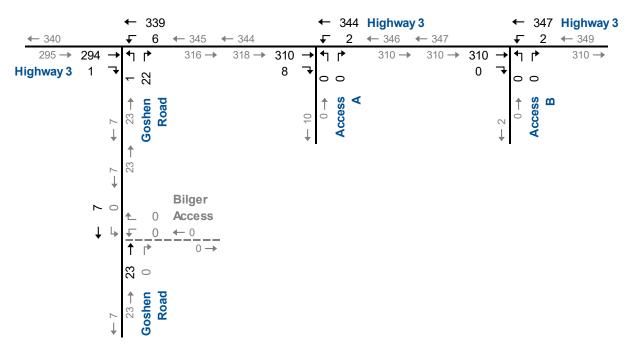
As seen in **Figure 4.6**, the study area roadway traffic volumes and turning movements are projected to remain low to moderate in 2034 as they are under existing conditions.

Table 4.6 summarizes the results of the 2034 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

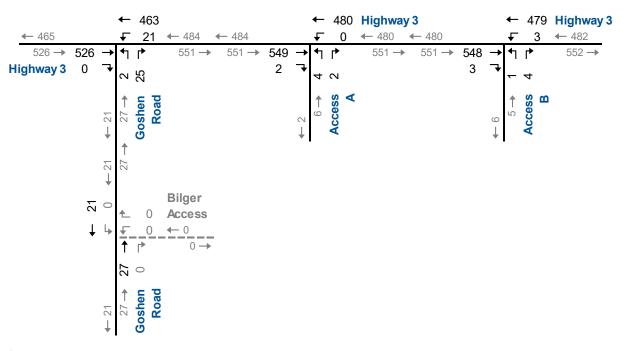
Appendix I contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2034 Total Traffic Volumes

TABLE 4.6: 2034 TOTAL TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	า					
erio					Eastb	ound			Westk	ound		ı	Northi	oound	I	;	South	bounc		
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	ijeŢ	Through	Right	Approach	Пеff	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
<u>.</u>	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^	0 >	v v v v	A 8 0.01 0		A 0	B 11 0.04 1		^ ^ ^ ^	B 11					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0		A 8 0.00		A 0	A 0 0.00 0		^ ^ ^	A 0					
A	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	v v v	A 8 0.00		A 0	A 0 0.00		^	A 0					
r	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0		A 9 0.03 1		A 0	B 13 0.06 2		^ ^ ^	B 13					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 18 0.02 1		^ ^ ^	C 18					
a	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	A 0	v v v	A 9 0.00 0		A 0	B 14 0.01 0		^ ^ ^ ^	B 14					

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</> - Shared with through movement

4.8 Review of Access Locations

The existing access locations for the subject site were reviewed for available sight distances and left-turn lane requirements. The sight distance assessment is based on field inspection and Transportation Association of Canada design standards.

4.8.1 Sight Distance Assessment

Available sight distances for the existing access points were measured during a site visit on 27 November 2023 and are compared with sight distance requirements identified in the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads⁵ (GDGCR).

Stopping and Decision Sight Distance requirements were reviewed for a design speed of 70 km/h corresponding to the posted speed of 60 km/h.

Table 4.7 summarizes the sight distance measurements and requirements at the access intersections, which indicate that the two existing driveway locations on Highway 3 satisfy the TAC requirements for Decision Sight Distances and Stopping Sight Distance.

TABLE 4.7: SIGHT DISTANCE MEASUREMENTS

	De	cision Sigh	t Distance	(m)	Stoppii	ng Sight
Intersection	Left	-Turn	Right	t-Turn	Distar	nce (m)
	Required	Measured	Required	Measured	Required	Measured
Highway 3 and Access A		250+		250+		250+
	210		185		185	
Highway 3 and		250+	, , ,	250+		250+
Access B		230+		230+		250+

4.8.2 Left-Turn Lanes

The assessment of the need for auxiliary left-turning lanes is based on the requirements and procedures detailed in the Ministry of Transportation Design Supplement for the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads⁶.

It is noted that the forecast westbound left-turn movements on Highway 3 at Site Access A and at Site Access B are less than five vph during AM and PM peak hours, which is less than 2.5% of the total

⁶ MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads, June 2017.



Transportation Association of Canada, Geometric Design Guide for Canadian Roads, June 2017.

westbound traffic volumes. The provision of left-turn lanes on Highway 3 at each of the two driveways is, therefore, identified as not warranted.

It is noted that the need for a westbound left-turn lane on Highway 3 at the intersection of Goshen Road is also identified as not warranted.

5 Conclusions and Recommendations

5.1 Conclusions

Based on the investigations carried out, it is concluded that:

- Existing Traffic Conditions: The intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are operating at satisfactory levels of service (LOS A/B).
- ▶ Site Traffic Volumes: The existing site driveway volumes indicate that 5 AM peak hour trips and 4 PM peak hour generated by the current site operations.

With the proposed changes, the site traffic volumes are projected to increase to 7 AM peak hour trips and 15 PM peak hour trips.

Based on the current and future site traffic volumes, the site could be considered to be a low trip generator with minimal impact on driveway and road traffic operations.

- ▶ **Background Traffic Conditions:** The two intersections are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 background traffic conditions.
- ▶ Total Traffic Conditions: The intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours under 2024, 2029, and 2034 total traffic conditions, as under existing conditions.
- ▶ **Site Accesses:** The Westerly and Easterly Site Access intersections on Highway 3 are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 total traffic conditions.

Both access locations satisfy the sight distance requirements, and do not require an auxiliary westbound left-turn lane on Highway 3 to accommodate inbound left-turning traffic.

5.2 Recommendations

Based on the findings and conclusions of this study, it is recommended that the Site Changes with two driveways for the property at 494 Highway 3, Courtland be considered for approval as proposed.

Appendix A

Pre-Study Consultation



From: Patrick Neal

To: "Mentley, Ryan (MTO)"; Stephen Gradish

Cc: Rajan Philips

Subject: (230661) 494 Highway 3, Courtland TIS - Pre-Study Consultation

Date: November 2, 2023 4:04:00 PM **Attachments:** (230661) Site Location.png

image001.png image002.png

23044 Preliminary Grading Plan.pdf

Hi Ryan and Stephen,

We have been retained to complete the Transportation Impact Study (TIS) in support of a Site Plan application for a Truck Yard and Warehouse expansion located at 494 Highway 3 in Courtland, Norfolk County.

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for a storage and sales facility for farm, residential and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3, and share its use with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 sq. m. with a larger warehouse of 1161 sq. m. GFA.

The site operations for Bilger Slinger Services will include:

- 18 employees.
- Storing and servicing 18 slinger trucks with a possible ten-year expansion to 24 trucks.
- Main deliveries twice a month to the site.

The site operations for Blue West Equipment will include:

- · Six to eight employees.
- 12 customers per day.
- Main deliveries twice a week to the site.

A Google Map showing the two properties and a preliminary grading plan for 494 Highway 3 are attached.

The property has two driveways on Highway 3 approximately 76 metres apart. Both driveways are proposed to be retained as part of redevelopment, and both are needed to serve the two businesses that will be located on the subject site. It is noted that the proposed site changes will not add new traffic to Highway 3, because traffic from the business at 480 Highway 3 is already included in the existing traffic volumes. The relocation will only shift a portion of the turning movements from the intersection at Goshen Road to the driveways at 494 Highway 3.

Based on the above information, we have prepared the following scope of work for

review/approval:

- Weekday AM and PM peak hours of analysis.
- Study Area Intersections:
 - Highway 3 and Goshen Road (unsignalized); and
 - two access intersections on Highway 3.
- Horizon Years (as required by MTO): (1) development completion, (2) five years after completion, and (3) ten years after completion.
- Background Growth: 2.0% compounded per annum, please confirm.
- Background Developments: Please confirm and provide corresponding site statistics or TIS.
- Roadway Improvements: Please confirm any planned changes to the study area roadways.
- Trip Generation: ITE Trip Generation Manual 11th Edition.
- Trip Distribution: Existing traffic patterns.

Please let us know if you have any questions or comments.

Regards,

Patrick Neal, EIT

Transportation Consultant



Paradigm Transportation Solutions Limited

5A-150 Pinebush Road, Cambridge ON N1R 8J8

p: 416.479.9684 x510 m: 416.688.7338 e: <u>pneal@ptsl.com</u>



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Appendix B

Existing Traffic Data





Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023

Page No: 1

Turning Movement Data

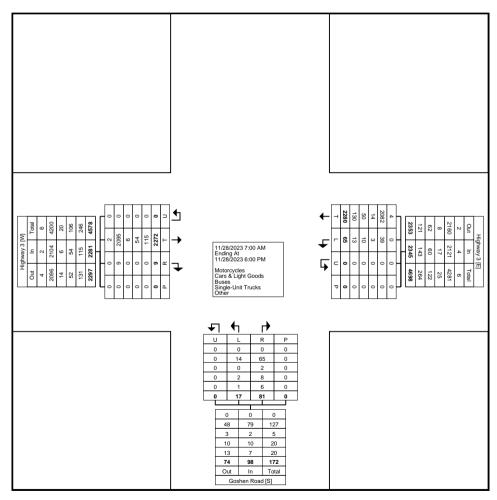
	I				ı	i dii	mig wie	VCITICITE L	Julu	1			0 1 5 1		1	1
			Highway 3					Highway 3					Goshen Road			
Start Time	Thru	Right	Eastbound U-Turn	Peds	Ann Total	1 644	Thru	Westbound U-Turn	Peds	Ann Total	Left	Right	Northbound U-Turn	Peds	App. Total	Int. Total
7:00 AM	52	2	0-14111	0	App. Total 54	Left 2	51	0-14111	0	App. Total 53	1	1	0-14111	0	Арр. Тотат 2	109
7:15 AM	66	1	0	0	67	1	56	0	0	57	0	4	0	0	4	128
7:30 AM	48	0	0	0	48	3	67	0	0	70	2	2	0	0	4	122
7:45 AM	61	0	0	0	61	<u>3</u>	71	0	0	72	0	8	0	0	8	141
Hourly Total	227	3	0	0	230	7	245	0	0	252	3	15	0	0	18	500
8:00 AM	63	1	0	0	64	1	66	0	0	67	1	1	0	0	2	133
8:15 AM	56	0	0	0	56	0	61	0	0	61	0	1	0	0	1	118
8:30 AM	53	0	0	0	53	3	75	0	0	78	0	8	0	0	8	139
8:45 AM	47	1	0	0	48	2	74	0	0	76	0	4	0	0	4	128
Hourly Total	219	2	0	0	221	6	276	0	0	282	1	14	0	0	15	518
9:00 AM	45	0	0	0	45	0	56	0	0	56	0	3	0	0	3	104
9:15 AM	46	0	0	0	46	1	64	0	0	65	0	0	0	0	0	111
9:30 AM	48	0	0	0	48	2	72	0	0	74	0	3	0	0	3	125
9:45 AM	42	2	0	0	44	3	61	0	0	64	0	1	0	0	1	109
Hourly Total	181	2	0	0	183	6	253	0	0	259	0	7	0	0	7	449
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:30 AM	66	0	0	0	66	1	66	0	0	67	0	0	0	0	0	133
11:45 AM	82	0	0	0	82	2	58	0	0	60	0	4	0	0	4	146
Hourly Total	148	0	0	0	148	3	124	0	0	127	0	4	0	0	4	279
12:00 PM	63	0	0	0	63	3	74	0	0	77	1	2	0	0	3	143
12:15 PM	89	0	0	0	89	3	64	0	0	67	1	3	0	0	4	160
12:30 PM	68	0	0	0	68	4	65	0	0	69	1	3	0	0	4	141
12:45 PM	62	0	0	0	62	3	65	0	0	68	0	0	0	0	0	130
Hourly Total	282	0	0	0	282	13	268	0	0	281	3	8	0	0	11	574
1:00 PM	78	0	0	0	78	0	74	0	0	74	2	1	0	0	3	155
1:15 PM	57	0	0	0	57	1	69	0	0	70	2	1	0	0	3	130
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	135	0	0	0	135	1	143	0	0	144	4	2	0	0	6	285
3:00 PM	99	0	0	0	99	2	70	0	0	72	0	2	0	0	2	173
3:15 PM	62	0	0	0	62	0	82	0	0	82	0	2	0	0	2	146
3:30 PM	97	0	0	0	97	1	82	0	0	83	0	0	0	0	0	180
3:45 PM	96	0	0	0	96	3	90	0	0	93	1	1	0	0	2	191
Hourly Total	354	0	0	0	354	6	324	0	0	330	1	5	0	0	6	690
4:00 PM	113	0	0	0	113	5	102	0	0	107	1	1	0	0	2	222
4:15 PM	110	0	0	0	110	4	77	0	0	81	0	5	0	0	5	196
4:30 PM	103	0	0	0	103	4	102	0	0	106	0	11	0	0	11	220

4:45 PM	86	0	0	0	86	4	88	0	0	92	1	5	0	0	6	184
Hourly Total	412	0	0	0	412	17	369	0	0	386	2	22	0	0	24	822
5:00 PM	96	0	0	0	96	1	86	0	0	87	2	0	0	0	2	185
5:15 PM	88	0	0	0	88	0	58	0	0	58	1	1	0	0	2	148
5:30 PM	76	1	0	0	77	2	75	0	0	77	0	0	0	0	0	154
5:45 PM	54	1	0	0	55	3	59	0	0	62	0	3	0	0	3	120
Hourly Total	314	2	0	0	316	6	278	0	0	284	3	4	0	0	7	607
Grand Total	2272	9	0	0	2281	65	2280	0	0	2345	17	81	0	0	98	4724
Approach %	99.6	0.4	0.0	-	-	2.8	97.2	0.0	-	-	17.3	82.7	0.0	-	-	-
Total %	48.1	0.2	0.0	-	48.3	1.4	48.3	0.0	-	49.6	0.4	1.7	0.0	-	2.1	-
Motorcycles	2	0	0	-	2	0	4	0	-	4	0	0	0	-	0	6
% Motorcycles	0.1	0.0	-	-	0.1	0.0	0.2	-	-	0.2	0.0	0.0	-	-	0.0	0.1
Cars & Light Goods	2095	9	0	-	2104	39	2082	0	-	2121	14	65	0	-	79	4304
% Cars & Light Goods	92.2	100.0	-	-	92.2	60.0	91.3	-	-	90.4	82.4	80.2	-	-	80.6	91.1
Buses	6	0	0	-	6	3	14	0	-	17	0	2	0	-	2	25
% Buses	0.3	0.0	-	-	0.3	4.6	0.6	-	-	0.7	0.0	2.5	-	-	2.0	0.5
Single-Unit Trucks	54	0	0	-	54	10	50	0	-	60	2	8	0	-	10	124
% Single-Unit Trucks	2.4	0.0	-	-	2.4	15.4	2.2	-	-	2.6	11.8	9.9	-	-	10.2	2.6
Articulated Trucks	114	0	0	-	114	13	130	0	-	143	1	6	0	-	7	264
% Articulated Trucks	5.0	0.0	-	-	5.0	20.0	5.7	-	-	6.1	5.9	7.4	-	-	7.1	5.6
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
% Pedestrians	-	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 3



Turning Movement Data Plot



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023

Page No: 4

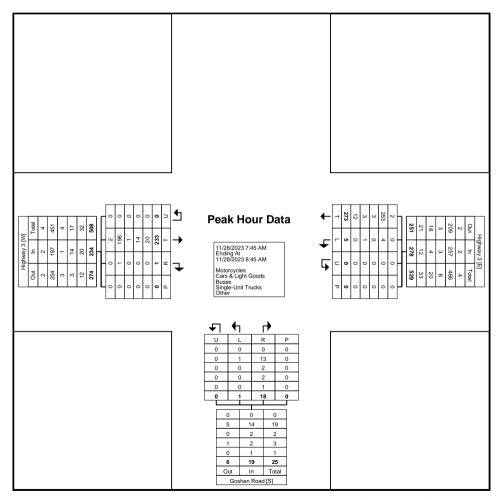
Turning Movement Peak Hour Data (7:45 AM)

						9		ait i 10 ai i	Jaia (1							
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:45 AM	61	0	0	0	61	1	71	0	0	72	0	8	0	0	8	141
8:00 AM	63	1	0	0	64	1	66	0	0	67	1	1	0	0	2	133
8:15 AM	56	0	0	0	56	0	61	0	0	61	0	1	0	0	1	118
8:30 AM	53	0	0	0	53	3	75	0	0	78	0	8	0	0	8	139
Total	233	1	0	0	234	5	273	0	0	278	1	18	0	0	19	531
Approach %	99.6	0.4	0.0	-	-	1.8	98.2	0.0	-	-	5.3	94.7	0.0	-	-	-
Total %	43.9	0.2	0.0	-	44.1	0.9	51.4	0.0	-	52.4	0.2	3.4	0.0	-	3.6	-
PHF	0.925	0.250	0.000	-	0.914	0.417	0.910	0.000	-	0.891	0.250	0.563	0.000	-	0.594	0.941
Motorcycles	2	0	0	-	2	0	2	0	-	2	0	0	0	-	0	4
% Motorcycles	0.9	0.0	-	-	0.9	0.0	0.7	-	-	0.7	0.0	0.0	-	-	0.0	0.8
Cars & Light Goods	196	1	0	-	197	4	253	0	-	257	1	13	0	-	14	468
% Cars & Light Goods	84.1	100.0	-	-	84.2	80.0	92.7	-	-	92.4	100.0	72.2	-	-	73.7	88.1
Buses	1	0	0	-	1	0	3	0	-	3	0	2	0	-	2	6
% Buses	0.4	0.0	-	-	0.4	0.0	1.1	-	-	1.1	0.0	11.1	-	-	10.5	1.1
Single-Unit Trucks	14	0	0	-	14	1	3	0	-	4	0	2	0	-	2	20
% Single-Unit Trucks	6.0	0.0	-	-	6.0	20.0	1.1	-	-	1.4	0.0	11.1	-	-	10.5	3.8
Articulated Trucks	19	0	0	-	19	0	12	0	-	12	0	1	0	-	1	32
% Articulated Trucks	8.2	0.0	-	-	8.1	0.0	4.4	-	-	4.3	0.0	5.6	-	-	5.3	6.0
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	0.4	0.0	-	-	0.4	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.2
Bicycles on Crosswalk	-	-	-	0	-	ı	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	·	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	ı	-	-	-	-	-	-	-	-	-	-



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 5



Turning Movement Peak Hour Data Plot (7:45 AM)



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 6

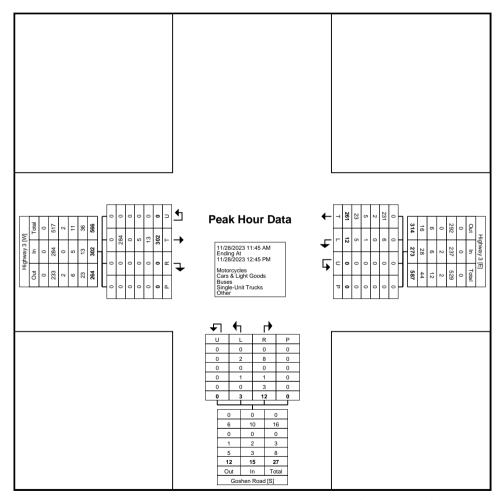
Turning Movement Peak Hour Data (11:45 AM)

					running	INIOACII	ICIIL F CO	IN I IOUI L	vala (1 1	.+J AIVI)						
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			1
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
11:45 AM	82	. 0	0	0	82	2	58	0	0	60	0	4	. 0	0	4	146
12:00 PM	63	0	0	0	63	3	74	0	0	77	1	2	0	0	3	143
12:15 PM	89	0	0	0	89	3	64	0	0	67	1	3	0	0	4	160
12:30 PM	68	. 0	. 0	0	68	4	65	0	0	69	1	3	. 0	0	4	141
Total	302	0	0	0	302	12	261	0	0	273	3	12	0	0	15	590
Approach %	100.0	0.0	0.0	-	-	4.4	95.6	0.0	-	-	20.0	80.0	0.0	-	-	-
Total %	51.2	0.0	0.0	-	51.2	2.0	44.2	0.0	-	46.3	0.5	2.0	0.0	-	2.5	-
PHF	0.848	0.000	0.000	-	0.848	0.750	0.882	0.000	-	0.886	0.750	0.750	0.000	-	0.938	0.922
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	_		-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	284	0	0	-	284	6	231	0	-	237	2	8	0	-	10	531
% Cars & Light Goods	94.0	-	-	-	94.0	50.0	88.5	-	-	86.8	66.7	66.7	-	-	66.7	90.0
Buses	0	0	0	-	0	0	2	0	-	2	0	0	0	-	0	2
% Buses	0.0	_	-	-	0.0	0.0	0.8	-	-	0.7	0.0	0.0	-	-	0.0	0.3
Single-Unit Trucks	5	0	0	-	5	1	5	0	-	6	1	1	0	-	2	13
% Single-Unit Trucks	1.7	_	-	-	1.7	8.3	1.9	-	-	2.2	33.3	8.3	-	-	13.3	2.2
Articulated Trucks	13	0	0	-	13	5	23	0	-	28	0	3	0	-	3	44
% Articulated Trucks	4.3	_	-	-	4.3	41.7	8.8	-	-	10.3	0.0	25.0	-	-	20.0	7.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	_	-	-	_	-	_	-	-	-	-	_	<u>-</u>	-	_	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	<u>-</u>	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
				,												



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 7



Turning Movement Peak Hour Data Plot (11:45 AM)



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 8

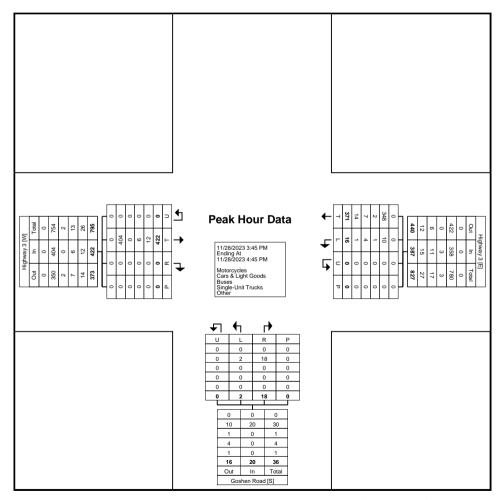
Turning Movement Peak Hour Data (3:45 PM)

					ı anınış	j iviovcii	ilonit i o	ak i loui i	Jaia (J.	. TO 1 141 <i>)</i>						
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
3:45 PM	96	0	. 0	0	96	3	90	0	0	93	1	1	0	0	2	191
4:00 PM	113	0	0	0	113	5	102	0	0	107	1	1	0	0	2	222
4:15 PM	110	0	0	0	110	4	77	0	0	81	0	5	0	0	5	196
4:30 PM	103	. 0	. 0	0	103	4	102	. 0	0	106	0	11	. 0	0	11	220
Total	422	0	0	0	422	16	371	0	0	387	2	18	0	0	20	829
Approach %	100.0	0.0	0.0	-	-	4.1	95.9	0.0	-	-	10.0	90.0	0.0	-	-	-
Total %	50.9	0.0	0.0	-	50.9	1.9	44.8	0.0	-	46.7	0.2	2.2	0.0	-	2.4	-
PHF	0.934	0.000	0.000	-	0.934	0.800	0.909	0.000	-	0.904	0.500	0.409	0.000	-	0.455	0.934
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	_	-	-	0.0	0.0	0.0		-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	404	0	0	-	404	10	348	0	-	358	2	18	0	-	20	782
% Cars & Light Goods	95.7		-	-	95.7	62.5	93.8		-	92.5	100.0	100.0		-	100.0	94.3
Buses	0	0	0	-	0	1	2	. 0	-	3	0	0	0	-	0	3
% Buses	0.0	_	-	-	0.0	6.3	0.5	<u> </u>	-	0.8	0.0	0.0	<u>-</u>	-	0.0	0.4
Single-Unit Trucks	6	0	0	-	6	4	7	0	-	11	0	0	0	-	0	17
% Single-Unit Trucks	1.4			-	1.4	25.0	1.9		-	2.8	0.0	0.0		-	0.0	2.1
Articulated Trucks	12	0	0	-	12	1	14	0	-	15	0	0	0	-	0	27
% Articulated Trucks	2.8	-	-	-	2.8	6.3	3.8	-	-	3.9	0.0	0.0	-	-	0.0	3.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	_	-	-	0.0	0.0	0.0	<u> </u>	-	0.0	0.0	0.0	<u>-</u>	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	_		-	-	-	_		-	-	-	-		-	-	-
Pedestrians	-	<u>-</u>	<u>-</u>	0	-	-	-	<u>-</u>	0	-	-	-	<u>-</u>	0	-	-
% Pedestrians	-	-		-	-	-	-	. -	-	-	-	-	<u>-</u>	-	_	-



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Turning Movement Peak Hour Data Plot (3:45 PM)



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Count Name: Access A Site Code: 230661 Start Date: 11/28/2023

Page No: 1

Turning Movement Data

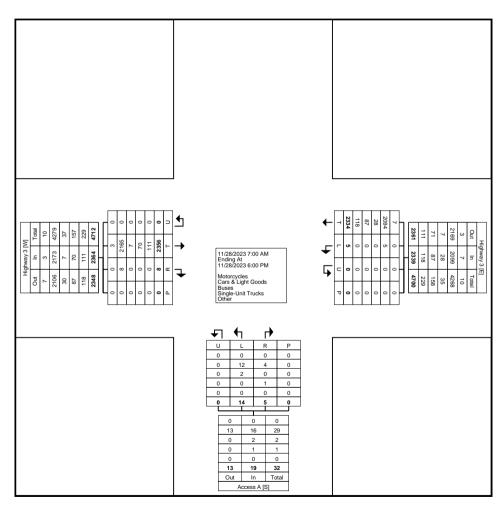
							90		- 4.4							1
			Highway 3					Highway 3					Access A			
Otant Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:00 AM	53	0	0	0	53	0	53	0	0	53	0	0	0	0	0	106
7:15 AM	72	0	0	0	72	0	62	0	0	62	0	0	0	0	0	134
7:30 AM	51	0	0	0	51	1	68	0	0	69	0	0	0	0	0	120
7:45 AM	66	3	0	0	69	1	74	0	0	75	0	0	0	0	0	144
Hourly Total	242	3	0	0	245	2	257	0	0	259	0	0	0	0	0	504
8:00 AM	65	0	0	0	65	0	65	0	0	65	0	0	0	0	0	130
8:15 AM	57	0	0	0	57	0	58	0	0	58	0	0	0	0	0	115
8:30 AM	61	1	0	0	62	0	80	0	0	80	0	0	0	0	0	142
8:45 AM	51	0	0	0	51	0	65	0	0	65	0	0	0	0	0	116
Hourly Total	234	. 1	0	0	235	0	268	0	0	268	0	0	0	0	0	503
9:00 AM	46	1	0	0	47	0	55	0	0	55	1	0	0	0	. 1	103
9:15 AM	46	0	0	0	46	0	66	0	0	66	1	0	0	0	1	113
9:30 AM	50	0	0	0	50	0	73	0	0	73	0	0	0	0	0	123
9:45 AM	42	0	0	0	42	0	69	0	0	69	0	0	0	0	0	111
Hourly Total	184	1	0	0	185	0	263	0	0	263	2	0	0	0	2	450
*** BREAK ***	-	_		-	_	-	_		-	-	-	-	-	-	-	-
11:30 AM	67	0	0	0	67	0	67	0	0	67	0	0	0	0	. 0	134
11:45 AM	81	0	0	0	81	0	62	0	0	62	0	0	0	0	0	143
Hourly Total	148	0	0	0	148	0	129	0	0	129	0	0	0	0	0	277
12:00 PM	67	0	0	0	67	1	75	0	0	76	0	0	0	0	0	143
12:15 PM	91	0	0	0	91	0	64	0	0	64	2	1	0	0	3	158
12:30 PM	71	0	0	0	71	1	67	0	0	68	0	0	. 0	0	0	139
12:45 PM	62	0	0	0	62	0	68	0	0	68	0	0	0	0	0	130
Hourly Total	291	0	0	0	291	2	274	0	0	276	2	1	0	0	3	570
1:00 PM	79	0	0	0	79	1	74	0	0	75	0	0	. 0	0	0	154
1:15 PM	57	1	0	0	58	0	72	0	0	72	0	1	0	0	. 1	131
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	136	. 1	0	0	137	1	146	0	0	147	0	1	. 0	0	1	285
3:00 PM	101	0	0	0	101	0	70	0	0	70	2	1	. 0	0	3	174
3:15 PM	72	0	0	0	72	0	82	0	0	82	0	0	0	0	0	154
3:30 PM	97	1	0	0	98	0	82	0	0	82	1	0	. 0	0	1	181
3:45 PM	100	0	0	0	100	0	91	0	0	91	1	0	0	0	. 1	192
Hourly Total	370	1	0	0	371	0	325	0	0	325	4	1	0	0	5	701
4:00 PM	114	0	0	0	114	0	109	0	0	109	1	1	0	0	2	225
4:15 PM	116	1	0	0	117	0	81	0	0	81	0	0	0	0	0	198
4:30 PM	109	0	. 0	0	109	0	104	0	0	104	0	0	0	0	. 0	213

4: 45 DM	00					_	00		0		_					407
4:45 PM	93	0	0	0	93	0	88	0	0	88	5	11	0	0	6	187
Hourly Total	432	1	0	. 0	433	0	382	0	0	382	6	2	0	0	. 8	823
5:00 PM	94	0	0	0	94	0	88	0	0	88	0	0	0	0	0	182
5:15 PM	92	0	0	0	92	0	61	0	0	61	0	0	0	0	0	153
5:30 PM	77	0	0	0	77	0	78	0	0	78	0	0	0	0	0	155
5:45 PM	56	0	0	0	56	0	63	0	0	63	0	0	0	0	0	119
Hourly Total	319	0	0	0	319	0	290	0	0	290	0	0	0	0	0	609
Grand Total	2356	8	0	0	2364	5	2334	0	0	2339	14	5	0	0	19	4722
Approach %	99.7	0.3	0.0	-	-	0.2	99.8	0.0	-	-	73.7	26.3	0.0	-	-	-
Total %	49.9	0.2	0.0	-	50.1	0.1	49.4	0.0	-	49.5	0.3	0.1	0.0	-	0.4	-
Motorcycles	3	0	0	-	3	0	7	0	-	7	0	0	0	-	0	10
% Motorcycles	0.1	0.0	-	-	0.1	0.0	0.3	-	-	0.3	0.0	0.0	-	-	0.0	0.2
Cars & Light Goods	2165	8	0	-	2173	5	2094	0	-	2099	12	4	0	-	16	4288
% Cars & Light Goods	91.9	100.0	-	-	91.9	100.0	89.7	-	-	89.7	85.7	80.0	-	-	84.2	90.8
Buses	7	0	0	-	7	0	28	0	-	28	2	0	0	-	2	37
% Buses	0.3	0.0	-	-	0.3	0.0	1.2	-	-	1.2	14.3	0.0	-	-	10.5	0.8
Single-Unit Trucks	70	0	0	-	70	0	87	0	-	87	0	1	0	-	1	158
% Single-Unit Trucks	3.0	0.0	-	-	3.0	0.0	3.7	-	-	3.7	0.0	20.0	-	-	5.3	3.3
Articulated Trucks	111	0	0	-	111	0	116	0	-	116	0	0	0	-	0	227
% Articulated Trucks	4.7	0.0	-	-	4.7	0.0	5.0	-	-	5.0	0.0	0.0	-	-	0.0	4.8
Bicycles on Road	0	0	0	-	0	0	2	0	-	2	0	0	0	-	0	2
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.1	-	-	0.1	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
				-	-					-					-	-



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Turning Movement Data Plot



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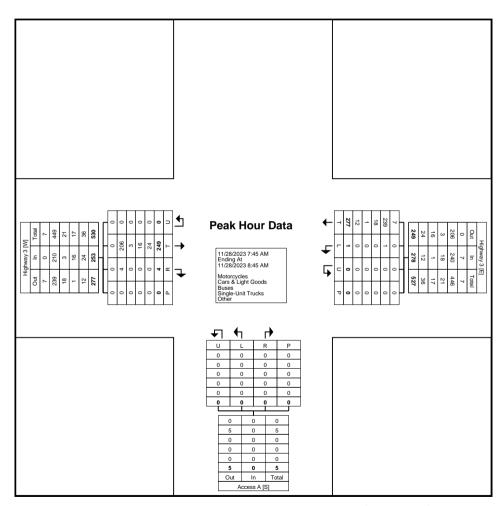
Turning Movement Peak Hour Data (7:45 AM)

,					rumni	j woven	nent Pe	ak nour i	Jaia (7.	.43 AIVI)						
			Highway 3					Highway 3					Access A			
Start Time			Eastbound					Westbound					Northbound			
- Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:45 AM	66	3	. 0	0	69	1	74	0	0	75	0	. 0	. 0	0	. 0	144
8:00 AM	65	0	0	0	65	0	65	0	0	65	0	0	0	0	0	130
8:15 AM	57	0	0	0	57	0	58	0	0	58	0	0	0	0	0	115
8:30 AM	61	1	0	0	62	0	80	0	0	80	0	0	0	0	0	142
Total	249	4	0	0	253	1	277	0	0	278	0	0	0	0	0	531
Approach %	98.4	1.6	0.0	-	-	0.4	99.6	0.0	-	-	0.0	0.0	0.0	-	-	-
Total %	46.9	0.8	0.0	-	47.6	0.2	52.2	0.0	-	52.4	0.0	0.0	0.0	-	0.0	-
PHF	0.943	0.333	0.000	-	0.917	0.250	0.866	0.000	-	0.869	0.000	0.000	0.000	-	0.000	0.922
Motorcycles	0	0	0	-	0	0	7	0	-	7	0	0	0	-	0	7
% Motorcycles	0.0	0.0	-	-	0.0	0.0	2.5	-	-	2.5	-	_	-	-	-	1.3
Cars & Light Goods	206	4	0	-	210	1	239	0	-	240	0	0	0	-	0	450
% Cars & Light Goods	82.7	100.0	-	-	83.0	100.0	86.3	-	-	86.3	-	-		-	-	84.7
Buses	3	0	0	-	3	0	18	0	-	18	0	0	0	-	0	21
% Buses	1.2	0.0	-	-	1.2	0.0	6.5	-	-	6.5	-	-	-	-	-	4.0
Single-Unit Trucks	16	0	0	-	16	0	1	0	-	1	0	0	0	-	0	17
% Single-Unit Trucks	6.4	0.0	-	-	6.3	0.0	0.4	-	-	0.4	-	-	-	-	-	3.2
Articulated Trucks	24	0	0	-	24	0	12	0	-	12	0	0	0	-	0	36
% Articulated Trucks	9.6	0.0	-	-	9.5	0.0	4.3	-	-	4.3	-	-	-	-	-	6.8
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (7:45 AM)



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Count Name: Access A Site Code: 230661 Start Date: 11/28/2023 Page No: 6

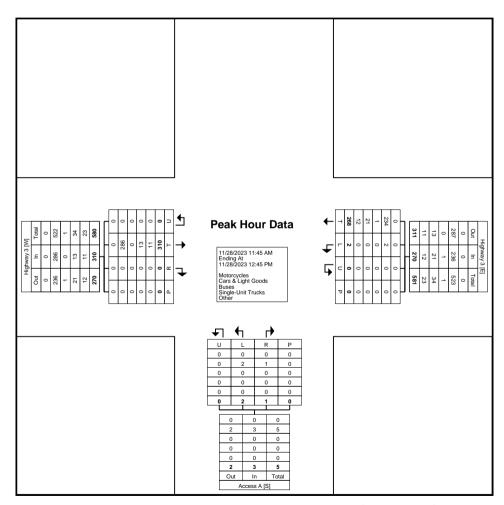
Turning Movement Peak Hour Data (11:45 AM)

					running	ivioveiii	ieni Pea	IK HOUI L	vala (1 1	.45 AIVI)						
			Highway 3					Highway 3					Access A			
Start Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
11:45 AM	81	0	0	0	81	0	62	0	0	62	0	0	0	0	0	143
12:00 PM	67	0	0	0	67	1	75	0	0	76	0	0	0	0	0	143
12:15 PM	91	0	0	0	91	0	64	0	0	64	2	1	0	0	3	158
12:30 PM	71	0	0	0	71	1	67	0	0	68	0	0	0	0	0	139
Total	310	0	0	0	310	2	268	0	0	270	2	1	0	0	3	583
Approach %	100.0	0.0	0.0	-	-	0.7	99.3	0.0	-	-	66.7	33.3	0.0	-	-	-
Total %	53.2	0.0	0.0	-	53.2	0.3	46.0	0.0	-	46.3	0.3	0.2	0.0	-	0.5	-
PHF	0.852	0.000	0.000	-	0.852	0.500	0.893	0.000	-	0.888	0.250	0.250	0.000	-	0.250	0.922
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	-	-	-	0.0	0.0	0.0	<u>-</u>	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	286	0	0	-	286	2	234	0	-	236	2	1	0	-	3	525
% Cars & Light Goods	92.3	_	-	-	92.3	100.0	87.3	-	-	87.4	100.0	100.0		-	100.0	90.1
Buses	0	0	0	-	0	0	1	0	-	1	0	0	0	-	0	1
% Buses	0.0	_	-	-	0.0	0.0	0.4	-	-	0.4	0.0	0.0		-	0.0	0.2
Single-Unit Trucks	13	0	0	-	13	0	21	0	-	21	0	0	0	-	0	34
% Single-Unit Trucks	4.2	_	<u>-</u>	-	4.2	0.0	7.8	-	-	7.8	0.0	0.0		-	0.0	5.8
Articulated Trucks	11	0	0	-	11	0	12	0	-	12	0	0	0	-	0	23
% Articulated Trucks	3.5	-	-	-	3.5	0.0	4.5	-	-	4.4	0.0	0.0	-	-	0.0	3.9
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	i	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-		0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (11:45 AM)



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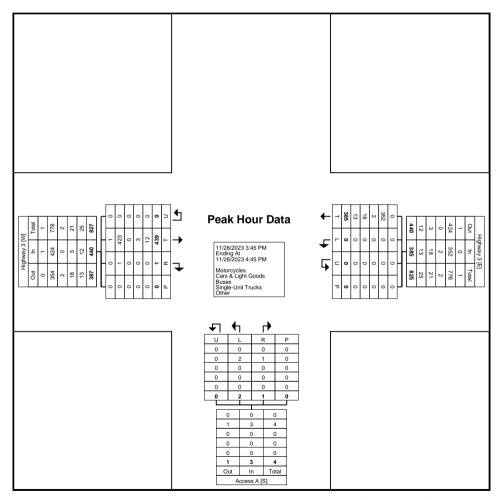
Turning Movement Peak Hour Data (3:45 PM)

Highway 3 Highway 3 Eastbound Westbound				Access A			
Monthound							
Start Time Eastbourid vvestbourid							
Thru Right U-Turn Peds App. Total Left Thru U-Turn Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
3:45 PM 100 0 0 0 100 0 91 0 0	91	1	0	0	0	. 1	192
4:00 PM 114 0 0 0 114 0 109 0 0	109	1	1	0	0	2	225
4:15 PM 116 1 0 0 117 0 81 0 0	81	0	0	0	0	0	198
4:30 PM 109 0 0 0 109 0 104 0 0	104	0	0	. 0	0	. 0	213
	385	2	1	0	0	3	828
Approach % 99.8 0.2 0.0 0.0 100.0 0.0 -	-	66.7	33.3	0.0	-	-	-
Total % 53.0 0.1 0.0 - 53.1 0.0 46.5 0.0 -	46.5	0.2	0.1	0.0	-	0.4	-
PHF 0.946 0.250 0.000 - 0.940 0.000 0.883 0.000 -	0.883	0.500	0.250	0.000	-	0.375	0.920
Motorcycles 1 0 0 - 1 0 0 0 -	0	0	0	0	-	0	1
% Motorcycles 0.2 0.0 0.2 - 0.0	0.0	0.0	0.0	<u>-</u>	-	0.0	0.1
Cars & Light Goods 423 1 0 - 424 0 352 0 -	352	2	1	0	-	3	779
% Cars & Light Goods 96.4 100.0 96.4 - 91.4	91.4	100.0	100.0	-	-	100.0	94.1
Buses 0 0 0 - 0 0 2 0 -	2	0	0	0	-	0	2
% Buses 0.0 0.0 0.0 - 0.5	0.5	0.0	0.0	-	-	0.0	0.2
Single-Unit Trucks 3 0 0 - 3 0 18 0 -	18	0	0	0	-	0	21
% Single-Unit Trucks 0.7 0.0 0.7 - 4.7	4.7	0.0	0.0	-	-	0.0	2.5
Articulated Trucks 12 0 0 - 12 0 13 0 -	13	0	0	0	-	0	25
% Articulated Trucks 2.7 0.0 2.7 - 3.4	3.4	0.0	0.0	-	-	0.0	3.0
Bicycles on Road 0 0 0 - 0 0 0 - 0 - 0 0 0	0	0	0	0	-	0	0
% Bicycles on Road 0.0 0.0 0.0 - 0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk 0 0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	<u>-</u>	-
Pedestrians 0 0	-	-	-	-	0	<u>-</u>	-
% Pedestrians	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (3:45 PM)



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Count Name: Bilger Access Site Code: 230661 Start Date: 11/28/2023

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Turning Movement Data

							90		- 4.4							
			Bilger Access					Goshen Road					Goshen Road			
Start Time	Westbound							Northbound					Southbound			
Start Time	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
7:00 AM	0	0	0	0	0	2	0	0	0	2	1	3	0	0	4	6
7:15 AM	0	0	0	0	0	4	0	0	0	4	1	1	0	0	2	6
7:30 AM	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7
7:45 AM	0	0	0	0	0	7	0	0	0	7	0	1	0	0	1	8
Hourly Total	0	0	0	0	0	17	0	0	0	17	2	8	0	0	10	27
8:00 AM	0	0	0	0	0	1	0	0	0	1	0	1	1	0	2	3
8:15 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	7	0	0	0	7	2	1	0	0	3	10
8:45 AM	1	0	0	0	1	5	0	0	0	5	0	3	0	0	3	9
Hourly Total	1	0	0	0	1	14	0	0	0	14	2	5	1	0	8	23
9:00 AM	0	2	0	0	2	2	0	0	0	2	0	0	0	0	0	4
9:15 AM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
9:30 AM	1	0	0	0	1	3	0	0	0	3	1	1	0	0	2	6
9:45 AM	1	0	0	0	1	1	0	0	0	1	1	4	0	0	5	7
Hourly Total	2	2	0	0	4	7	0	0	0	7	2	6	0	0	8	19
*** BREAK ***	-			-		-	_		-	-	-	_	-	-	-	-
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
11:45 AM	0	0	0	0	0	4	0	0	0	4	0	2	0	0	2	6
Hourly Total	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7
12:00 PM	0	0	0	0	0	3	0	0	0	3	2	1	0	0	3	6
12:15 PM	0	2	0	0	2	3	0	0	0	3	0	3	0	0	3	8
12:30 PM	0	0	0	0	0	4	0	0	0	4	0	4	0	0	4	8
12:45 PM	0	0	0	0	0	1	0	0	0	1	0	3	0	0	3	4
Hourly Total	0	2	0	0	2	11	0	0	0	11	2	11	0	0	13	26
1:00 PM	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	2
1:15 PM	0	0	0	0	0	3	0	0	0	3	0	1	0	0	1	4
*** BREAK ***	-			-	-	-			-	-	-			-	-	-
Hourly Total	0	0	0	0	0	5	0	0	0	5	0	1	0	0	1	6
3:00 PM	0	1	0	0	1	3	0	0	0	3	2	0	0	0	2	6
3:15 PM	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	2
3:30 PM	0	0	0	0	0	2	0	0	0	2	0	1	0	0	1	3
3:45 PM	0	1	0	0	1	1	0	0	0	1	2	1	0	0	3	5
Hourly Total	0	2	0	0	2	8	0	0	0	8	4	2	0	0	6	16
4:00 PM	1	1	0	0	2	1	0	0	0	1	0	0	0	0	0	3
4:15 PM	0	0	0	0	0	5	3	0	0	8	1	3	0	0	4	12
4:30 PM	0	0	0	0	0	11	0	0	0	11	1	2	0	0	3	14

4:45 PM	0	3	0	0	3	3	0	0	0	3	1	3	0	0	4	10
Hourly Total	1	4	0	0	5	20	3	0	0	23	3	8	0	0	11	39
5:00 PM	0	2	0	0	2	0	0	0	0	0	0	1	0	0	1	3
5:15 PM	0	0	0	0	0	2	0	0	0	2	1	2	0	0	3	5
5:30 PM	1	0	0	0	1	0	0	0	0	0	2	2	0	0	4	5
5:45 PM	0	1	0	0	1	4	0	0	0	4	2	2	0	0	4	9
Hourly Total	1	3	0	0	4	6	0	0	0	6	5	7	0	0	12	22
Grand Total	5	13	0	0	18	92	3	0	0	95	20	51	1	0	72	185
Approach %	27.8	72.2	0.0	-	-	96.8	3.2	0.0	-	-	27.8	70.8	1.4	-	-	-
Total %	2.7	7.0	0.0	-	9.7	49.7	1.6	0.0	-	51.4	10.8	27.6	0.5	-	38.9	-
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Cars & Light Goods	4	10	0	-	14	77	3	0	-	80	9	38	1	-	48	142
% Cars & Light Goods	80.0	76.9	-	-	77.8	83.7	100.0	-	-	84.2	45.0	74.5	100.0	-	66.7	76.8
Buses	0	0	0	-	0	2	0	0	-	2	0	3	0	-	3	5
% Buses	0.0	0.0	-	-	0.0	2.2	0.0	-	-	2.1	0.0	5.9	0.0	-	4.2	2.7
Single-Unit Trucks	1	2	0	-	3	10	0	0	-	10	9	6	0	-	15	28
% Single-Unit Trucks	20.0	15.4	-	-	16.7	10.9	0.0	-	-	10.5	45.0	11.8	0.0	-	20.8	15.1
Articulated Trucks	0	1	0	-	1	3	0	0	-	3	2	4	0	-	6	10
% Articulated Trucks	0.0	7.7	-	-	5.6	3.3	0.0	-	-	3.2	10.0	7.8	0.0	-	8.3	5.4
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: Bilger Access Site Code: 230661 Start Date: 11/28/2023 Page No: 3

 Goshen Road [N]

 Out
 In
 Total
 L U P 11/28/2023 7:00 AM Ending At 11/28/2023 6:00 PM Motorcycles Cars & Light Goods Buses Single-Unit Trucks Other 56 95 151 Goshen Road [S]

Turning Movement Data Plot



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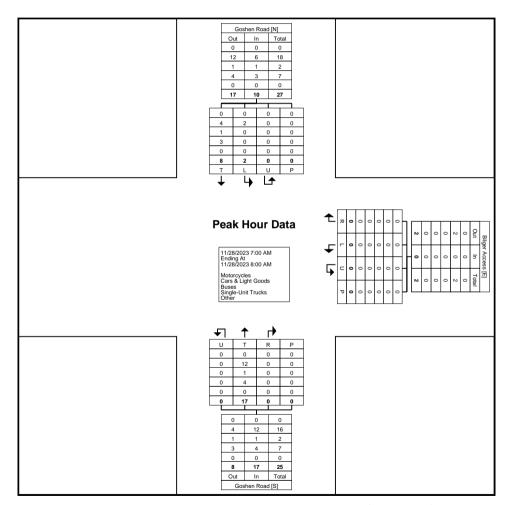
Turning Movement Peak Hour Data (7:00 AM)

					runni	i ivioveii	ICHT I C	ak moui i	Jaia (1.	.00 AIVI) _,						ı		
			Bilger Access					Goshen Road			Goshen Road							
Start Time			Westbound					Northbound										
Start Time	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total		
7:00 AM	0	0	0	0	0	2	0	0	0	2	1	3	0	0	4	6		
7:15 AM	0	0	0	0	0	4	0	0	0	4	1	1	0	0	2	6		
7:30 AM	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7		
7:45 AM	0	0	0	0	0	7	0	0	0	7	0	1	0	0	1	8		
Total	0	0	0	0	0	17	0	0	0	17	2	8	0	0	10	27		
Approach %	0.0	0.0	0.0	-	-	100.0	0.0	0.0	-	-	20.0	80.0	0.0	-	-	-		
Total %	0.0	0.0	0.0	-	0.0	63.0	0.0	0.0	-	63.0	7.4	29.6	0.0	-	37.0	-		
PHF	0.000	0.000	0.000	-	0.000	0.607	0.000	0.000	-	0.607	0.500	0.667	0.000	-	0.625	0.844		
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0		
% Motorcycles	-	_	-	-	-	0.0	_	-	-	0.0	0.0	0.0	-	-	0.0	0.0		
Cars & Light Goods	0	0	0	-	0	12	0	0	-	12	2	4	0	-	6	18		
% Cars & Light Goods	-	-	-	-	-	70.6	-	-	-	70.6	100.0	50.0	-	-	60.0	66.7		
Buses	0	0	0	-	0	1	0	0	-	1	0	1	0	-	1	2		
% Buses	-	-	-	-	-	5.9	-	-	-	5.9	0.0	12.5	-	-	10.0	7.4		
Single-Unit Trucks	0	0	0	-	0	4	0	0	-	4	0	3	0	-	3	7		
% Single-Unit Trucks	-	<u>-</u>	-	-	-	23.5	_	-	-	23.5	0.0	37.5	-	-	30.0	25.9		
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0		
% Articulated Trucks	-	-	-	-	-	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0		
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0		
% Bicycles on Road	-	-	-	-	-	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0		
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-		
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-		
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-		



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Turning Movement Peak Hour Data Plot (7:00 AM)



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Turning Movement Peak Hour Data (11:45 AM)

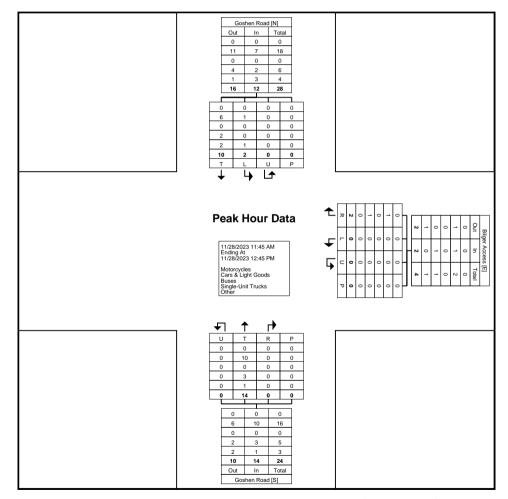
			Bilger Access					Goshen Road								
Start Time			Westbound					Northbound								
L	_eft	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
11:45 AM	0	0	0	0	0	4	0	0	0	4	0	2	0	0	2	6
12:00 PM	0	0	0	0	0	3	0	0	0	3	2	1	0	0	3	6
12:15 PM	0	2	0	0	2	3	0	0	0	3	0	3	0	0	3	8
12:30 PM	0	0	0	0	. 0	4	0	0	0	4	0	4	. 0	0	4	8
Total	0	2	0	0	2	14	0	0	0	14	2	10	0	0	12	28
Approach %	0.0	100.0	0.0	-	-	100.0	0.0	0.0	-	-	16.7	83.3	0.0	-	-	-
Total %	0.0	7.1	0.0	-	7.1	50.0	0.0	0.0	-	50.0	7.1	35.7	0.0	-	42.9	-
PHF 0.	.000	0.250	0.000	-	0.250	0.875	0.000	0.000	-	0.875	0.250	0.625	0.000	-	0.750	0.875
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	-	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	0	1	0	-	1	10	0	0	-	10	1	6	0	-	7	18
% Cars & Light Goods	-	50.0	-	-	50.0	71.4	-	-	-	71.4	50.0	60.0	-	-	58.3	64.3
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	-	0.0	-	-	0.0	0.0	-		-	0.0	0.0	0.0	<u> </u>	-	0.0	0.0
Single-Unit Trucks	0	1	0	-	1	3	0	0	-	3	0	2	0	-	2	6
% Single-Unit Trucks	-	50.0	-	-	50.0	21.4	-		-	21.4	0.0	20.0		-	16.7	21.4
Articulated Trucks	0	0	0	-	0	1	0	0	-	1	1	2	0	-	3	4
% Articulated Trucks	-	0.0	-	-	0.0	7.1	-	-	-	7.1	50.0	20.0	-	-	25.0	14.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Paradigm Transportation Solutions Limited 5A-150 Pinebush Rd

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Turning Movement Peak Hour Data Plot (11:45 AM)



Paradigm Transportation Solutions Limited 5A-150 Pinebush Rd

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Turning Movement Peak Hour Data (4:00 PM)

ı					Turning	j ivioven	nent Pea	ak Hour I	Jata (4)	(UU PIVI)						İ
			Bilger Access					Goshen Road					Goshen Road			
Start Time			Westbound					Northbound					Southbound			
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
4:00 PM	1	1	. 0	0	2	1	0	. 0	0	. 1	0	0	0	0	. 0	3
4:15 PM	0	0	0	0	0	5	3	0	0	8	1	3	0	0	4	12
4:30 PM	0	0	0	0	0	11	0	0	0	11	1	2	0	0	3	14
4:45 PM	0	3	. 0	0	3	3	0	. 0	0	3	1	3	. 0	0	. 4	10
Total	1	4	0	0	5	20	3	0	0	23	3	8	0	0	11	39
Approach %	20.0	80.0	0.0	-	-	87.0	13.0	0.0	-	-	27.3	72.7	0.0	-	-	-
Total %	2.6	10.3	0.0	-	12.8	51.3	7.7	0.0	-	59.0	7.7	20.5	0.0	-	28.2	
PHF	0.250	0.333	0.000	-	0.417	0.455	0.250	0.000	-	0.523	0.750	0.667	0.000	-	0.688	0.696
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	0	3	0	-	3	20	3	0	-	23	0	8	0	-	8	34
% Cars & Light Goods	0.0	75.0	-	-	60.0	100.0	100.0	-	-	100.0	0.0	100.0	-	-	72.7	87.2
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	1	1	0	-	2	0	0	0	-	0	3	0	0	-	3	5
% Single-Unit Trucks	100.0	25.0	-	-	40.0	0.0	0.0	-	-	0.0	100.0	0.0	-	-	27.3	12.8
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-		0	-	-	-	-	0	_	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	_	-	-	-	-	_	-	-	-	-	_	-	-	-



Paradigm Transportation Solutions Limited 5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Bilger Access Site Code: 230661 Start Date: 11/28/2023 Page No: 9

L U P **Peak Hour Data** 11/28/2023 4:00 PM Ending At 11/28/2023 5:00 PM Motorcycles Cars & Light Goods Buses Single-Unit Trucks Other 9 23 32 Goshen Road [S]

Turning Movement Peak Hour Data Plot (4:00 PM)

Appendix C

Existing Traffic Operations Reports

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	•	7	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	Y	
Traffic Volume (vph)	233	1	5	273	1	18
Future Volume (vph)	233	1	5	273	1	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1666	0	0	1787	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1666	0	0	1787	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	253	1	5	297	1	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	254	0	0	302	21	0
Sign Control	Free			Free	Stop	
l-t						

Intersection Summary Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.4%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	233	1	5	273	1	18
Future Vol. veh/h	233	1	5	273	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	
Storage Length		-		-	0	-
Veh in Median Storage	e.# 0	_	_	0	0	_
Grade. %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	253	1	5	297	1	20
WWIT Flow	253	- 1	5	297	- 1	20
	Major1	1	Major2	1	Minor1	
Conflicting Flow All	0	0	254	0	561	254
Stage 1	-	-	-	-	254	-
Stage 2	-	-	-	-	307	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-		3.453
Pot Cap-1 Maneuver	_	-	1213	-	492	749
Stage 1	-	-	-	-	793	
Stage 2	_	_	_	-	751	-
Platoon blocked, %		-		-	701	
Mov Cap-1 Maneuver			1213	-	490	749
Mov Cap-1 Maneuver	-		1213	-	490	149
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	747	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.1	
HCM LOS	U		0.1		В	
TIOW LOO						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		729	-	-	1213	-
HCM Lane V/C Ratio		0.028	-	-	0.004	-
HCM Control Delay (s)	10.1	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh	1)	0.1	-	-	0	-
	,					

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	*	•	•	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.			ર્ન	¥		
Traffic Volume (vph)	249	4	1	277	0	0	
Future Volume (vph)	249	4	1	277	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.998						
Flt Protected							
Satd. Flow (prot)	1638	0	0	1810	1900	0	
Flt Permitted							
Satd. Flow (perm)	1638	0	0	1810	1900	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%	
Adj. Flow (vph)	271	4	1	301	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	275	0	0	302	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary							

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 18.7%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3

Paradigm Transportation Solutions Limited

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	¥	
Traffic Vol, veh/h	249	4	1	277	0	0
Future Vol, veh/h	249	4	1	277	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	271	4	1	301	0	0
WWITETIOW	211	7		001	U	U
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	275	0	576	273
Stage 1	-	-	-	-	273	-
Stage 2	-	-	-	-	303	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1300	-	482	771
Stage 1	-	-	-	-	778	-
Stage 2	-	-	-	-	754	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1300	-	482	771
Mov Cap-2 Maneuver	-	-	-	-	482	-
Stage 1	-	_	_	_	778	-
Stage 2	-	-	-	-	753	-
5.030 L						
Approach	EB		WB		NB	
	0					
HCM Control Delay, s	U		0		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1300	-
HCM Lane V/C Ratio		-	-	-	0.001	-
HCM Control Delay (s)		0	_	_	7.8	0
HCM Lane LOS		A	-	-	Α.	A
HCM 95th %tile Q(veh)		-	_	_	0	-
TIOM JOHN JUHIC Q(VEII)					J	

Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	422	0	16	371	2	18
Future Volume (vph)	422	0	16	371	2	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.877	
Flt Protected				0.998	0.995	
Satd. Flow (prot)	1827	0	0	1772	1658	0
Flt Permitted				0.998	0.995	
Satd. Flow (perm)	1827	0	0	1772	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	459	0	17	403	2	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	459	0	0	420	22	0
Sign Control	Free			Free	Stop	

Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 42.5%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	,,,,,,
Traffic Vol, veh/h	422	0	16	371	2	18
Future Vol. veh/h	422	0	16	371	2	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	Stop -	None
Storage Length		NOTICE -	-	NOTICE -	0	NOHE
Veh in Median Storage	e. # 0	-	-	0	0	
Grade, %	0		-	0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mvmt Flow	459	0	17	403	2	20
Major/Minor I	Major1	1	Major2	1	Minor1	
Conflicting Flow All	0	0	459	0	896	459
Stage 1	-	-	-	-	459	-
Stage 2	-	-	-	-	437	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	_	-	_	5.4	_
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	965	_	313	606
Stage 1			300		641	-
Stage 2	-	-		-	655	
Platoon blocked, %			_		055	_
	-	-	005	-	200	000
Mov Cap-1 Maneuver	-	-	965	-	306	606
Mov Cap-2 Maneuver	-	-	-	-	306	-
Stage 1	-	-	-	-	641	-
Stage 2	-	-	-	-	640	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.8	
HCM LOS	U		0.4		11.0 B	
HUM LUS					В	
Minor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		552	-	-	965	-
HCM Lane V/C Ratio		0.039			0.018	
HCM Control Delay (s)		11.8	_	-	8.8	0
HCM Lane LOS		В	-	-	A	A
HCM 95th %tile Q(veh)	١	0.1	_	_	0.1	

Page 1

Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	€	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			ર્ન	Y	
Traffic Volume (vph)	439	1	0	385	2	1
Future Volume (vph)	439	1	0	385	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	477	1	0	418	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	478	0	0	418	3	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 33.2%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3 Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.1					
• •	EDT	EDD	WDI	WDZ	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	¥	
Traffic Vol, veh/h	439	1	0	385	2	1
Future Vol, veh/h	439	1	0	385	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	477	1	0	418	2	1
					_	
	lajor1		/lajor2		Minor1	
Conflicting Flow All	0	0	478	0	896	478
Stage 1	-	-	-	-	478	-
Stage 2	-	-	-	-	418	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	_	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	1095	_	313	591
Stage 1			-	-	628	-
Stage 2	_	_	_	_	669	-
Platoon blocked, %	-				000	
Mov Cap-1 Maneuver	-	-	1095	-	313	591
					313	- 180
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	628	-
Stage 2	-	-	-	-	669	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		14.8	
HCM LOS	U		0		14.0 B	
I IOWI LUO					٥	
Minor Lane/Major Mvmt	I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		371	-	-	1095	-
HCM Lane V/C Ratio		0.009			-	
HCM Control Delay (s)		14.8	_	-	0	-
HCM Lane LOS		В	-		A	
HCM 95th %tile Q(veh)		0	-		0	-
HOW SOUL WILL CA (VELL)		U		_	U	

Appendix D

2024 Background Traffic Operations Reports

2024 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

2024 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Lane Group EBT EBR WBL WBT NBL NBR Lane Configurations ♣ ♣ ♣ ♣ ♣ ✔
Traffic Volume (vph) 238 1 5 278 1 18 Future Volume (vph) 238 1 5 278 1 18 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 100 1.00
Future Volume (vph) 238 1 5 278 1 18 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 100 1.00
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 100 1.00
Lane Util. Factor 1.00
Fit 0.999 0.871 Fit Protected 0.999 0.998 Satd. Flow (prot) 1666 0 0 1787 1421 0 Fit Permitted 0.999 0.998 Satd. Flow (perm) 1666 0 0 1787 1421 0 Link Speed (k/h) 50 50 50 50 50 50 Link Distance (m) 256.2 158.5 186.1
Fit Protected 0.999 0.998 Satd. Flow (prot) 1666 0 0 1787 1421 0 Fit Permitted 0.999 0.998 0.998 Satd. Flow (perm) 1666 0 1787 1421 0 Link Speed (k/h) 50 50 50 50 50 Link Distance (m) 256.2 158.5 186.1
Satd. Flow (prot) 1666 0 0 1787 1421 0 Flt Permitted 0.999 0.998 <t< td=""></t<>
Fit Permitted 0.999 0.998 Satd. Flow (perm) 1666 0 0 1787 1421 0 Link Speed (k/h) 50 50 50 Link Distance (m) 256.2 158.5 186.1
Satd. Flow (perm) 1666 0 0 1787 1421 0 Link Speed (k/h) 50 50 50 Link Distance (m) 256.2 158.5 186.1
Link Speed (k/h) 50 50 50 Link Distance (m) 256.2 158.5 186.1
Link Distance (m) 256.2 158.5 186.1
Travel Time (a) 10.4 11.4 12.4
Travel Time (s) 18.4 11.4 13.4
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92
Heavy Vehicles (%) 14% 0% 20% 6% 0% 17%
Adj. Flow (vph) 259 1 5 302 1 20
Shared Lane Traffic (%)
Lane Group Flow (vph) 260 0 0 307 21 0
Sign Control Free Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.6% ICU Level of Service A
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	0.4					
• •	EDT	EDD	WDI	WDT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	,	-	4	Y	40
Traffic Vol, veh/h	238	1	5	278	1	18
Future Vol, veh/h	238	1	5	278	1	18
Conflicting Peds, #/hr	_ 0	0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	259	1	5	302	1	20
		•	_			
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	260	0	572	260
Stage 1	-	-	-	-	260	-
Stage 2	-	-	-	-	312	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	_	5.4	-
Follow-up Hdwy	-	-	2.38	-	3.5	3.453
Pot Cap-1 Maneuver	-	-	1207	-	485	744
Stage 1	-	-	-	-	788	-
Stage 2	-	-	_	_	747	_
Platoon blocked. %					1-11	
Mov Cap-1 Maneuver			1207	_	483	744
Mov Cap-1 Maneuver			1201		483	- 144
Stage 1			-		788	-
	-	-				
Stage 2	-	-	-	-	743	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.1	
HCM LOS	U		0.1		В	
I IOW LOG					D	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		723	-	-	1207	-
HCM Lane V/C Ratio		0.029	-	-		-
HCM Control Delay (s)		10.1	-	_	8	0
HCM Lane LOS		В	-		A	A
HCM 95th %tile Q(veh)	١	0.1	_		0	
HOW SOUL FOUR Q(VEIL)	1	0.1			U	_

	-	\rightarrow	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^			ર્ન	Y	
Traffic Volume (vph)	254	4	1	283	0	0
Future Volume (vph)	254	4	1	283	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1638	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1638	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	276	4	1	308	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	280	0	0	309	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 19.0%			IC	CU Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
			14/51	14/DT	N.D.	ND.
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			4	Y	
Traffic Vol, veh/h	254	4	1	283	0	0
Future Vol, veh/h	254	4	1	283	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mvmt Flow	276	4	1	308	0	0
Major/Minor N	Major1		Major2		Minor1	
Conflicting Flow All	0	0	280	0	588	278
Stage 1	U	·	200	-	278	210
Stage 2		-			310	_
Critical Hdwy	-	-	4.1		6.4	6.2
			4.1	-	5.4	0.2
Critical Hdwy Stg 1	-				5.4	-
Critical Hdwy Stg 2	-	-	-	-		
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1294	-	475	766
Stage 1	-	-	-	-	774	-
Stage 2	-	-	-	-	748	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1294	-	475	766
Mov Cap-2 Maneuver	-	-	-	-	475	-
Stage 1	-	-	-	_	774	-
Stage 2	-	-	-	-	747	-
Annraaah	EB		WB		NB	
Approach	0		0		0	
HCM Control Delay, s	0		0		•	
HCM LOS					Α	
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-		-	1294	-
HCM Lane V/C Ratio				_	7.8	0
HCM Lane V/C Ratio		0	_			
HCM Control Delay (s)		0 A	-			_
		0 A	-	-	7.0 A	A

HCM 6th TWSC

2: Access A & Highway 3

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2024 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	•	7	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	430	0	16	378	2	18
Future Volume (vph)	430	0	16	378	2	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.877	
Flt Protected				0.998	0.995	
Satd. Flow (prot)	1827	0	0	1772	1658	0
Flt Permitted				0.998	0.995	
Satd. Flow (perm)	1827	0	0	1772	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	467	0	17	411	2	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	467	0	0	428	22	0
Sign Control	Free			Free	Stop	
l-t						

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 42.9%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2024 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

0.5					
	EBR	WBL			NBR
	-				18
430	0	16	378	2	18
0	0	0	0	0	0
Free	Free	Free	Free	Stop	Stop
-	None	-	None	-	None
-	-	-	-	0	-
# 0	-	-	0	0	-
0	-	-	0	0	-
92	92	92	92	92	92
4	0	31	6	0	0
467	0	17	411	2	20
-	0				467
	-				-
	-				-
	-				6.2
	-				-
	-				-
-	-		-		3.3
-	-	958	-		600
-	-	-	-		-
-	-	-	-	650	-
-	-		-		
-	-	958	-	300	600
-	-	-	-	300	-
-	-	-	-	635	-
-	-	-	-	635	-
		14/0		ND	
U		0.4			
				В	
t 1	NBLn1	EBT	EBR	WBL	WBT
	545	-	-	958	-
	0.04	-	-	0.018	-
			_	8.8	0
	11.9	-	-		
	11.9 B	-		Α.	Ā
	0 Free	# 0	## Company	## Company of Company	## BT EBR WBL WBT NBL ## A30 0 16 378 2 ## A30 0 16 378 2 ## A30 0 0 0 0 ## O - - - - 0 ## O - - - 0 0 ## O - - 0 0 0 ## O 0 0 0 0 ## O 0 0 ## O 0 0 0 ## O 0 0 0 ## O 0 0 0 ## O

Synchro 11 Report

Page 1

	-	•	•	-	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	¥	
Traffic Volume (vph)	448	1	0	393	2	1
Future Volume (vph)	448	1	0	393	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	487	1	0	427	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	488	0	0	427	3	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 33.6%			IC	CU Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
•			MD:	MIDT	ND	NDC
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	448	1	0	393	2	1
Future Vol, veh/h	448	1	0	393	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	487	1	0	427	2	1
WWW.CTIOW	101		U	121	_	
Major/Minor N	Major1	1	Major2	ı	Minor1	
Conflicting Flow All	0	0	488	0	915	488
Stage 1	-	-	-	-	488	-
Stage 2	-	-	-	-	427	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1		-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy		-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver		-	1086	-	305	584
Stage 1			-	-	621	-
Stage 2		_		_	662	_
Platoon blocked. %	-	-	_	-	002	_
Mov Cap-1 Maneuver			1086	-	305	584
Mov Cap-1 Maneuver			1000		305	304
			-			
Stage 1	-	-	-	-	621	-
Stage 2	-	-	-	-	662	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		15	
HCM LOS	U		U		C	
HOW LOS					U	
Minor Lane/Major Mvm	it I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		363	-	-	1086	-
HCM Lane V/C Ratio		0.009		-	-	-
HCM Control Delay (s)		15	-	-	0	_
HCM Lane LOS		C			A	
HCM 95th %tile Q(veh)	١	0	_	_	0	_
TION JOHN JOHN Q(VOII)		0			0	

HCM 6th TWSC

2: Access A & Highway 3

Appendix E

2024 Total Traffic Operations Reports

2024 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

Page 1

	-	•	•	•	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			ર્ન	¥		
Traffic Volume (vph)	242	1	5	278	1	18	
Future Volume (vph)	242	1	5	278	1	18	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.999				0.871		
Flt Protected				0.999	0.998		
Satd. Flow (prot)	1666	0	0	1787	1421	0	
Flt Permitted				0.999	0.998		
Satd. Flow (perm)	1666	0	0	1787	1421	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	256.2			158.5	186.1		
Travel Time (s)	18.4			11.4	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%	
Adj. Flow (vph)	263	1	5	302	1	20	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	264	0	0	307	21	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize							
Intersection Capacity Utili	zation 28.6%			IC	CU Level	of Service A	Α
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	LDIN	TTDL	4	¥	HUIT
Traffic Vol, veh/h	242	1	5	278	1	18
Future Vol. veh/h	242	1	5	278	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length		None -	-	NOHE -	0	NOHE -
Veh in Median Storage,		-		0	0	-
Grade, %	# 0	-	-	0	0	-
	92			92	-	
Peak Hour Factor		92	92		92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mvmt Flow	263	1	5	302	1	20
Major/Minor N	lajor1	N	Major2	. 1	Minor1	
Conflicting Flow All	0	0	264	0	576	264
Stage 1	-	-		-	264	
Stage 2	-		-	-	312	-
Critical Hdwy			4.3	-	6.4	6.37
Critical Hdwy Stg 1			4.5		5.4	0.01
Critical Hdwy Stg 2					5.4	-
Follow-up Hdwy			2.38			3.453
Pot Cap-1 Maneuver			1203	_	482	740
Stage 1		_	1203		785	740
Stage 2	-	-		-	747	-
Platoon blocked. %		-		-	141	
	-		1203		480	740
Mov Cap-1 Maneuver	-	-		-		
Mov Cap-2 Maneuver	-	-	-	-	480	-
Stage 1	-	-	-	-	785	-
Stage 2	-	-	-	-	743	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.2	
HCM LOS			0.1		В	
TIOM EGO						
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		719	-	-	1203	-
HCM Lane V/C Ratio		0.029	-	-	0.005	-
HCM Control Delay (s)		10.2	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.1	-	-	0	-
,						

2024 Total AM Peak Hour

(230661) 494 Highway 3, Courtland TIS

	-	•	•	←	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	^			ર્ન	Y		
Traffic Volume (vph)	254	8	2	283	0	0	
Future Volume (vph)	254	8	2	283	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.996						
Flt Protected							
Satd. Flow (prot)	1639	0	0	1810	1900	0	
Flt Permitted							
Satd. Flow (perm)	1639	0	0	1810	1900	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%	
Adj. Flow (vph)	276	9	2	308	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	285	0	0	310	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 19.8%			IC	CU Level of	of Service	èΑ
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	¥	
Traffic Vol, veh/h	254	8	2	283	0	0
Future Vol, veh/h	254	8	2	283	0	0
Conflicting Peds, #/hr	204	0	0	203	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length		None -		None -	0	None -
Veh in Median Storage,		-	-	0	0	-
	# 0		-	0	0	-
Grade, %	-	-		-	-	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mvmt Flow	276	9	2	308	0	0
Major/Minor N	lajor1	ı	Major2		Minor1	
Conflicting Flow All	0	0	285	0	593	281
Stage 1	-	-		-	281	-
Stage 2					312	
Critical Hdwy	-		4.1	-	6.4	6.2
		_	4.1		5.4	
Critical Hdwy Stg 1	-	-		-		-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1289	-	472	763
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	747	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1289	-	471	763
Mov Cap-2 Maneuver	-	-	-	-	471	-
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	746	-
Annroach	EB		WB		ND	
Approach					NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	- LDIX	1289	-
					0.002	-
HCM Cantral Dalay (a)		-			7.8	
HCM Control Delay (s)		0	-	-		0
HCM Lane LOS		Α	-	-	A	Α
HCM 95th %tile Q(veh)		-	-	-	0	-

Paradigm Transportation Solutions Limited

HCM 6th TWSC

3: Access B & Highway 3

Paradigm Transportation Solutions Limited

	-	•	•	•	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			र्स	¥	
Traffic Volume (vph)	254	0	2	285	0	0
Future Volume (vph)	254	0	2	285	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1863	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1863	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	276	0	2	310	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	276	0	0	312	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 19.9%			IC	U Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
	• • • •					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	Y	
Traffic Vol, veh/h	254	0	2	285	0	0
Future Vol, veh/h	254	0	2	285	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	_	0	0	_
Grade. %	. 0			0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	276	0	2	310	0	0
Major/Minor N	lajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	276	0	590	276
Stage 1	-	U	210	-	276	210
		-		-		-
Stage 2	-	-	- 40		314	
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1287	-	470	763
Stage 1	-	-	-	-	771	-
Stage 2		_	_	_	741	-
Platoon blocked. %			_		741	_
	-	-	4007	-	400	700
Mov Cap-1 Maneuver			1287		469	763
Mov Cap-2 Maneuver	-	-	-	-	469	-
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	740	-
A	EB		WD		ND	
Approach			WB		NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minar Lana/Maiar Muma	. ,	UDI 51	FDT	EDD	WDI	WBT
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	
Capacity (veh/h)		-	-	-	1287	-
HCM Lane V/C Ratio		-	-	-	0.002	-
HCM Control Delay (s)		0	-	-	7.8	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)		-	_	-	0	_
σσαι /σαισ α(νοιι)						

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	*	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ની	Y	
Traffic Volume (vph)	431	0	17	380	2	21
Future Volume (vph)	431	0	17	380	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.876	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1658	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	468	0	18	413	2	23
Shared Lane Traffic (%)						
Lane Group Flow (vph)	468	0	0	431	25	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 43.8%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

Paradigm Transportation Solutions Limited

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
•		EDD	MD	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	431	0	17	380	2	21
Future Vol, veh/h	431	0	17	380	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	468	0	18	413	2	23
IVIVIIIL FIUW	400	U	10	413	2	23
Major/Minor I	Major1		Major2	- 1	Minor1	
Conflicting Flow All	0	0	468	0	917	468
Stage 1	_	_	_	_	468	_
Stage 2	-	-		-	449	
Critical Hdwy	_	-	4.41	_	6.4	6.2
Critical Hdwy Stg 1				-	5.4	0.2
Critical Hdwy Stg 2					5.4	
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
					304	599
Pot Cap-1 Maneuver	-	-	958	-		
Stage 1	-	-	-	-	634	-
Stage 2	-	-	-	-	647	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	958	-	297	599
Mov Cap-2 Maneuver	-	-	-	-	297	-
Stage 1	-	-	-	-	634	-
Stage 2	-	-	-	-	631	-
olago 2						
			1475			
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.9	
HCM LOS					В	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
	it I				958	
Capacity (veh/h)		550	-	-		-
HCM Lane V/C Ratio		0.045	-		0.019	-
HCM Control Delay (s)		11.9	-	-	8.8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh))	0.1	-	-	0.1	-

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	€	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	451	2	0	394	4	2
Future Volume (vph)	451	2	0	394	4	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1843	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1843	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	490	2	0	428	4	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	492	0	0	428	6	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 33.9%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Intersection Int Delay, s/veh	0.1					
•						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	451	2	0	394	4	2
Future Vol, veh/h	451	2	0	394	4	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	490	2	0	428	4	2
Major/Minor	Major1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	492	0	919	491
Stage 1	-	-		-	491	
Stage 2		-	-	-	428	
Critical Hdwy	-	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1					5.4	- 0.2
Critical Hdwy Stg 2		_		_	5.4	_
Follow-up Hdwy		-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	1082	_	304	582
Stage 1	-	-	-	-	619	-
Stage 2	-	-	_	-	662	-
Platoon blocked, %		-		-		
Mov Cap-1 Maneuver	-	-	1082	-	304	582
Mov Cap-2 Maneuver	-	-	-	-	304	-
Stage 1	-	-	-	-	619	-
Stage 2		-	-	-	662	-
·						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		15.1	
HCM LOS	U		U		13.1 C	
IICWI LOS					U	
Minor Lane/Major Mvm	nt I	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		362	-	-	1082	-
HCM Lane V/C Ratio		0.018	-	-	-	-
HCM Control Delay (s))	15.1	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)	0.1	-	-	0	-
,	,					

	-	\rightarrow	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	450	3	3	393	1	4
Future Volume (vph)	450	3	3	393	1	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.892	
Flt Protected					0.990	
Satd. Flow (prot)	1861	0	0	1863	1645	0
Flt Permitted					0.990	
Satd. Flow (perm)	1861	0	0	1863	1645	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	489	3	3	427	1	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	492	0	0	430	5	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ration 33.9%			IC	CU Level of	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	TTUL	₩ <u>₩</u>	₩.	ADI
Traffic Vol, veh/h	450	3	3	393	- T	4
	450	3	3	393		4
Future Vol, veh/h		-	-		1	
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	489	3	3	427	1	4
	Major1		/lajor2		Minor1	
Conflicting Flow All	0	0	492	0	924	491
Stage 1	-	-	-	-	491	-
Stage 2	-	-	-	-	433	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	_	_	1071	-	299	578
Stage 1	-	-	-	-	615	-
Stage 2	_	_		_	654	-
Platoon blocked, %		-			001	
Mov Cap-1 Maneuver		_	1071	_	298	578
Mov Cap-1 Maneuver			1071		298	3/0
	-		-	-		-
Stage 1	-	-	-		615	-
Stage 2	-	-	-	-	651	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		12.5	
HCM LOS			0.1		B	
110111 200						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		487	-	-	1071	-
HCM Lane V/C Ratio		0.011	-	-	0.003	-
)	12.5	-	-	8.4	0
HCM Control Delay (s)						
		В	_	-	Α	Α
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)	B 0	-	-	A 0	Α -

HCM 6th TWSC

Appendix F

2029 Background Traffic Operations Reports



Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2029 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	262	1	6	307	1	20
Future Volume (vph)	262	1	6	307	1	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	285	1	7	334	1	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	286	0	0	341	23	0
Sign Control	Free			Free	Stop	
Intersection Cummens						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 31.0%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2029 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

0.5					
0.5					
EBT	EBR	WBL	WBT	NBL	NBR
	LDI	TTUL			ADI
	1	6			20
					20
		-			0
	•	-	-	_	•
					Stop
					None
				-	-
	-				-
-	-	-		-	-
92	92	92	92	92	92
14	0	20	6	0	17
285	1	7	334	1	22
0	0	286	0		286
-	-	-	-	286	-
-	-	-	-	348	-
-	-	4.3	-	6.4	6.37
-	-	-	-	5.4	-
_	_	_	-		_
					3.453
					719
					113
	_				-
	-	-		7 19	
		4400		110	740
					719
-	-	-	-		-
-	-	-	-	767	-
-	-	-	-	714	-
ED		MD		ND	
0		0.2			
				В	
nt I	MRI n1	FRT	FRP	WRI	WBT
it I					WDI -
					-
	10.3	-	-	8.1	0
)					
) ı)	B 0.1	-	-	A 0	A
	262 262 262 2 0 0 Free e, # 0 0 0 92 144 285 Major1 0	262 1 262 1 262 1 0 0 Free Free - None - Non	1 6 262 1 6 6 262 1 6 6 6 6 6 6 6 6 6	262 1 6 307 262 1 6 307 262 1 6 307 0 0 0 0 0 Free Free Free Free - None e,# 0 0 92 92 92 92 14 0 20 6 285 1 7 334 Major1 Major2 0 0 286 0	262 1 6 307 1 262 1 6 307 1 262 1 6 307 1 0 0 0 0 0 0 Free Free Free Free Stop - None - None - O 0 e, # 0 0 0 0 0 0 0 292 92 92 92 14 0 20 6 0 285 1 7 334 1 Major1 Major2 Minor1 0 0 286 0 634 286 348 4.3 - 6.4 5.4 180 - 443 180 - 446 180 - 446 180 - 446 180 - 446 180 - 446 767 1180 - 443 719 1180 - 443 719 719 719 719 714 EB WB NB 0 0.0.2 10.3 B

	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	¥	
Traffic Volume (vph)	280	4	1	312	0	0
Future Volume (vph)	280	4	1	312	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1638	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1638	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	304	4	1	339	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	308	0	0	340	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 20.5%			IC	CU Level	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	TVDL	₩DI	NDL W	NUI
Traffic Vol, veh/h	280	4	1	312	'T'	0
Future Vol. veh/h	280	4	1	312	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	- 100	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	_	0	0	-
Grade. %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	304	4	1	339	0	0
Major/Minor M	ajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	308	0	647	306
Stage 1	-	-	300	-	306	300
Stage 2	-		-		341	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-		4.1		5.4	0.2
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	-		2.2		3.5	3.3
Pot Cap-1 Maneuver	_	_	1264	_	439	739
Stage 1	-		-		751	-
Stage 2	-	_	_	_	725	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	1264	_	439	739
Mov Cap-2 Maneuver	-	-	-	-	439	-
Stage 1	-	-	_	-	751	-
Stage 2	-	-	-	-	724	-
J						
Annroach	EB		WB		NB	
Approach	0		0		0	
HCM Control Delay, s HCM LOS	U		U		A	
FIGNI LOS					А	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
		-	-	-	1264	-
Capacity (veh/h)					0.001	-
Capacity (veh/h) HCM Lane V/C Ratio		-	-			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0	-	-	7.9	0
Capacity (veh/h) HCM Lane V/C Ratio						0 A

HCM 6th TWSC

2: Access A & Highway 3

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	*	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ર્ન	¥	
Traffic Volume (vph)	475	0	18	418	2	20
Future Volume (vph)	475	0	18	418	2	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.876	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1658	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	516	0	20	454	2	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	516	0	0	474	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 46.6%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			ની	Y	
Traffic Vol, veh/h	475	0	18	418	2	20
Future Vol, veh/h	475	0	18	418	2	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	516	0	20	454	2	22
WWW.CT IOW	010	U	20	101	_	
	Major1		Major2		Minor1	=10
Conflicting Flow All	0	0	516	0		516
Stage 1	-	-	-	-	516	-
Stage 2	-	-	-	-	494	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-		-	3.5	3.3
Pot Cap-1 Maneuver	-	-	917	-	268	563
Stage 1	-	-	-	-	603	-
Stage 2	-	-	-	-	617	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	917	-	260	563
Mov Cap-2 Maneuver	-	-	-	-	260	-
Stage 1	-	_	_	-	603	-
Stage 2	-	-	-	-	599	
Stage 2		_	_	_	333	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		12.4	
HCM LOS					В	
Minor Lane/Major Mvm	t 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	. 1	509	EDI	- EDR	917	WDI -
					0.021	
HCM Cartes Dalay (2)		0.047	-			-
HCM Control Delay (s)		12.4	-	-	9	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.1		_	0.1	_

Lanes, Volumes, Timings 2: Access A & Highway 3

Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph)
Ideal Flow (vphpl)
Lane Util. Factor

Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)

Link Speed (k/h)
Link Distance (m)
Travel Time (s)
Peak Hour Factor Heavy Vehicles (%) Adj. Flow (vph)
Shared Lane Traffic (%)
Lane Group Flow (vph)

Sign Control

2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

→	*	•	+	1	<i>></i>
EBT	EBR	WBL	WBT	NBL	NBR
₽.			4	¥	
494	1	0	434	2	1
494	1	0	434	2	1
1900	1900	1900	1900	1900	1900
1.00	1.00	1.00	1.00	1.00	1.00
				0.955	
				0.968	
1845	0	0	1759	1756	0
				0.968	
1845	0	0	1759	1756	0
50			50	50	
158.5			70.1	130.3	
11.4			5.0	9.4	
0.92	0.92	0.92	0.92	0.92	0.92
3%	0%	0%	8%	0%	0%
537	1	0	472	2	1
538	0	0	472	3	0

Intersection Summary

Area Type: Control Type: Unsignalized

Intersection Capacity Utilization 36.1% Analysis Period (min) 15

Free

ICU Level of Service A

Stop Free

HCM 6th TWSC 2: Access A & Highway 3 2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

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Intersection						
Int Delay, s/veh	0.1					
•		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	♣		^	<u>4</u>	À	
Traffic Vol, veh/h	494	1	0	434	2	1
Future Vol, veh/h	494	1	0	434	2	1
Conflicting Peds, #/hr	_ 0	0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	537	1	0	472	2	1
Major/Minor N	/lajor1		Major2		Minor1	
Conflicting Flow All	0	0	538	0	1010	538
Stage 1	U	-	-	U	538	-
Stage 2					472	_
Critical Hdwy			4.1		6.4	6.2
	-	-		-		
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1040	-	268	547
Stage 1	-	-	-	-	589	-
Stage 2	-	-	-	-	632	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1040	-	268	547
Mov Cap-2 Maneuver	-	-	-	-	268	-
Stage 1	-	-	_	-	589	-
Stage 2	-	-	-	-	632	-
Olugo Z					002	
			14/5			
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		16.3	
HCM LOS					С	
	t 1	NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Maior Mymi					1040	-
Minor Lane/Major Mvmt		323				
Capacity (veh/h)		323				_
Capacity (veh/h) HCM Lane V/C Ratio		0.01	-		-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.01 16.3	-	-	0	-
Capacity (veh/h) HCM Lane V/C Ratio		0.01	-			-

Appendix G

2029 Total Traffic Operations Reports

2029 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ર્ન	¥	
Traffic Volume (vph)	266	1	6	307	1	20
Future Volume (vph)	266	1	6	307	1	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	289	1	7	334	1	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	290	0	0	341	23	0
Sign Control	Free			Free	Stop	
Intersection Summany						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 31.0%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

2029 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	W	
Traffic Vol. veh/h	266	1	6	307	1	20
Future Vol. veh/h	266	1	6	307	1	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -	None	-		Stop -	None
Storage Length		None -		None -	0	None
Veh in Median Storage		-	-	0	0	-
Grade. %	0,#	-	-	0	0	-
	•			-	•	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mvmt Flow	289	1	7	334	1	22
Major/Minor N	Major1	ı	Major2		Minor1	
Conflicting Flow All	0	0	290	0	638	290
Stage 1	-	_		_	290	-
Stage 2		-	-	-	348	
Critical Hdwy	_	-	4.3	_	6.4	6.37
Critical Hdwy Stg 1	-	-	7.0	-	5.4	0.01
Critical Hdwy Stg 2		-			5.4	
Follow-up Hdwy			2.38		3.5	3.453
Pot Cap-1 Maneuver		-	1176	-	444	715
	-				764	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	719	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1176	-	441	715
Mov Cap-2 Maneuver	-	-	-	-	441	-
Stage 1	-	-	-	-	764	-
Stage 2	-	-	-	-	714	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.4	
HCM LOS	U		0.2		10.4 B	
I IOW LOS					0	
		NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Major Mvm	it I	VDLIII				
	nt I	694	-	-	1176	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	nt I				1176 0.006	-
Capacity (veh/h) HCM Lane V/C Ratio		694	-			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		694 0.033 10.4	-	-	0.006	- 0
Capacity (veh/h) HCM Lane V/C Ratio		694 0.033	-	-	0.006	-

Lane Group EBT EBR WBL WBT NBL NBR Lane Configurations ♣ ♣ ♣ ✔ ✔ ✔ ✓
Traffic Volume (vph) 280 8 2 312 0 0
Traffic Volume (vph) 280 8 2 312 0 0
Firture Values (inh) 000 0 0 240 0 0
Future Volume (vph) 280 8 2 312 0 0
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00
Frt 0.996
FIt Protected
Satd. Flow (prot) 1638 0 0 1810 1900 0
Flt Permitted
Satd. Flow (perm) 1638 0 0 1810 1900 0
Link Speed (k/h) 50 50
Link Distance (m) 158.5 70.1 130.3
Travel Time (s) 11.4 5.0 9.4
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92
Heavy Vehicles (%) 16% 0% 0% 5% 0% 0%
Adj. Flow (vph) 304 9 2 339 0 0
Shared Lane Traffic (%)
Lane Group Flow (vph) 313 0 0 341 0 0
Sign Control Free Free Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 21.3% ICU Level of Service A
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	0.1					
	EBT	EDD	WDi	MDT	ND	NDD
Movement		EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	•	^	4	À	^
Traffic Vol, veh/h	280	8	2	312	0	0
Future Vol, veh/h	280	8	2	312	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	304	9	2	339	0	0
M-i/Mi	4-14	_	4-:0	_	M:4	
	Major1		Major2		Minor1	200
Conflicting Flow All	0	0	313	0	652	309
Stage 1	-	-	-	-	309	-
Stage 2	-	-	-	-	343	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1259	-	436	736
Stage 1	-	-	-	-	749	-
Stage 2	-	-	-	-	723	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1259	-	435	736
Mov Cap-2 Maneuver	-	-	-	-	435	-
Stage 1	-	-	-	-	749	-
Stage 2	-	-	-	-	722	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		NDLIII	EDI	EDK -	1259	WDI
HCM Lane V/C Ratio					0.002	
		0	-	-	7.9	0
HCM Control Delay (s) HCM Lane LOS		A	-		7.9 A	A
		А	-	-		
HCM 95th %tile Q(veh)		-	-	-	0	-

	-	\rightarrow	•	•	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ _e			ની	¥	
Traffic Volume (vph)	280	0	2	314	0	0
Future Volume (vph)	280	0	2	314	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1863	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1863	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	304	0	2	341	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	304	0	0	343	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 21.5%			IC	U Level o	of Service
Analysis Period (min) 15						

Intersection Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/m Sign Control RT Channelized Storage Length Voland In Median Storag Grade, % Peak Hour Factor	0 EBT 280 280 280	EBR 0	WBL	WBT	NDI	
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280	0	WBL		NDI	
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280	0	VVDL			NBR
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280				NBL	NDK
Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280		0	4	À	٥
Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor			2	314	0	0
Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	r 0	0	2	314	0	0
RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor		0	0	0	0	0
Storage Length Veh in Median Storag Grade, % Peak Hour Factor	Free	Free	Free	Free	Stop	Stop
Veh in Median Storag Grade, % Peak Hour Factor	-	None	-	None	-	None
Grade, % Peak Hour Factor	-	-	-	-	0	-
Peak Hour Factor	qe,# 0	-	-	0	0	-
	0	-	-	0	0	-
	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	304	0	2	341	0	0
INIVITIE I TOW	304	U		J 4 I	U	U
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	304	0	649	304
Stage 1	-	-	_	-	304	-
Stage 2		-	-	-	345	-
Critical Hdwy	-	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	-	-	-1.12	-	5.42	0.22
Critical Hdwy Stg 1					5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
		-				
Pot Cap-1 Maneuver		-	1257	-	434	736
Stage 1	-	-	-	-	748	-
Stage 2	-	-	-	-	717	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	r -	-	1257	-	433	736
Mov Cap-2 Maneuver	r -	-	-	-	433	-
Stage 1	_	-	_	-	748	_
Stage 2		-	-	-	716	-
otago 2						
Approach	EB		WB		NB	
HCM Control Delay, s	s 0		0		0	
HCM LOS					Α	
Minor Lane/Major Mv	mt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-			1257	
HCM Lane V/C Ratio		-				
HCM Control Delay (s	S)	0	-	-	7.9	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(ve	h)	-	-	-	0	-

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f a			ર્ન	¥	
Traffic Volume (vph)	476	0	19	420	2	23
Future Volume (vph)	476	0	19	420	2	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.875	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	517	0	21	457	2	25
Shared Lane Traffic (%)						
Lane Group Flow (vph)	517	0	0	478	27	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized
Intersection Capacity Utilization 47.5%
Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	¥	
Traffic Vol, veh/h	476	0	19	420	2	23
Future Vol, veh/h	476	0	19	420	2	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	- 100	None	-	None
Storage Length	-	-		-	0	-
Veh in Median Storage,		_	_	0	0	_
Grade, %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	517	0	21	457	2	25
IVIVITE FIOW	517	U	21	457	2	25
	/lajor1		Major2		Minor1	
Conflicting Flow All	0	0	517	0	1016	517
Stage 1	-	-	-	-	517	-
Stage 2	-	-	-	-	499	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy		-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver		-	916	-	266	562
Stage 1		_	-	_	603	-
Stage 2	_	_	_	_	614	-
Platoon blocked. %	-	-	_	-	014	_
Mov Cap-1 Maneuver	-	-	916	-	258	562
Mov Cap-1 Maneuver			910	-	258	502
	-	-	-			
Stage 1	-	-	-	-	603	-
Stage 2	-	-	-	-	595	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		12.4	
HCM LOS	Ū		0.1		В	
110111 200						
Minor Lane/Major Mvmt	t 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		514	-	-	916	-
HCM Lane V/C Ratio		0.053	-	-	0.023	-
HCM Control Delay (s)		12.4	-	-	9	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.2	-	-	0.1	-

2029 Total PM Peak Hour

(230661) 494 Highway 3, Courtland TIS

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 2: Access A & Highway 3 2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	→	•	•	•	1	_
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	î»			ર્ન	Y	
Traffic Volume (vph)	497	2	0	435	4	2
Future Volume (vph)	497	2	0	435	4	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	540	2	0	473	4	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	542	0	0	473	6	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 36.3%			IC	CU Level of	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	497	2	0	435	4	2
Future Vol. veh/h	497	2	0	435	4	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	,# 0	_	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	540	2	0	473	4	2
	0.0	_			•	_
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	542	0	1014	541
Stage 1	-	-	-	-	541	-
Stage 2	-	-	-	-	473	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	1037	-	267	545
Stage 1	-	-	-	-	588	-
Stage 2	-	-	_	-	631	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	_	1037	_	267	545
Mov Cap-2 Maneuver		_	-		267	-
Stage 1	-	_	_	_	588	_
Stage 2	-		-	-	631	
Stage 2					001	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		16.4	
HCM LOS					С	
Miner Lene/Major Major		IDI n4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvm	t P	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		322	-	-	1037	-
HCM Lane V/C Ratio		0.02	-	-	-	-
HCM Control Delay (s)		16.4	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	_		0	_

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	•	1	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			ર્ન	¥		
Traffic Volume (vph)	497	3	3	434	1	4	
Future Volume (vph)	497	3	3	434	1	4	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.999				0.892		
Flt Protected					0.990		
Satd. Flow (prot)	1861	0	0	1863	1645	0	
Flt Permitted					0.990		
Satd. Flow (perm)	1861	0	0	1863	1645	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	70.1			184.9	93.4		
Travel Time (s)	5.0			13.3	6.7		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	540	3	3	472	1	4	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	543	0	0	475	5	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 36.3%			IC	CU Level of	of Service A	
Analysis Period (min) 15							

HCM 6th TWSC 3: Access B & Highway 3

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	¥	
Traffic Vol, veh/h	497	3	3	434	1	4
Future Vol. veh/h	497	3	3	434	1	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	None
Storage Length	-	-		-	0	-
Veh in Median Storage	e.# 0	-	-	0	0	_
Grade, %	0, "		-	0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	540	3	3	472	1	4
IVIVIIIL FIOW	540	3	ა	412	- 1	4
Major/Minor	Major1	- 1	Major2	- 1	Minor1	
Conflicting Flow All	0	0	543	0	1020	542
Stage 1	-	-	-	-	542	-
Stage 2	-	-	-	-	478	-
Critical Hdwy	-	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	_	-	_	-	5.42	_
Follow-up Hdwy	-	-	2.218	-	3.518	3 318
Pot Cap-1 Maneuver	_	_	1026	-	262	540
Stage 1		-	1020	-	583	J+0
Stage 2	_	_		_	624	
Platoon blocked. %			_		024	_
Mov Cap-1 Maneuver			1026	-	261	540
		-	1020		261	540
Mov Cap-2 Maneuver	-	-	-	-		-
Stage 1	-	-	-	-	583	-
Stage 2	-	-	-	-	622	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		13.2	
HCM LOS	U		0.1		10.2 B	
TIOW LOO						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		445	-	-	1026	-
HCM Lane V/C Ratio		0.012	-	-	0.003	-
HCM Control Delay (s)	13.2	-	-	8.5	0
HCM Lane LOS	,	В	-	-	A	A
HCM 95th %tile Q(veh	1)	0	_	_	0	- '-
0041 70410 04(101	.,					

Appendix H

2034 Background Traffic Operations Reports

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2034 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	•	7	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	290	1	6	339	1	22
Future Volume (vph)	290	1	6	339	1	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.870	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1418	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1418	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	315	1	7	368	1	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	316	0	0	375	25	0
Sign Control	Free			Free	Stop	
Intersection Commons						

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 32.6%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2034 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			4	¥	
Traffic Vol, veh/h	290	1	6	339	1	22
Future Vol, veh/h	290	1	6	339	1	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		None
Storage Length		-		-	0	-
Veh in Median Storage,	# 0	-	_	0	0	-
Grade, %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	315	1	7	368	1	24
IVIVIIIL FIOW	313	- 1	1	300	- 1	24
Major/Minor N	/lajor1	1	Major2	- 1	Minor1	
Conflicting Flow All	0	0	316	0	698	316
Stage 1	-	-	-	-	316	-
Stage 2	-	-	-	-	382	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-	3.5	3.453
Pot Cap-1 Maneuver	-	_	1149	_	410	691
Stage 1	-	-	-	-	744	-
Stage 2	-	_	-	_	694	-
Platoon blocked. %	-	-		-		
Mov Cap-1 Maneuver	-	_	1149	_	407	691
Mov Cap-2 Maneuver	-	-	-	-	407	-
Stage 1	_	_	_	_	744	_
Stage 2	-	-	-	-	688	
Stage 2					000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.6	
HCM LOS					В	
Minor Lane/Major Mvmt	. ,	NBLn1	EBT	EBR	WBL	WBT
	l I	671	EDI		1149	WDI
Capacity (veh/h)				-		
		0.037	-	-	0.006	-
HCM Lane V/C Ratio						0
HCM Control Delay (s)		10.6	-			,
		10.6 B 0.1	-	-	A 0	A

	-	\rightarrow	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	310	4	1	344	0	0
Future Volume (vph)	310	4	1	344	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1637	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1637	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	337	4	1	374	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	341	0	0	375	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	zation 22.2%			IC	CU Level	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	LDI	WDL	₩DI	NDL W	NOIL
Traffic Vol, veh/h	310	4	1	344	'T'	0
Future Vol. veh/h	310	4	1	344	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length		-		-	0	-
Veh in Median Storage,				0	0	
Grade. %	0			0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	92	92	92 5	92	92
	337	4	1	374	0	0
Mvmt Flow	331	4		3/4	U	U
Major/Minor Ma	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	341	0	715	339
Stage 1	-	-	-	-	339	-
Stage 2	-	-	-		376	
Critical Hdwy	_	_	4.1	-	6.4	6.2
Critical Hdwy Stg 1		_		-	5.4	- 0.2
Critical Hdwy Stg 2	_	_	-	-	5.4	-
Follow-up Hdwy			2.2	-	3.5	3.3
Pot Cap-1 Maneuver	_	_	1229	-	400	708
Stage 1	-	-	1225		726	- 100
Stage 2	_	_	_	_	699	_
Platoon blocked, %				-	000	
Mov Cap-1 Maneuver	-	-	1229	-	400	708
Mov Cap-1 Maneuver	-	-	1229	-	400	700
Stage 1	-	_	_	-	726	-
		-	-		698	-
Stage 2	-	-	-	-	698	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		0	
HCM LOS					Ā	
					NA/EX	14/0=
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1229	-
HCM Lane V/C Ratio		-	-		0.001	-
HCM Control Delay (s)		0	-	-	7.9	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)		-	-	-	0	-

HCM 6th TWSC

2: Access A & Highway 3

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2034 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^			4	Y	
Traffic Volume (vph)	525	0	20	461	2	22
Future Volume (vph)	525	0	20	461	2	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.875	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	571	0	22	501	2	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	571	0	0	523	26	0
Sign Control	Free			Free	Stop	
Internaction Cummen						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 50.5%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2034 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
		EDD.	WDI	MOT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- ♣	^	00	ની	M	00
Traffic Vol, veh/h	525	0	20	461	2	22
Future Vol, veh/h	525	0	20	461	2	22
Conflicting Peds, #/hr	0	0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mvmt Flow	571	0	22	501	2	24
Major/Minor N	Major1		Major2		Minor1	
Conflicting Flow All	0	0	571	0		571
Stage 1	-	-	-	-	571	-
Stage 2		-			545	
Critical Hdwy	_	_	4.41	_	6.4	6.2
Critical Hdwy Stg 1	-	-	7.71	-	5.4	0.2
Critical Hdwy Stg 2	-			-	5.4	
Follow-up Hdwy	-				3.5	3.3
Pot Cap-1 Maneuver	-	-	873	-		524
Stage 1	-	-	0/3		569	524
				-	585	
Stage 2	-	-	-		202	-
Platoon blocked, %	-	-	070	-	004	F0.4
Mov Cap-1 Maneuver	-	-	873	-	224	524
Mov Cap-2 Maneuver	-	-	-	-	224	-
Stage 1	-	-	-	-	569	-
Stage 2	-	-	-	-	565	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		13.1	
HCM LOS	-				В	
Miner Lene/Meier Marie		UDI 54	EDT	EDD	WBL	WDT
Minor Lane/Major Mvm	it l	VBLn1	EBT	EBR		WBT
Capacity (veh/h)		471	-	-	0.0	-
HCM Lane V/C Ratio		0.055	-		0.025	-
HCM Control Delay (s)		13.1	-	-	9.2	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.2	_		0.1	-

	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ર્ન	¥	
Traffic Volume (vph)	546	1	0	479	2	1
Future Volume (vph)	546	1	0	479	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	593	1	0	521	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	594	0	0	521	3	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliz	ation 38.8%			IC	CU Level	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	546	1	0	479	2	1
Future Vol, veh/h	546	1	0	479	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	593	1	0	521	2	1
M-:/M:	4-14		M-:0		Minor1	
	Major1 0	0	Major2 594	0		594
Conflicting Flow All			594	-	1115	594
Stage 1	-	-			594	
Stage 2	-	-	- 4.4	-	521	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	992	-	232	509
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	600	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	992	-	232	509
Mov Cap-2 Maneuver	-	-	-	-	232	-
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	600	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		17.9	
HCM LOS	U		U		C	
TIOM EGO					Ŭ	
Minor Lane/Major Mvmt	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		283	-	-	992	-
HCM Lane V/C Ratio		0.012	-	-	-	-
HCM Control Delay (s)		17.9	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
		C 0	-	-	A 0	-

HCM 6th TWSC

2: Access A & Highway 3

Appendix I

2034 Total Traffic Operations Reports

2034 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Lanes, Volumes, Timings
1: Goshen Road & Highway 3

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	294	1	6	339	1	22
Future Volume (vph)	294	1	6	339	1	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.870	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1418	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1418	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	320	1	7	368	1	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	321	0	0	375	25	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 32.6%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

2034 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
	•••	EDD	WD	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ની	¥	
Traffic Vol, veh/h	294	1	6	339	1	22
Future Vol, veh/h	294	1	6	339	1	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	320	1	7	368	1	24
WYTHE I IOW	020		- 1	000		4
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	321	0	703	321
Stage 1	-	-	-	-	321	-
Stage 2	-	-	-	-	382	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	_	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-	3.5	3.453
Pot Cap-1 Maneuver	-	_	1144	_	407	686
Stage 1		-	-		740	-
Stage 2	-	_	-	-	694	-
Platoon blocked. %					ООТ	
Mov Cap-1 Maneuver		-	1144	-	404	686
Mov Cap-1 Maneuver		-			404	000
	-	-	-			
Stage 1	-	-	-	-	740	-
Stage 2	-	-	-	-	688	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.6	
HCM LOS	- 0		0.1		В	
200					٥	
Minor Lane/Major Mvm	nt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		666	-	-	1144	-
HCM Lane V/C Ratio		0.038	-	-	0.006	-
HCM Control Delay (s))	10.6	-	-	8.2	0
HCM Lane LOS		В	-		A	A
HCM 95th %tile Q(veh)	0.1	-	-	0	-
TOW COLLY TOUR Q(VOI)	,	0.1			U	

	-	•	•	←	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	î»			ર્ન	¥		
Traffic Volume (vph)	310	8	2	344	0	0	
Future Volume (vph)	310	8	2	344	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.996						
Flt Protected							
Satd. Flow (prot)	1637	0	0	1810	1900	0	
Flt Permitted							
Satd. Flow (perm)	1637	0	0	1810	1900	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%	
Adj. Flow (vph)	337	9	2	374	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	346	0	0	376	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 23.0%			IC	CU Level of	of Service	A (
Analysis Period (min) 15							

Novement Set
Movement
Movement
Care Configurations Care Color Color
fraffic Vol, veh/h 310 8 2 344 0 0 uture Vol, veh/h 310 8 2 344 0 0 conflicting Peds, #hr 0
Future Vol, veh/h Conflicting Peds, #/hr Conflicting Flow All Conflicti
Conflicting Peds, #hr O O O O O O O O O
Sign Control Free Stop Free None Free None Free None Free None Free None Free None Stop None Stop None Stop None Stop None No
None
Storage Length
Veh in Median Storage, # 0 - 0 0 - Grade, % 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0
Grade, % 0 0 0 0 - Peak Hour Factor 92 92 92 92 92 92 Peavy Vehicles, % 16 0 0 5 0 0 Adjor/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 346 0 720 342 Stage 1 342 - Stage 2 378 - Critical Hdwy Stg 1 5, 4 - Critical Hdwy Stg 1 5, 4 - Critical Hdwy Stg 2 5, 4 - Critical Hdwy Stg 2 5, 4 - Critical Hdwy Stg 2 6, 697 - Critical Hdwy Stg 2 7, 24 - Stage 1 1224 - 398 705 Stage 1 724 - Stage 1 724 - Stage 2 6697 - Critical Hdwy Stg 2 6, 697 - Critical Hdwy Stg 2 1224 - 397 705 Stage 1 724 - 397 705 Approach EB WB NB HCM Control Delay, s 0 0 0 0 GCM Control Delay, s 0 0 0 0 GCM Control Delay (s) 0 - 7, 724 - 1224 - Critical Hdwy Stg 2 1224 - 1224
Peak Hour Factor 92 92 92 92 92 92 92 Peleavy Vehicles, % 16 0 0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Heavy Vehicles, % 16
Major/Minor Major1 Major2 Minor1
Major/Minor Major1 Major2 Minor1
Conflicting Flow All
Conflicting Flow All
Stage 1
Stage 2
Critical Hdwy Critical Hdwy Stg 1 4.1 - 6.4 6.2 Critical Hdwy Stg 1 5.4 5.4 Critical Hdwy Stg 2 5.4 6.9 Critical Hdwy Stg 2
Dritical Hdwy Stg 1
District Control Con
Follow-up Hdwy - 2.2 - 3.5 3.3 Pot Cap-1 Maneuver - 1224 - 398 705 Stage 1 697 - 697 Platoon blocked, % 1224 - 397 705 Mov Cap-1 Maneuver - 1224 - 397 705 Mov Cap-2 Maneuver - 1224 - 397 705 Stage 1 724 - 397 705 Stage 2 696 - 696 - 696 Approach EB WB NB HCM Control Delay, s 0 0 0 0 HCM LOS ABLE BER WBL WBT Capacity (veh/h) 1224 -
Dot Cap-1 Maneuver
Stage 1
Stage 2
Platoon blocked, %
Mov Cap-1 Maneuver - 1224 - 397 705 Mov Cap-2 Maneuver - - - 397 - Stage 1 - - - 724 - Stage 2 - - - 696 - Approach EB WB NB NB ICM Control Delay, s 0 0 0 0 ICM LOS A A WBT WBT Dapacity (veh/h) - - 1224 - ICM Lane V/C Ratio - - - 0.002 - ICM Control Delay (s) 0 - - 7.9 0 ICM Lane LOS A - - A A
Mov Cap-2 Maneuver
Stage 1 - - - 724 - Stage 2 - - - 696 - Approach EB WB NB HCM Control Delay, s 0 0 0 0 4CM LOS A A - - 0 0 Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT WBT 2apacity (veh/h) - - 1224 - 1CM Lane V/C Ratio - - 0.002 - 1CM Lane LOS A - - A A
Stage 2
Approach EB WB NB
CM Control Delay, s
CM Control Delay, s
CM Control Delay, s
A A A A A A A A A A
Capacity (veh/h) - - 1224 - HCM Lane V/C Ratio - - 0.002 - HCM Control Delay (s) 0 - 7.9 0 HCM Lane LOS A - A A
Capacity (veh/h) - - 1224 - HCM Lane V/C Ratio - - 0.002 - HCM Control Delay (s) 0 - 7.9 0 HCM Lane LOS A - A A
Capacity (veh/h) - - 1224 - HCM Lane V/C Ratio - - 0.002 - HCM Control Delay (s) 0 - 7.9 0 HCM Lane LOS A - A A
HCM Lane V/C Ratio 0.002
HCM Control Delay (s) 0 - 7.9 0 HCM Lane LOS A - A A
HCM Lane LOS A A A
HCM 95th %tile Q(ven) U -

Lane Group

(230661) 494 Highway 3, Courtland TIS

Page 5

Laric Group	LDI	LDIX	VVDL	*****	INDL	INDIX	
Lane Configurations	ĵ»			ર્ન	¥		
Traffic Volume (vph)	310	0	2	347	0	0	
Future Volume (vph)	310	0	2	347	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1863	0	0	1863	1863	0	
Flt Permitted							
Satd. Flow (perm)	1863	0	0	1863	1863	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	70.1			184.9	93.4		
Travel Time (s)	5.0			13.3	6.7		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	337	0	2	377	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	337	0	0	379	0	0	
Sign Control	Free			Free	Ston		

Intersection Summary		
Area Type: Other		
Control Type: Unsignalized		
Intersection Capacity Utilization 23.2%	ICU Level of Service A	
Analysis Period (min) 15		

				_		
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	LDI	WOL	₩DI	NDL M	וטוו
		٥	0			٥
Traffic Vol, veh/h	310	0	2	347	0	0
Future Vol, veh/h	310	0	2	347	0	0
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storag	ge,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	337	0	2	377	0	0
WWITCHIOW	001	U		511	U	U
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	337	0	718	337
Stage 1	-	-	-	-	337	-
Stage 2	-	-	-	-		-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1222	-	396	705
Stage 1	-	-	-	-	723	-
Stage 2	_	-	_	-	691	_
Platoon blocked, %					001	
Mov Cap-1 Maneuver			1222	-	395	705
Mov Cap-1 Maneuve			1222		395	705
		-				
Stage 1	-	-	-	-	723	-
Stage 2	-	-	-	-	690	-
Approach	EB		WB		NB	
HCM Control Delay, s	s 0		0		0	
HCM LOS	,		Ū		A	
TIOW EOO						
Minor Lane/Major Mv	mt i	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1222	-
		-	-	-	0.002	-
HCM Lane V/C Ratio				-	8	0
HCM Lane V/C Ratio HCM Control Delay (s	s)	0	-	-	U	
	s)	0 A	-		A	A
HCM Control Delay (s						

HCM 6th TWSC

3: Access B & Highway 3

2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	*	1	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	526	0	21	463	2	25
Future Volume (vph)	526	0	21	463	2	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.874	
Flt Protected				0.998	0.997	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.997	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	572	0	23	503	2	27
Shared Lane Traffic (%)						
Lane Group Flow (vph)	572	0	0	526	29	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 51.4%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
**	CDT	EDD	WDI	W/DT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	^	04	4	¥	٥٢
Traffic Vol, veh/h	526	0	21	463	2	25
Future Vol, veh/h	526	0	21	463	2	25
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mvmt Flow	572	0	23	503	2	27
N. 1. (N. 1)						
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	572	0	1121	572
Stage 1	-	-	-	-	572	-
Stage 2	-	-	-	-	549	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	872	_	230	523
Stage 1	-	-	-	-	569	-
Stage 2	_	-	_	-	583	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver		-	872	_	221	523
Mov Cap-1 Maneuver		-	- 012	-	221	- 020
Stage 1					569	
		-	-		561	-
Stage 2	-	-	-	-	1 00	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		13.1	
HCM LOS	U		0.1		В	
TIOW EOO						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		475	-	-	872	-
HCM Lane V/C Ratio		0.062	-	-	0.026	-
HCM Control Delay (s))	13.1	-	-	9.2	0
HCM Lane LOS		В	-	-	Α	A
HCM 95th %tile Q(veh	۸.	0.2	_	-	0.1	

	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	Y	
Traffic Volume (vph)	549	2	0	480	4	2
Future Volume (vph)	549	2	0	480	4	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	597	2	0	522	4	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	599	0	0	522	6	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 39.0%			IC	CU Level of	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>	LDI	TTDL	4	¥	HUIN
Traffic Vol, veh/h	549	2	0	480	4	2
Future Vol, veh/h	549	2	0	480	4	2
		0	0	400	0	0
Conflicting Peds, #/hr	0	-		-	_	_
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	597	2	0	522	4	2
	00.	_	•	022	•	_
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	599	0	1120	598
Stage 1	-	-	-	-	598	-
Stage 2	-	-	-	-	522	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	_	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	_	_	988	-	231	506
Stage 1	-	-	-	-	553	-
Stage 2					599	-
		-	-		299	-
Platoon blocked, %	-	-	000	-	004	=00
Mov Cap-1 Maneuver	-	-	988	-	231	506
Mov Cap-2 Maneuver	-	-	-	-	231	-
Stage 1	-	-	-	-	553	-
Stage 2	-	-	-	-	599	-
, and the second						
			MD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		18.1	
HCM LOS					С	
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
					988	
Capacity (veh/h)		282	-	-		-
HCM Lane V/C Ratio		0.023	-	-	-	-
HCM Control Delay (s)		18.1	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh))	0.1	-	-	0	-

	-	•	•	•	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4î			ર્ન	¥		
Traffic Volume (vph)	548	3	3	479	1	4	
Future Volume (vph)	548	3	3	479	1	4	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.999				0.892		
Flt Protected					0.990		
Satd. Flow (prot)	1861	0	0	1863	1645	0	
Flt Permitted					0.990		
Satd. Flow (perm)	1861	0	0	1863	1645	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	70.1			184.9	93.4		
Travel Time (s)	5.0			13.3	6.7		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	596	3	3	521	1	4	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	599	0	0	524	5	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	d						
Intersection Capacity Utiliz	zation 39.0%			IC	CU Level of	of Service A	Α
Analysis Period (min) 15							

Intersection Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	548	EBR	WBL			
Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	EBT 548	EBR	WBL			
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	1→ 548	EBR	WBL			
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	548			WBT	NBL	NBR
Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %				ની	¥	
Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	E 40	3	3	479	1	4
Sign Control RT Channelized Storage Length Veh in Median Stora Grade, %	548	3	3	479	1	4
RT Channelized Storage Length Veh in Median Stora Grade, %	ır 0	0	0	0	0	0
Storage Length Veh in Median Stora Grade, %	Free	Free	Free	Free	Stop	Stop
Veh in Median Stora Grade, %	-	None	-	None	-	None
Grade, %	-	-	-	-	0	-
Grade, %	ae.# 0	-	-	0	0	-
	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	596	3	3	521	1	4
IVIVIIIL I IOW	330	J	J	JZI	- 1	-
Major/Minor	Major1	N	Major2		Minor1	
Conflicting Flow All	0	0	599	0	1125	598
Stage 1	-	-	-	-	598	-
Stage 2	-	-	-	-	527	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	r -	-	978	-	227	502
Stage 1		-	-	-	549	-
Stage 2	-	-	_	-	592	_
Platoon blocked. %		-		-		
Mov Cap-1 Maneuve	er -	-	978	-	226	502
Mov Cap-2 Maneuve		_	-		226	-
Stage 1	JI -	_		_	549	_
		-			590	
Stage 2	-		_	-	590	-
Approach	EB		WB		NB	
HCM Control Delay,	s 0		0.1		14.1	
HCM LOS					В	
		NDL 4	EDT	EDD	WDI	MOT
Minor Lane/Major My	/mt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		403	-	-	978	-
HCM Lane V/C Ratio		0.013	-		0.003	-
HCM Control Delay ((s)	14.1	-	-	8.7	0
		В	-	-	Α	Α
HCM Lane LOS		0	_	_	0	-
HCM 95th %tile Q(ve	eh)	U			·	

HCM 6th TWSC



494 Highway 3, Courtland, Norfolk County Transportation Impact Study

Paradigm Transportation Solutions Limited

2024-10 230661





Project Summary



Project Number:

230661

Date and Version:

2024**-**10 1.0.0

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494 Highway 3, Courtland, Norfolk County Transportation Impact Study

<< Original Signed By >>

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Executive Summary

Content

Paradigm Transportation Solutions Limited (Paradigm) has been retained to conduct this Transportation Impact Study (TIS) for the proposed changes to the property located at 494 Highway 3 in the community of Courtland, Norfolk County. The changes will provide for expanding an existing Equipment Warehouse and accommodate a new Truck Yard for two different businesses.

This TIS includes description and analysis of existing road and traffic conditions, a description of the proposed development, analysis of future traffic conditions, and assessment of the existing access arrangement to accommodate the proposed changes.

Proposed Site Changes

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

TIS Scope

The scope of the Transportation Impact Study for the proposed development includes:

Study Area Intersections:



- Highway 3 and Goshen Road; and
- Two site access intersections on Highway 3.
- Analysis Periods: Weekday AM and PM peak hours.
- ▶ **Traffic Conditions:** Existing (2023), expansion completion (2024), five years after completion (2029), and ten years after completion (2034).

Conclusions

Based on the investigations carried out, it is concluded that:

- Existing Traffic Conditions: The intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are operating at satisfactory levels of service (LOS A/B).
- ▶ **Site Traffic Volumes:** The existing site driveway volumes indicate that 5 AM peak hour trips and 4 PM peak hour generated by the current site operations.

With the proposed changes, the site traffic volumes are projected to increase to 7 AM peak hour trips and 15 PM peak hour trips.

Based on the current and future site traffic volumes, the site could be considered to be a low trip generator with minimal impact on driveway and road traffic operations.

- ▶ **Background Traffic Conditions:** The two intersections are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 background traffic conditions.
- ▶ Total Traffic Conditions: The intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours under 2024, 2029, and 2034 total traffic conditions, as under existing conditions.
- ▶ **Site Accesses:** The Westerly and Easterly Site Access intersections on Highway 3 are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 total traffic conditions.

Both access locations satisfy the sight distance requirements, and do not require an auxiliary westbound left-turn lane on Highway 3 to accommodate inbound left-turning traffic.

Recommendations

Based on the findings and conclusions of this study, it is recommended that the Site Changes with two driveways for the property at 494 Highway 3, Courtland be considered for approval as proposed.

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Append	dices							
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1 Introduction

1.1 Overview

Paradigm Transportation Solutions Limited (Paradigm) has been retained to conduct this Transportation Impact Study (TIS) for the proposed changes to the property located at 494 Highway 3 in the community of Courtland, Norfolk County. The changes will provide for expanding an existing Equipment Warehouse and accommodate a new Truck Yard for two different businesses.

Figure 1.1 details the subject development location.

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

1.2 Purpose and Scope

The purpose of this report is to assess the location and operation of the existing site driveways to accommodate the proposed site changes. The scope of the study, shared with Norfolk County and Ministry staff via e-mail in November 2023, includes:

 assessment of the current road, traffic and site conditions within the study area;



- estimates of background traffic growth for the year of the proposed site changes (2024), five years after changes (2029), and ten years after changes (2034);
- estimates of additional traffic generated by the proposed site changes;
- analyses of the impact of the future traffic on the surrounding road network, including the following study area intersections:
 - Highway 3 and Goshen Road; and
 - Two access intersections on Highway 3.
- Operational and safety assessment of the two site driveways in accommodating the proposed site changes.

Appendix A contains the pre-study consultation material.

This study has been prepared in accordance with the requirements detailed by the MTO TIS Guidelines¹ and Norfolk County TIS Guidelines².

Norfolk County Integrated Sustainable Master Plan (ISMP), Appendix J: TIS Guidelines, September 2016.



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General Guidelines for the Preparation of Traffic Impact Studies, Ministry of Transportation Ontario, March 2023.





Location of Subject Site

2 Existing Conditions

2.1 Existing Roadways

The main roadways near the subject development considered in assessing the traffic impacts of the development include:

- ► **Highway 3** is an east-west provincial highway³ with a two-lane cross section. The posted speed limit is 80 km/h.
- ▶ **Goshen Road** is a north-south local road with a two-lane cross section. The posted speed limit is 60 km/h.

Side-street stop control is provided at the intersection of Highway 3 and Goshen Road.

Figure 2.1 illustrates the traffic control and lane configuration at the intersections of Highway 3 and Goshen Road.

2.2 Existing Driveways on Highway 3

Highway 3 in the study area is identified as a 2B Arterial Road in MTO's Access Management Classification System.⁴

2B Arterial roads typically allows consideration of access connections based on spacing, density (number of accesses over a kilometre), property frontage, and safety requirements.

In the section of Highway 3 between Goshen Road and Plowman's Line, a distance of 625 metres, there are seven driveways on the north side of the highway and six on the south.

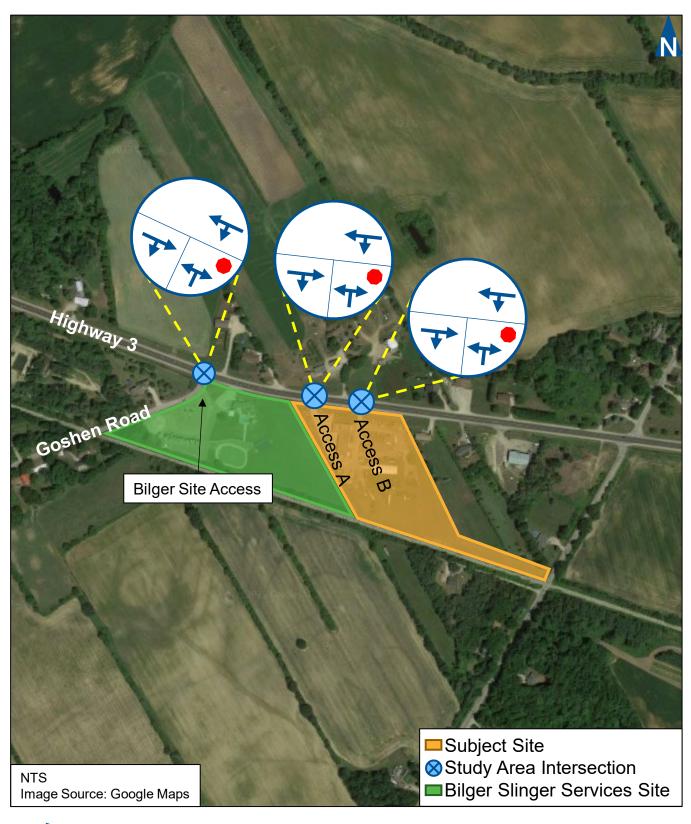
The driveways on the south side include the two driveways to the subject property. The property frontage on Highway 3 is 160 metres, and the two driveways are 70 metres apart, exceeding the minimum separation requirement of 30 metres. The westerly and easterly site driveways are located at 165 metres and 235 metres west of Goshen Road, respectively. The two driveways are under stop sign control as shown in **Figure 2.1**.

Ministry of Transportation. Highway Corridor Management Manual. April 2022.



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Norfolk County Official Plan, Schedule E-4: Delhi Transportation, 5 October 2018.





Existing Lane Configuration and Traffic Control

2.3 Traffic Volumes

Paradigm conducted turning movement counts at the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Blue West Equipment Access A on 28 November 2023. Paradigm also conducted turning movement counts at the Goshen Road and Bilger Access intersection on 28 November 2023 to determine the existing traffic in and out of the Bilger Slinger Services site.

Table 2.1 summarizes the traffic peak hours at each intersection.

 Intersection
 AM Peak Hour
 PM Peak Hour

 Highway 3 and Goshen Road
 7:45 - 8:45
 3:45 - 4:45

 Highway 3 and the Blue West Equipment Access A Goshen Road and Bilger Access
 7:45 - 8:45
 3:45 - 4:45

 Tool - 8:00
 4:00 - 5:00

TABLE 2.1: INTERSECTION PEAK HOURS

Appendix B contains the detailed traffic counts for the intersections.

Figure 2.2 illustrates the existing AM and PM weekday peak hour turning movement traffic volumes at the study area intersections.

Table 2.2 shows the 8-hour hourly breakdown of inbound and outbound traffic on Gosehn Road at the Bilger site access.

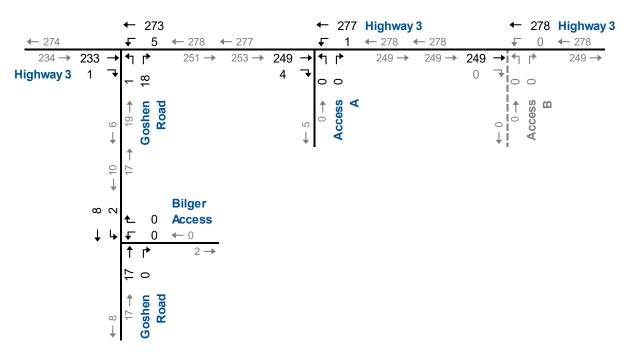
The study area roadway and driveway traffic volumes and turning movements are noted to be low to moderate:

- The peak directional traffic volume on Highway 3 is under 300 vph in the AM peak hour and under 450 vph in the PM peak hour.
- ► The traffic on Goshen Road is under 20 vph in either direction during either peak hour.
- ▶ The driveway turning movements are less than 5 vph.

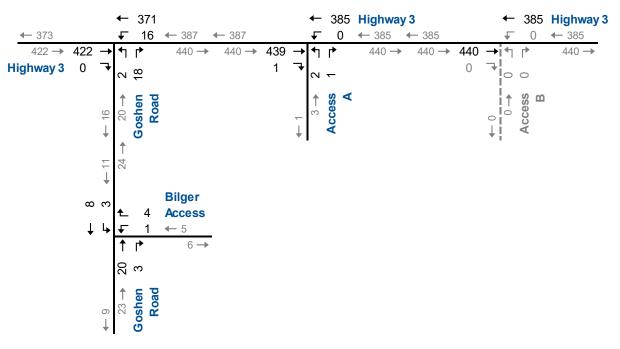
It is noted that with the proposed changes, the driveway volumes on Goshen Road for the property at 480 Highway 3 will be transferred to the subject site driveways on Highway 3 for the property at 494 Highway 3.

AM Peak Hour





PM Peak Hour





Existing Traffic Volumes

TABLE 2.2: BILGER SITE ACCESS 8-HOUR TRAFFIC VOLUMES

Time	Inbound	Outbound
7:00 – 8:00 AM	2	0
8:00 – 9:00 AM	2	1
9:00 – 10:00 AM	2	4
11:30 AM – 12:30 PM	2	2
12:30 – 1:30 PM	0	0
3:00 – 4:00 PM	4	2
4:00 – 5:00 PM	6	5
5:00 - 6:00 PM	5	4
Total	20	18

2.4 Traffic Operations

The level of service conditions at the Highway 3 and Goshen Road intersection, and at the Highway 3 and Access A intersection have been assessed through intersection operational analysis using Synchro 11.

Intersection level of service (LOS) is a recognized method of quantifying the average delay experienced by drivers at intersections. It is based on the delay experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles intending to make a particular movement, compared to the estimated capacity for that movement. The capacity is based on several criteria related to the opposing traffic flows and intersection geometry.

The highest possible rating is LOS A, under which the average total delay is equal or less than 10.0 seconds per vehicle. When the average delay exceeds 80 seconds for signalized intersections, 50 seconds for unsignalized intersections or when the volume to capacity (v/c) ratio is greater than 1.00, the movement is classed as LOS F and remedial measures are usually implemented if they are feasible. LOS E is usually used as a guideline for the determination of road improvement needs on through lanes, while LOS F may be acceptable for left-turn movements at peak times, depending on delays.

Movements are considered critical under the following conditions:

- Signalized Intersections:
 - v/c ratios for overall intersection operations, through movements or shared through/turning movements increased to 0.85 or above;
 - v/c ratios for exclusive movements increased to 0.95 or above; or



- queues for an individual movement are projected to exceed available turning lane storage.
- Unsignalized Intersections:
 - LOS based on average delay per vehicle, on individual movements exceeds LOS "E"; or
 - the estimated 95th percentile queue length for an individual movement exceeds the available queue storage.

Table 2.3 summarizes the results of the intersection operational analysis under existing conditions, including the AM and PM peak hour LOS, v/c ratios, and 95th percentile queues experienced.

The results indicate that the two intersections are operating at satisfactory levels of service (LOS A/B), and with no problem movements.

Appendix C contains the detailed Synchro 11 reports.

TABLE 2.3: EXISTING TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	h					
Period					Eastb	ound			Westl	ound		I	Northi	oounc	ı	;	South	bounc	I	
Analysis P	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
ınc	Highway 3 & Goshen Road	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Noau		V/C Q		0.00	>		< <	0.00			0.03		>						
eal	Highway 3 & Access A		LOS		A	>	Α	<	A		Α	A		>	Α					
AM		TWSC	Delay		0	>	0	<	8		0	0		>	0					
٩			V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
'n	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.04		>						
ak			Q		0	>	Α.	<	1			1		>	-					
			LOS		A	>	Α	<	A		A 0	В		>	В					
PM	Highway 3 & Access A	TWSC	Delay V/C		0	^	0	<	0		U	15		>	15					
	3		Q Q		0.00	>		< <	0.00			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



3 Proposed Site Changes

3.1 Description of Site Changes

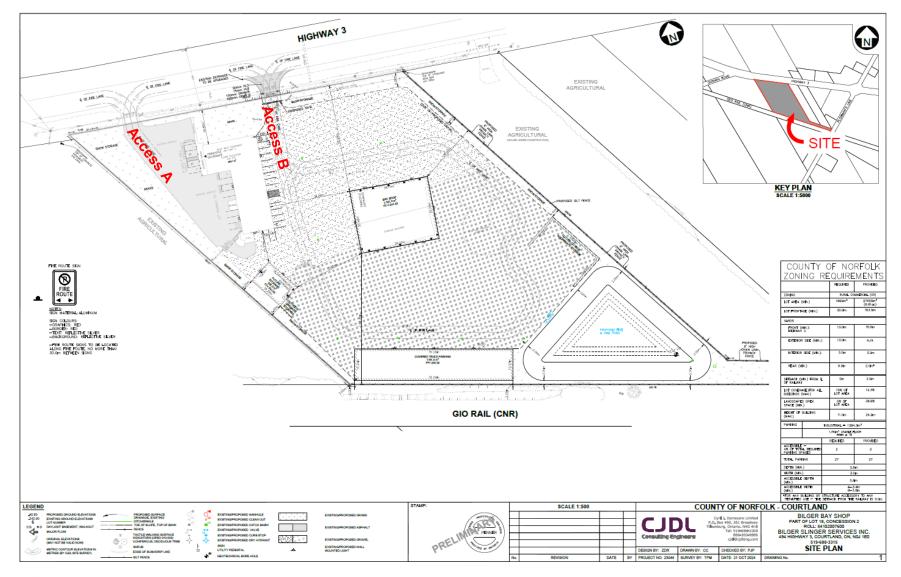
The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for use as a storage and sales facility for farm, residential, and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3 and shared use of the site with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 m² with a larger warehouse of 1161 m² GFA for Blue West Equipment. Bilger Slinger Services currently stores and services 18 slinger trucks but is planning to expand to include 24 trucks by 2034.

The property has two driveways on Highway 3 approximately 70 metres apart and at distances of 165 metres and 235 metres east of Goshen Road. The westerly driveway is proposed to be used for Blue West Equipment, and the easterly driveway by Bilger Slinger Services. It is noted that the easterly driveway is temporarily closed.

Figure 3.1 includes the preliminary grading plan for the site illustrating the proposed changes including the access arrangement.





Proposed Site Changes

3.2 Site Trip Generation

The trip generation for each of the Blue West Equipment and Bilger Slinger Services sites are expected to be the same as under existing conditions.

Table 3.1 summarizes the number of trips generated by the proposed development.

TABLE 3.1: TRIP GENERATION

Site	AM	Peak H	lour	PM Peak Hour					
Site	In	Out	Total	In	Out	Total			
Blue West Equipment	5	0	5	1	3	4			
Bilger Slinger Services	2	0	2	6	5	11			
Total Trip Generation	7	0	7	7	8	15			

3.3 Development Trip Distribution and Assignment

The trip distribution for the Bilger Slinger Services site traffic was determined based on the existing assignment of traffic volumes at the site driveway and the existing travel patterns within the study area. **Table 3.2** displays the breakdown of trip distributions used in this study.

TABLE 3.2: ESTIMATED TRIP DISTRIBUTION

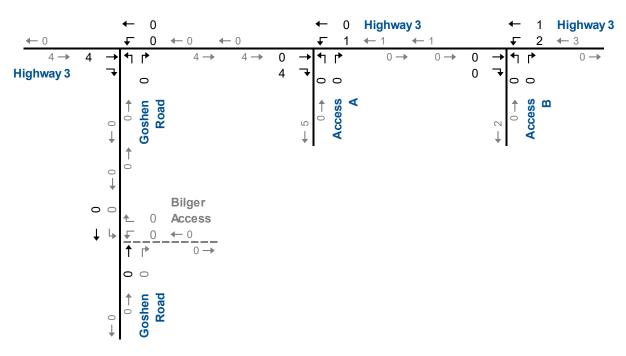
Origin/Destination	AM Pea	ak Hour	PM Peak Hour					
Origin/Destination	Inbound	Outbound	Inbound	Outbound				
East via Highway 3	83%	0%	50%	72%				
West via Highway 3	17%	0%	0%	8%				
South via Goshen Road	0%	0%	50%	20%				
Total	100%	0%	100%	100%				

Figure 3.2 illustrates the site-generated traffic volumes for the AM and PM peak hours. The site traffic for Blue West Equipment is unchanged from the existing traffic volumes.

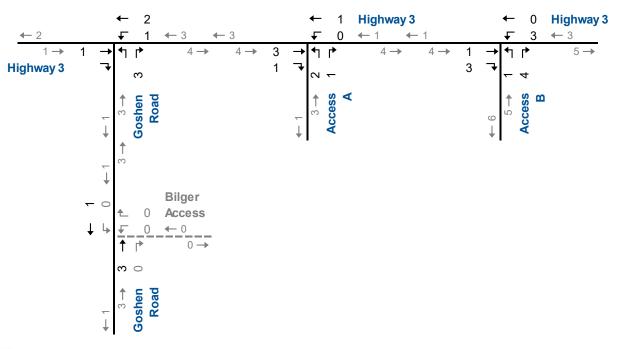
It is noted that as under existing traffic conditions the driveway volumes will remain minimal following the proposed site changes.

AM Peak Hour





PM Peak Hour





Site Generated Traffic Volumes

4 Access Assessment

The assessment of the subject driveways under future traffic conditions is based on future background and total traffic volumes, corresponding to the proposed timing of site changes (2024), five years after changes (2029), and ten years after changes (2034).

4.1 Background Traffic Forecasts

In order to derive the generalized background traffic volumes, a growth rate of 2.0% per annum was applied to the existing roadway traffic volumes.

No background developments were included in the future traffic volumes.

4.2 2024 Background Traffic Operations

Figure 4.1 illustrates the 2024 background traffic volumes, including road traffic growth.

The 2024 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

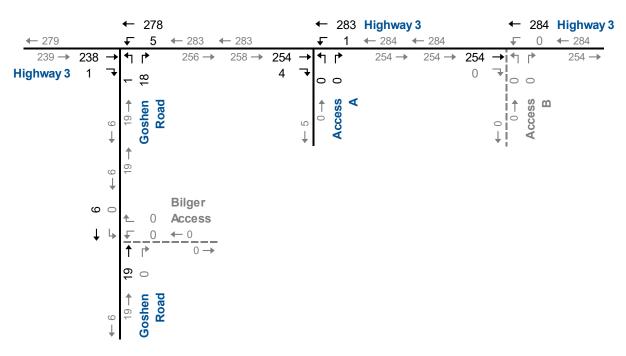
It is noted that no changes have been made to the turning movements at the intersection of Highway 3 and Goshen Road even they would likely be reduced following the proposed relocation of Bilger truck operations.

Table 4.1 summarizes the results of the 2024 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at satisfactory levels of service (LOS A/B), and with no problem movements.

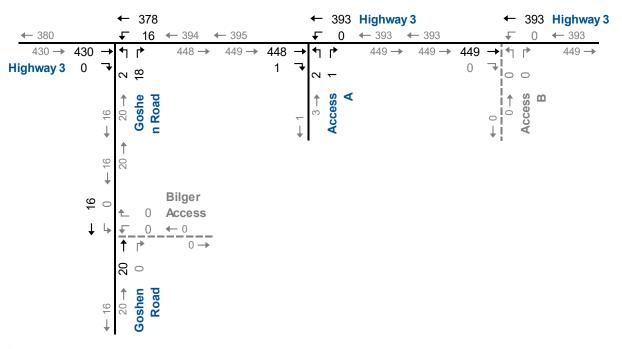
Appendix D contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2024 Background Traffic Volumes

TABLE 4.1: 2024 BACKGROUND TRAFFIC OPERATIONS

ठ										Directi	on/Mo	veme	nt/App	oroach	1					
erio					Eastb	ound			Westk	ound			Northl	bound	I	;	South	bounc	I	
Analysis Period	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
ın	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Road		V/C Q		0.00	>		<	0.01			0.03		>						
eak		TWSC	LOS		A	^	Α		A		Α	A		>	Α					
AM P			Delay		0	>	0	<	8		0	0		>	0					
4	Highway 3 & Access A		V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
5	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.04		>						
ak			Q LOS		0 A	>	Α	<	1 A		Α	1 B		>	В					
			Delay		0	>	0	<	0		0	15		>	15					
PM	Highway 3 & Access A	TWSC	V/C		0.00	>			0.00			0.01		>	10					
			Q		0.00	>		<	0			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.3 2024 Total Traffic Operations

Figure 4.2 illustrates the 2024 total traffic volumes, including trips generated by the proposed development.

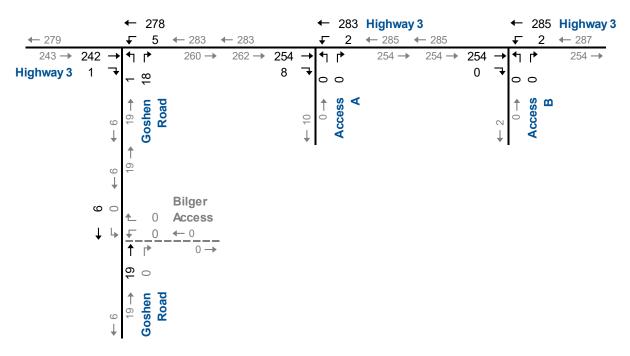
The 2024 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

Table 4.2 summarizes the results of the 2024 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

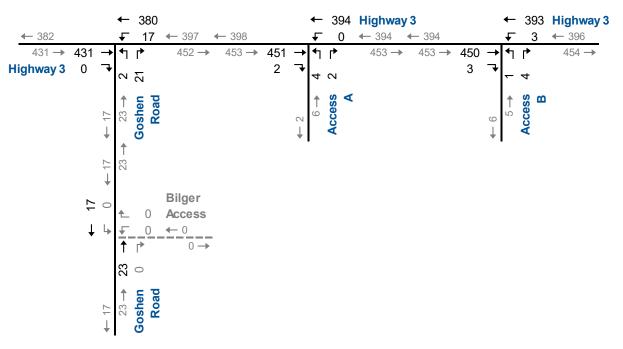
Appendix E contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2024 Total Traffic Volumes

TABLE 4.2: 2024 TOTAL TRAFFIC OPERATIONS

ъ										Directi	on/Mo	oveme	nt/App	roacl	1					
erio					Eastb	ound			Westk	ound		I	Northi	oound	I	;	South	bound	I	
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	ijeŢ	Through	Right	Approach	Пеff	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
<u>-</u>	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	0 >	v v v v	A 8 0.01 0		A 0	B 10 0.03 1		^ ^ ^ ^	B 10					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^ ^	A 0	v v v	A 8 0.00		A 0	A 0 0.00 0		^ ^ ^	A 0					
AM	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0		A 8 0.00		A 0	A 0 0.00		^	A 0					
_	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 9 0.02 1		A 0	B 12 0.04 1		^ ^ ^	B 12					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 15 0.02 1		^ ^ ^	C 15					
PM	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	A 0	v v v	A 8 0.00 0		A 0	B 12 0.01 0		^ ^ ^ ^	B 12					

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</> - Shared with through movement



4.4 2029 Background Traffic Operations

Figure 4.3 illustrates the 2029 background traffic volumes, including road traffic growth.

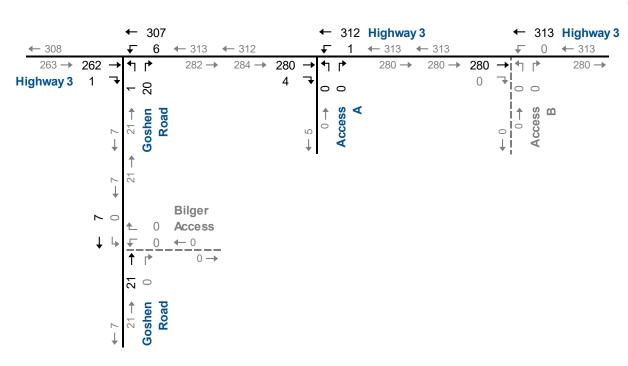
The 2029 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

Table 4.3 summarizes the results of the 2029 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at acceptable levels of service, and with no problem movements.

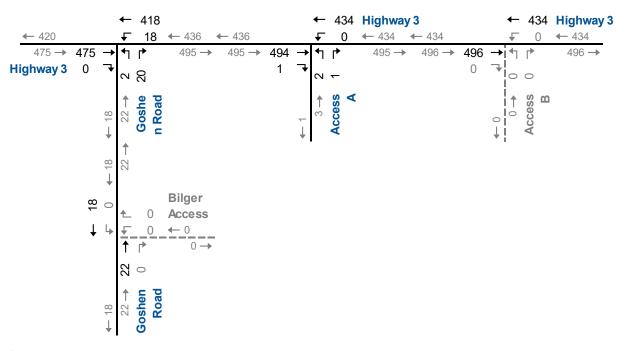
Appendix F contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2029 Background Traffic Volumes

TABLE 4.3: 2029 BACKGROUND TRAFFIC OPERATIONS

ठ										Directi	on/Mo	veme	nt/App	oroacl	1					
erio					Eastb	ound			Westk	ound			Northi	bound	I	;	South	bounc	I	
Analysis Period	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
nr	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	8		0	10		>	10					
Peak Hour	Road		V/C Q		0.00	>		<	0.01			0.03		>						
ea			LOS		A	>	Α	<	A		Α	A		>	Α					
AMF	Highway 3 & Access A	TWSC	Delay		0	>	0	<	8		0	0		>	0					
⋖	nighway 5 & Access A	TWSC	V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
늄	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	12		>	12					
Peak Hour	Road		V/C		0.00	>		<	0.02			0.05		>						
ak			Q		0	>	^	<	1		^	1		>	С					
			LOS		A 0	>	A 0	<	A 0		A 0	C 16		>	16					
PM		TWSC	Delay V/C		0.00	>	U	<	0.00		U	0.01		>	10					
	Highway 3 & Access A		Q		0.00	>		<	0.00			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.5 2029 Total Traffic Operations

Figure 4.4 illustrates the 2029 total traffic volumes, including trips generated by the proposed development.

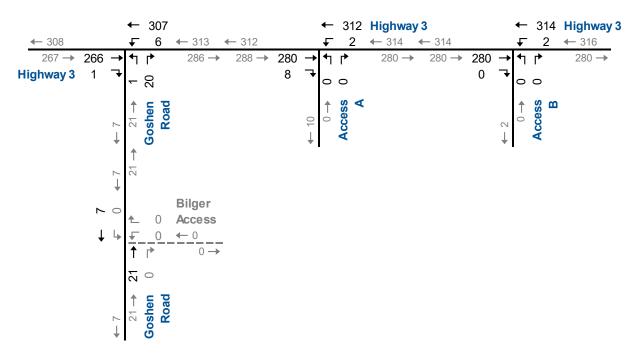
The 2029 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

Table 4.4 summarizes the results of the 2029 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

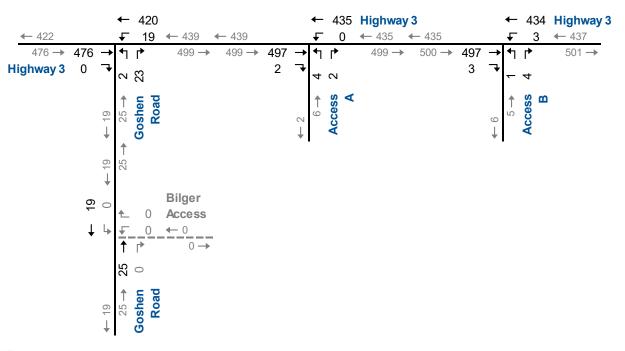
Appendix G contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2029 Total Traffic Volumes

TABLE 4.4: 2029 TOTAL TRAFFIC OPERATIONS

ъ										Directi	on/Mo	oveme	nt/App	roacl	h					
erio					Eastb	ound			Westl	ound		ı	Northi	oound	i	,	South	bound	I	
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	IJeТ	Through	Right	Approach	Left	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0		A 8 0.01 0		A 0	B 10 0.03 1		^ ^ ^	B 10					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	· · · · · ·	A 8 0.00		A 0	A 0 0.00		^ ^ ^	A 0					
AN	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	/	A 8 0.00		A 0	A 0 0.00		^ ^ ^	A 0					
r	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	<td>A 9 0.02 1</td> <td></td> <td>A 0</td> <td>B 12 0.05 2</td> <td></td> <td>^ ^ ^</td> <td>B 12</td> <td></td> <td></td> <td></td> <td></td> <td></td>	A 9 0.02 1		A 0	B 12 0.05 2		^ ^ ^	B 12					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 16 0.02 1		^ ^ ^	C 16					
ď	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 8 0.00 0		A 0	B 13 0.01 0		^ ^ ^	B 13					

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.6 2034 Background Traffic Operations

Figure 4.5 illustrates the 2034 background traffic volumes, including road traffic growth.

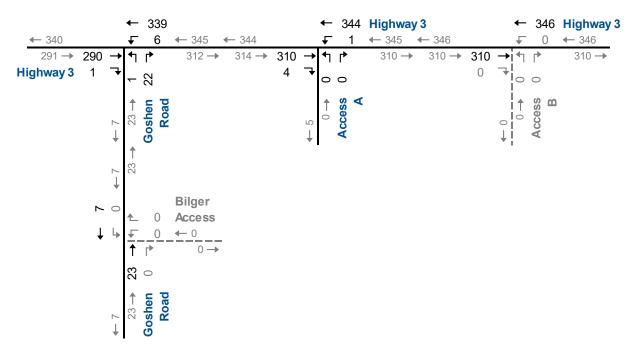
The 2034 background traffic volumes have been analyzed using the same methodology as under existing traffic conditions.

Table 4.5 summarizes the results of the 2034 background traffic operations. The results indicate that the intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are forecast to operate at acceptable levels of service, and with no problem movements.

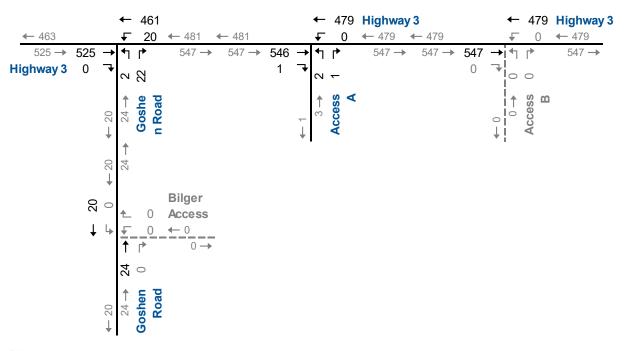
Appendix H contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2034 Background Traffic Volumes

TABLE 4.5: 2034 BACKGROUND TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	h					
Period					Eastb	ound			Westl	ound		I	Northi	oounc	ı	;	South	bounc	i	
Analysis P	Intersection	Control Type	MOE	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Overall
			LOS		Α	>	Α	<	Α		Α	В		>	В					
'n	Highway 3 & Goshen Road	TWSC	Delay		0	>	0	<	8		0	11		>	11					
Peak Hour	Road		V/C Q		0.00	>		< <	0.01			0.04		>						
eal			LOS		A	>	Α	<	A		Α	A		>	Α					
AM F	Highway 3 & Access A	TWSC	Delay		0	>	0	<	8		0	0		>	0					
⋖	nigriway 5 & Access A	TWSC	V/C		0.00	>		<	0.00			0.00		>						
			Q		0	>		<	0			0		>						
			LOS		Α	>	Α	<	Α		Α	В		>	В					
5	Highway 3 & Goshen	TWSC	Delay		0	>	0	<	9		0	13		>	13					
Peak Hour	Road		V/C		0.00	>		<	0.03			0.06		>						
ak			Q		0	>		<	1			2		>						
			LOS		A	>	A	<	Α		A	С		>	C					
P		TWSC	Delay		0	>	0	<	0		0	18		>	18					
	Highway 3 & Access A		V/C Q		0.00	>		< <	0.00			0.01		>						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</>- Shared with through movement



4.7 2034 Total Traffic Operations

Figure 4.6 illustrates the 2034 total traffic volumes, including trips generated by the proposed development.

The 2034 total traffic volumes have been analyzed using the same methodology as under existing and background traffic conditions.

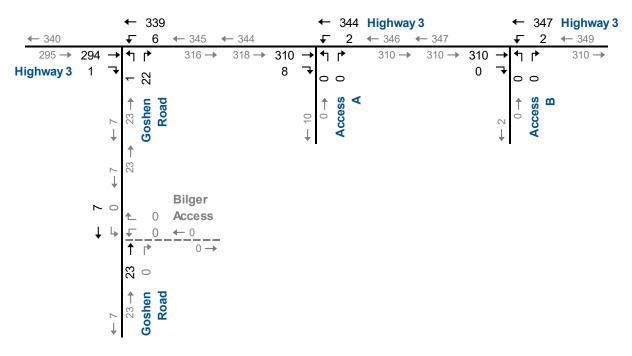
As seen in **Figure 4.6**, the study area roadway traffic volumes and turning movements are projected to remain low to moderate in 2034 as they are under existing conditions.

Table 4.6 summarizes the results of the 2034 total traffic operations. The results indicate that the intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours. Both Site Access intersections to Highway 3 are forecast to operate at acceptable levels of service during the AM and PM peak hours.

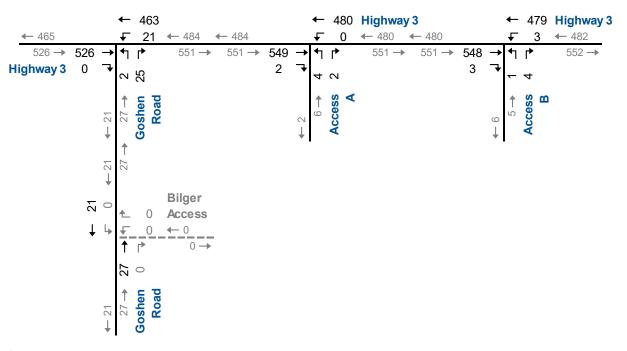
Appendix I contains the supporting detailed Synchro 11 reports.

AM Peak Hour





PM Peak Hour





2034 Total Traffic Volumes

TABLE 4.6: 2034 TOTAL TRAFFIC OPERATIONS

7										Directi	on/Mo	veme	nt/App	roacl	า					
erio					Eastb	ound			Westk	ound		ı	Northi	oound	I	;	South	bounc		
Analysis Period	Intersection	Control Type	MOE	ijeŢ	Through	Right	Approach	ijeŢ	Through	Right	Approach	Пеff	Through	Right	Approach	IJeТ	Through	Right	Approach	Overall
<u>.</u>	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^	0 >	v v v v	A 8 0.01 0		A 0	B 11 0.04 1		^ ^ ^ ^	B 11					
AM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0		A 8 0.00		A 0	A 0 0.00 0		^ ^ ^	A 0					
A	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00	^ ^ ^	A 0	v v v	A 8 0.00		A 0	A 0 0.00		^	A 0					
r	Highway 3 & Goshen Road	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	<td>A 9 0.03 1</td> <td></td> <td>A 0</td> <td>B 13 0.06 2</td> <td></td> <td>^ ^ ^</td> <td>B 13</td> <td></td> <td></td> <td></td> <td></td> <td></td>	A 9 0.03 1		A 0	B 13 0.06 2		^ ^ ^	B 13					
PM Peak Hour	Highway 3 & Access A	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^	A 0	v v v	A 0 0.00 0		A 0	C 18 0.02 1		^ ^ ^	C 18					
a	Highway 3 & Access B	TWSC	LOS Delay V/C Q		A 0 0.00 0	^ ^ ^ ^	A 0	v v v	A 9 0.00 0		A 0	B 14 0.01 0		^ ^ ^ ^	B 14					

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m) TWSC - Two-Way Stop Control

</> - Shared with through movement

4.8 Review of Access Locations

The existing access locations for the subject site were reviewed for available sight distances and left-turn lane requirements. The sight distance assessment is based on field inspection and Transportation Association of Canada design standards.

4.8.1 Sight Distance Assessment

Available sight distances for the existing access points were measured during a site visit on 27 November 2023 and are compared with sight distance requirements identified in the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads⁵ (GDGCR).

Stopping and Decision Sight Distance requirements were reviewed for a design speed of 70 km/h corresponding to the posted speed of 60 km/h.

Table 4.7 summarizes the sight distance measurements and requirements at the access intersections, which indicate that the two existing driveway locations on Highway 3 satisfy the TAC requirements for Decision Sight Distances and Stopping Sight Distance.

TABLE 4.7: SIGHT DISTANCE MEASUREMENTS

	De	cision Sigh	t Distance	(m)	Stoppii	ng Sight
Intersection	Left	-Turn	Right	t-Turn	Distar	nce (m)
	Required	Measured	Required	Measured	Required	Measured
Highway 3 and Access A		250+		250+		250+
	210		185		185	
Highway 3 and		250+	, , ,	250+		250+
Access B		230+		230+		250+

4.8.2 Left-Turn Lanes

The assessment of the need for auxiliary left-turning lanes is based on the requirements and procedures detailed in the Ministry of Transportation Design Supplement for the Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads⁶.

It is noted that the forecast westbound left-turn movements on Highway 3 at Site Access A and at Site Access B are less than five vph during AM and PM peak hours, which is less than 2.5% of the total

⁶ MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads, June 2017.



Transportation Association of Canada, Geometric Design Guide for Canadian Roads, June 2017.

westbound traffic volumes. The provision of left-turn lanes on Highway 3 at each of the two driveways is, therefore, identified as not warranted.

It is noted that the need for a westbound left-turn lane on Highway 3 at the intersection of Goshen Road is also identified as not warranted.

5 Conclusions and Recommendations

5.1 Conclusions

Based on the investigations carried out, it is concluded that:

- Existing Traffic Conditions: The intersections of (1) Highway 3 and Goshen Road and (2) Highway 3 and the Westerly Access are operating at satisfactory levels of service (LOS A/B).
- ▶ Site Traffic Volumes: The existing site driveway volumes indicate that 5 AM peak hour trips and 4 PM peak hour generated by the current site operations.

With the proposed changes, the site traffic volumes are projected to increase to 7 AM peak hour trips and 15 PM peak hour trips.

Based on the current and future site traffic volumes, the site could be considered to be a low trip generator with minimal impact on driveway and road traffic operations.

- ▶ **Background Traffic Conditions:** The two intersections are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 background traffic conditions.
- ▶ Total Traffic Conditions: The intersection of Highway 3 and Goshen Road is forecast to operate at satisfactory levels of service (LOS A/B) during the AM and PM peak hours under 2024, 2029, and 2034 total traffic conditions, as under existing conditions.
- ▶ **Site Accesses:** The Westerly and Easterly Site Access intersections on Highway 3 are forecast to operate at acceptable levels of service under 2024, 2029, and 2034 total traffic conditions.

Both access locations satisfy the sight distance requirements, and do not require an auxiliary westbound left-turn lane on Highway 3 to accommodate inbound left-turning traffic.

5.2 Recommendations

Based on the findings and conclusions of this study, it is recommended that the Site Changes with two driveways for the property at 494 Highway 3, Courtland be considered for approval as proposed.

Appendix A

Pre-Study Consultation



From: Patrick Neal

To: "Mentley, Ryan (MTO)"; Stephen Gradish

Cc: Rajan Philips

Subject: (230661) 494 Highway 3, Courtland TIS - Pre-Study Consultation

Date: November 2, 2023 4:04:00 PM **Attachments:** (230661) Site Location.png

image001.png image002.png

23044 Preliminary Grading Plan.pdf

Hi Ryan and Stephen,

We have been retained to complete the Transportation Impact Study (TIS) in support of a Site Plan application for a Truck Yard and Warehouse expansion located at 494 Highway 3 in Courtland, Norfolk County.

The subject property is located on the south side of Highway 3, east of Goshen Road. The property is owned by Bilger Slinger Services and is currently rented to Blue West Equipment for a storage and sales facility for farm, residential and commercial equipment.

The proposed changes to the site are to accommodate the relocation of Bilger Slinger Services from their current location at 480 Highway 3 to the property at 494 Highway 3, and share its use with Blue West Equipment.

Specifically, the changes will include the development of a truck yard for Bilger Slinger Services and the replacement of an existing warehouse with a GFA of 456 sq. m. with a larger warehouse of 1161 sq. m. GFA.

The site operations for Bilger Slinger Services will include:

- 18 employees.
- Storing and servicing 18 slinger trucks with a possible ten-year expansion to 24 trucks.
- Main deliveries twice a month to the site.

The site operations for Blue West Equipment will include:

- · Six to eight employees.
- 12 customers per day.
- Main deliveries twice a week to the site.

A Google Map showing the two properties and a preliminary grading plan for 494 Highway 3 are attached.

The property has two driveways on Highway 3 approximately 76 metres apart. Both driveways are proposed to be retained as part of redevelopment, and both are needed to serve the two businesses that will be located on the subject site. It is noted that the proposed site changes will not add new traffic to Highway 3, because traffic from the business at 480 Highway 3 is already included in the existing traffic volumes. The relocation will only shift a portion of the turning movements from the intersection at Goshen Road to the driveways at 494 Highway 3.

Based on the above information, we have prepared the following scope of work for

review/approval:

- Weekday AM and PM peak hours of analysis.
- Study Area Intersections:
 - Highway 3 and Goshen Road (unsignalized); and
 - two access intersections on Highway 3.
- Horizon Years (as required by MTO): (1) development completion, (2) five years after completion, and (3) ten years after completion.
- Background Growth: 2.0% compounded per annum, please confirm.
- Background Developments: Please confirm and provide corresponding site statistics or TIS.
- Roadway Improvements: Please confirm any planned changes to the study area roadways.
- Trip Generation: ITE Trip Generation Manual 11th Edition.
- Trip Distribution: Existing traffic patterns.

Please let us know if you have any questions or comments.

Regards,

Patrick Neal, EIT

Transportation Consultant



Paradigm Transportation Solutions Limited

5A-150 Pinebush Road, Cambridge ON N1R 8J8

p: 416.479.9684 x510 m: 416.688.7338 e: <u>pneal@ptsl.com</u>

w: www.ptsl.com



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Appendix B

Existing Traffic Data





Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023

Page No: 1

Turning Movement Data

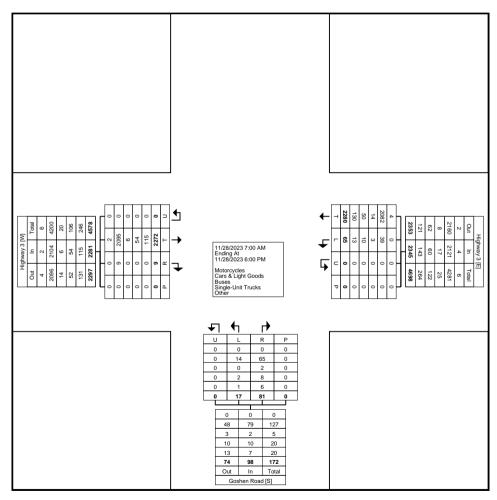
	I				ı	i dii	mig wie	VCITICITE L	Julu	1			0 1 5 1		1	1
			Highway 3					Highway 3					Goshen Road			
Start Time	Thru	Right	Eastbound U-Turn	Peds	Ann Total	1 644	Thru	Westbound U-Turn	Peds	Ann Total	Left	Right	Northbound U-Turn	Peds	App. Total	Int. Total
7:00 AM	52	2	0-14111	0	App. Total 54	Left 2	51	0-14111	0	App. Total 53	1	1	0-14111	0	Арр. Тотат 2	109
7:15 AM	66	1	0	0	67	1	56	0	0	57	0	4	0	0	4	128
7:30 AM	48	0	0	0	48	3	67	0	0	70	2	2	0	0	4	122
7:45 AM	61	0	0	0	61	<u>3</u>	71	0	0	72	0	8	0	0	8	141
Hourly Total	227	3	0	0	230	7	245	0	0	252	3	15	0	0	18	500
8:00 AM	63	1	0	0	64	1	66	0	0	67	1	1	0	0	2	133
8:15 AM	56	0	0	0	56	0	61	0	0	61	0	1	0	0	1	118
8:30 AM	53	0	0	0	53	3	75	0	0	78	0	8	0	0	8	139
8:45 AM	47	1	0	0	48	2	74	0	0	76	0	4	0	0	4	128
Hourly Total	219	2	0	0	221	6	276	0	0	282	1	14	0	0	15	518
9:00 AM	45	0	0	0	45	0	56	0	0	56	0	3	0	0	3	104
9:15 AM	46	0	0	0	46	1	64	0	0	65	0	0	0	0	0	111
9:30 AM	48	0	0	0	48	2	72	0	0	74	0	3	0	0	3	125
9:45 AM	42	2	0	0	44	3	61	0	0	64	0	1	0	0	1	109
Hourly Total	181	2	0	0	183	6	253	0	0	259	0	7	0	0	7	449
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:30 AM	66	0	0	0	66	1	66	0	0	67	0	0	0	0	0	133
11:45 AM	82	0	0	0	82	2	58	0	0	60	0	4	0	0	4	146
Hourly Total	148	0	0	0	148	3	124	0	0	127	0	4	0	0	4	279
12:00 PM	63	0	0	0	63	3	74	0	0	77	1	2	0	0	3	143
12:15 PM	89	0	0	0	89	3	64	0	0	67	1	3	0	0	4	160
12:30 PM	68	0	0	0	68	4	65	0	0	69	1	3	0	0	4	141
12:45 PM	62	0	0	0	62	3	65	0	0	68	0	0	0	0	0	130
Hourly Total	282	0	0	0	282	13	268	0	0	281	3	8	0	0	11	574
1:00 PM	78	0	0	0	78	0	74	0	0	74	2	1	0	0	3	155
1:15 PM	57	0	0	0	57	1	69	0	0	70	2	1	0	0	3	130
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	135	0	0	0	135	1	143	0	0	144	4	2	0	0	6	285
3:00 PM	99	0	0	0	99	2	70	0	0	72	0	2	0	0	2	173
3:15 PM	62	0	0	0	62	0	82	0	0	82	0	2	0	0	2	146
3:30 PM	97	0	0	0	97	1	82	0	0	83	0	0	0	0	0	180
3:45 PM	96	0	0	0	96	3	90	0	0	93	1	1	0	0	2	191
Hourly Total	354	0	0	0	354	6	324	0	0	330	1	5	0	0	6	690
4:00 PM	113	0	0	0	113	5	102	0	0	107	1	1	0	0	2	222
4:15 PM	110	0	0	0	110	4	77	0	0	81	0	5	0	0	5	196
4:30 PM	103	0	0	0	103	4	102	0	0	106	0	11	0	0	11	220

4:45 PM	86	0	0	0	86	4	88	0	0	92	1	5	0	0	6	184
Hourly Total	412	0	0	0	412	17	369	0	0	386	2	22	0	0	24	822
5:00 PM	96	0	0	0	96	1	86	0	0	87	2	0	0	0	2	185
5:15 PM	88	0	0	0	88	0	58	0	0	58	1	1	0	0	2	148
5:30 PM	76	1	0	0	77	2	75	0	0	77	0	0	0	0	0	154
5:45 PM	54	1	0	0	55	3	59	0	0	62	0	3	0	0	3	120
Hourly Total	314	2	0	0	316	6	278	0	0	284	3	4	0	0	7	607
Grand Total	2272	9	0	0	2281	65	2280	0	0	2345	17	81	0	0	98	4724
Approach %	99.6	0.4	0.0	-	-	2.8	97.2	0.0	-	-	17.3	82.7	0.0	-	-	-
Total %	48.1	0.2	0.0	-	48.3	1.4	48.3	0.0	-	49.6	0.4	1.7	0.0	-	2.1	-
Motorcycles	2	0	0	-	2	0	4	0	-	4	0	0	0	-	0	6
% Motorcycles	0.1	0.0	-	-	0.1	0.0	0.2	-	-	0.2	0.0	0.0	-	-	0.0	0.1
Cars & Light Goods	2095	9	0	-	2104	39	2082	0	-	2121	14	65	0	-	79	4304
% Cars & Light Goods	92.2	100.0	-	-	92.2	60.0	91.3	-	-	90.4	82.4	80.2	-	-	80.6	91.1
Buses	6	0	0	-	6	3	14	0	-	17	0	2	0	-	2	25
% Buses	0.3	0.0	-	-	0.3	4.6	0.6	-	-	0.7	0.0	2.5	-	-	2.0	0.5
Single-Unit Trucks	54	0	0	-	54	10	50	0	-	60	2	8	0	-	10	124
% Single-Unit Trucks	2.4	0.0	-	-	2.4	15.4	2.2	-	-	2.6	11.8	9.9	-	-	10.2	2.6
Articulated Trucks	114	0	0	-	114	13	130	0	-	143	1	6	0	-	7	264
% Articulated Trucks	5.0	0.0	-	-	5.0	20.0	5.7	-	-	6.1	5.9	7.4	-	-	7.1	5.6
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
% Pedestrians	-	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 3



Turning Movement Data Plot



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023

Page No: 4

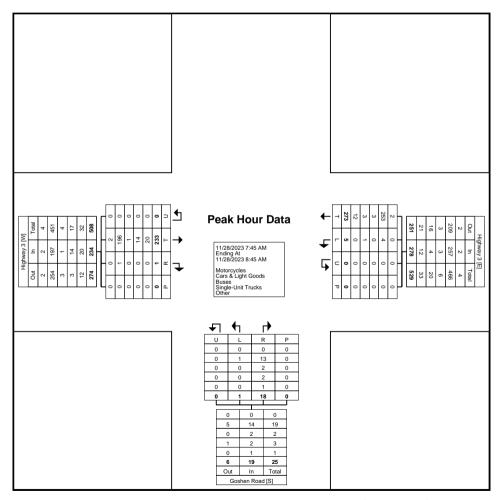
Turning Movement Peak Hour Data (7:45 AM)

						9		ait i 10 ai i	Jaia (1							
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:45 AM	61	0	0	0	61	1	71	0	0	72	0	8	0	0	8	141
8:00 AM	63	1	0	0	64	1	66	0	0	67	1	1	0	0	2	133
8:15 AM	56	0	0	0	56	0	61	0	0	61	0	1	0	0	1	118
8:30 AM	53	0	0	0	53	3	75	0	0	78	0	8	0	0	8	139
Total	233	1	0	0	234	5	273	0	0	278	1	18	0	0	19	531
Approach %	99.6	0.4	0.0	-	-	1.8	98.2	0.0	-	-	5.3	94.7	0.0	-	-	-
Total %	43.9	0.2	0.0	-	44.1	0.9	51.4	0.0	-	52.4	0.2	3.4	0.0	-	3.6	-
PHF	0.925	0.250	0.000	-	0.914	0.417	0.910	0.000	-	0.891	0.250	0.563	0.000	-	0.594	0.941
Motorcycles	2	0	0	-	2	0	2	0	-	2	0	0	0	-	0	4
% Motorcycles	0.9	0.0	-	-	0.9	0.0	0.7	-	-	0.7	0.0	0.0	-	-	0.0	0.8
Cars & Light Goods	196	1	0	-	197	4	253	0	-	257	1	13	0	-	14	468
% Cars & Light Goods	84.1	100.0	-	-	84.2	80.0	92.7	-	-	92.4	100.0	72.2	-	-	73.7	88.1
Buses	1	0	0	-	1	0	3	0	-	3	0	2	0	-	2	6
% Buses	0.4	0.0	-	-	0.4	0.0	1.1	-	-	1.1	0.0	11.1	-	-	10.5	1.1
Single-Unit Trucks	14	0	0	-	14	1	3	0	-	4	0	2	0	-	2	20
% Single-Unit Trucks	6.0	0.0	-	-	6.0	20.0	1.1	-	-	1.4	0.0	11.1	-	-	10.5	3.8
Articulated Trucks	19	0	0	-	19	0	12	0	-	12	0	1	0	-	1	32
% Articulated Trucks	8.2	0.0	-	-	8.1	0.0	4.4	-	-	4.3	0.0	5.6	-	-	5.3	6.0
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	0.4	0.0	-	-	0.4	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.2
Bicycles on Crosswalk	-	-	-	0	-	ı	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	·	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	ı	-	-	-	-	-	-	-	-	-	-



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 5



Turning Movement Peak Hour Data Plot (7:45 AM)



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 6

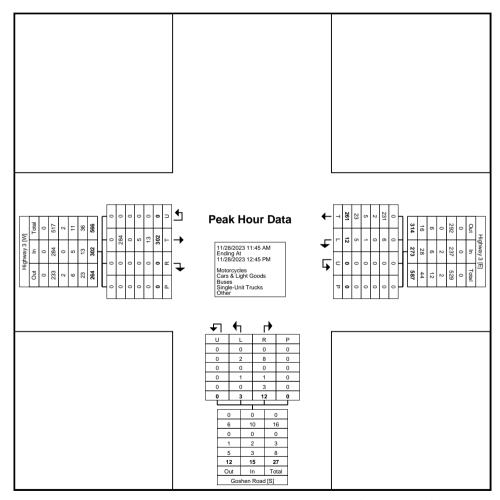
Turning Movement Peak Hour Data (11:45 AM)

					running	INIOACII	ICHT L CO	IN I IOUI L	<i>γ</i> αια (1 1	.+J AIVI)						
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			1
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
11:45 AM	82	. 0	0	0	82	2	58	0	0	60	0	4	. 0	0	4	146
12:00 PM	63	0	0	0	63	3	74	0	0	77	1	2	0	0	3	143
12:15 PM	89	0	0	0	89	3	64	0	0	67	1	3	0	0	4	160
12:30 PM	68	. 0	. 0	0	68	4	65	0	0	69	1	3	. 0	0	4	141
Total	302	0	0	0	302	12	261	0	0	273	3	12	0	0	15	590
Approach %	100.0	0.0	0.0	-	-	4.4	95.6	0.0	-	-	20.0	80.0	0.0	-	-	-
Total %	51.2	0.0	0.0	-	51.2	2.0	44.2	0.0	-	46.3	0.5	2.0	0.0	-	2.5	-
PHF	0.848	0.000	0.000	-	0.848	0.750	0.882	0.000	-	0.886	0.750	0.750	0.000	-	0.938	0.922
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	_		-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	284	0	0	-	284	6	231	0	-	237	2	8	0	-	10	531
% Cars & Light Goods	94.0	-	-	-	94.0	50.0	88.5	-	-	86.8	66.7	66.7	-	-	66.7	90.0
Buses	0	0	0	-	0	0	2	0	-	2	0	0	0	-	0	2
% Buses	0.0	_	-	-	0.0	0.0	0.8	-	-	0.7	0.0	0.0	-	-	0.0	0.3
Single-Unit Trucks	5	0	0	-	5	1	5	0	-	6	1	1	0	-	2	13
% Single-Unit Trucks	1.7	_	-	-	1.7	8.3	1.9	-	-	2.2	33.3	8.3	-	-	13.3	2.2
Articulated Trucks	13	0	0	-	13	5	23	0	-	28	0	3	0	-	3	44
% Articulated Trucks	4.3	_	-	-	4.3	41.7	8.8	-	-	10.3	0.0	25.0	-	-	20.0	7.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	_	-	-	_	-	_	-	-	-	-	_	<u>-</u>	-	_	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	<u>-</u>	0	-	-
% Pedestrians	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-
				,												



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 7



Turning Movement Peak Hour Data Plot (11:45 AM)



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Count Name: Highway 3 & Goshen Road Site Code: 230661 Start Date: 11/28/2023 Page No: 8

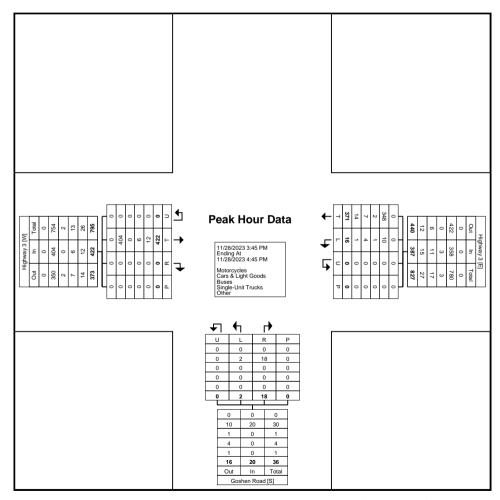
Turning Movement Peak Hour Data (3:45 PM)

					ı anınış	j iviovcii	ilonit i o	ak i loui i	Jaia (J.	. TO 1 101 <i>)</i>						
			Highway 3					Highway 3					Goshen Road			
Start Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
3:45 PM	96	0	. 0	0	96	3	90	0	0	93	1	1	0	0	2	191
4:00 PM	113	0	0	0	113	5	102	0	0	107	1	1	0	0	2	222
4:15 PM	110	0	0	0	110	4	77	0	0	81	0	5	0	0	5	196
4:30 PM	103	. 0	. 0	0	103	4	102	. 0	0	106	0	11	. 0	0	11	220
Total	422	0	0	0	422	16	371	0	0	387	2	18	0	0	20	829
Approach %	100.0	0.0	0.0	-	-	4.1	95.9	0.0	-	-	10.0	90.0	0.0	-	-	-
Total %	50.9	0.0	0.0	-	50.9	1.9	44.8	0.0	-	46.7	0.2	2.2	0.0	-	2.4	-
PHF	0.934	0.000	0.000	-	0.934	0.800	0.909	0.000	-	0.904	0.500	0.409	0.000	-	0.455	0.934
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	_	-	-	0.0	0.0	0.0		-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	404	0	0	-	404	10	348	0	-	358	2	18	0	-	20	782
% Cars & Light Goods	95.7		-	-	95.7	62.5	93.8		-	92.5	100.0	100.0		-	100.0	94.3
Buses	0	0	0	-	0	1	2	. 0	-	3	0	0	0	-	0	3
% Buses	0.0	_	-	-	0.0	6.3	0.5	<u> </u>	-	0.8	0.0	0.0	<u>-</u>	-	0.0	0.4
Single-Unit Trucks	6	0	0	-	6	4	7	0	-	11	0	0	0	-	0	17
% Single-Unit Trucks	1.4			-	1.4	25.0	1.9		-	2.8	0.0	0.0		-	0.0	2.1
Articulated Trucks	12	0	0	-	12	1	14	0	-	15	0	0	0	-	0	27
% Articulated Trucks	2.8	-	-	-	2.8	6.3	3.8	-	-	3.9	0.0	0.0	-	-	0.0	3.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	_	-	-	0.0	0.0	0.0	<u> </u>	-	0.0	0.0	0.0	<u>-</u>	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	_	-	-	-	-	_		-	-	-	-	<u> </u>	-	-	-
Pedestrians	-	<u>-</u>	<u>-</u>	0	-	-	-	<u>-</u>	0	-	-	-	<u>-</u>	0	-	-
% Pedestrians	-	-		-	-	-	-	. -	-	-	-	-	<u>-</u>	-	_	-



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Turning Movement Peak Hour Data Plot (3:45 PM)



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Count Name: Access A Site Code: 230661 Start Date: 11/28/2023

Page No: 1

Turning Movement Data

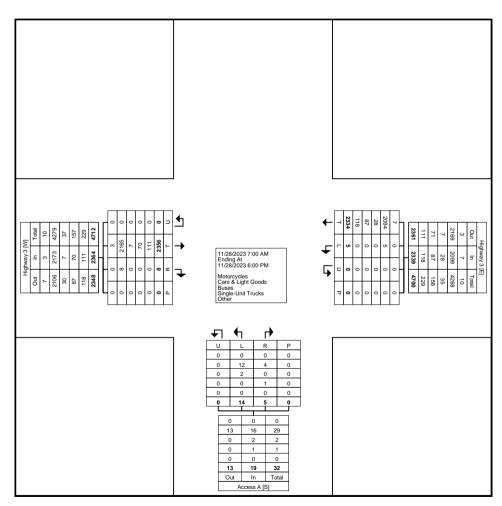
							90		- a.u							1
			Highway 3					Highway 3					Access A			
Otant Time			Eastbound					Westbound					Northbound			
Start Time	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:00 AM	53	0	0	0	53	0	53	0	0	53	0	0	0	0	0	106
7:15 AM	72	0	0	0	72	0	62	0	0	62	0	0	0	0	0	134
7:30 AM	51	0	0	0	51	1	68	0	0	69	0	0	0	0	0	120
7:45 AM	66	3	0	0	69	1	74	0	0	75	0	0	0	0	0	144
Hourly Total	242	3	0	0	245	2	257	0	0	259	0	0	0	0	0	504
8:00 AM	65	0	0	0	65	0	65	0	0	65	0	0	0	0	0	130
8:15 AM	57	0	0	0	57	0	58	0	0	58	0	0	0	0	0	115
8:30 AM	61	1	0	0	62	0	80	0	0	80	0	0	0	0	0	142
8:45 AM	51	0	0	0	51	0	65	0	0	65	0	0	0	0	0	116
Hourly Total	234	. 1	0	0	235	0	268	0	0	268	0	0	0	0	0	503
9:00 AM	46	1	0	0	47	0	55	0	0	55	1	0	0	0	. 1	103
9:15 AM	46	0	0	0	46	0	66	0	0	66	1	0	0	0	1	113
9:30 AM	50	0	0	0	50	0	73	0	0	73	0	0	0	0	0	123
9:45 AM	42	0	0	0	42	0	69	0	0	69	0	0	0	0	0	111
Hourly Total	184	1	0	0	185	0	263	0	0	263	2	0	0	0	2	450
*** BREAK ***	-	_		-	-	-	_		-	-	-	-	-	-	-	-
11:30 AM	67	0	0	0	67	0	67	0	0	67	0	0	0	0	. 0	134
11:45 AM	81	0	0	0	81	0	62	0	0	62	0	0	0	0	0	143
Hourly Total	148	0	0	0	148	0	129	0	0	129	0	0	0	0	0	277
12:00 PM	67	0	0	0	67	1	75	0	0	76	0	0	0	0	0	143
12:15 PM	91	0	0	0	91	0	64	0	0	64	2	1	0	0	3	158
12:30 PM	71	0	0	0	71	1	67	0	0	68	0	0	. 0	0	0	139
12:45 PM	62	0	0	0	62	0	68	0	0	68	0	0	0	0	0	130
Hourly Total	291	0	0	0	291	2	274	0	0	276	2	1	0	0	3	570
1:00 PM	79	0	0	0	79	1	74	0	0	75	0	0	. 0	0	0	154
1:15 PM	57	1	0	0	58	0	72	0	0	72	0	1	0	0	. 1	131
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	136	. 1	0	0	137	1	146	0	0	147	0	1	. 0	0	1	285
3:00 PM	101	0	0	0	101	0	70	0	0	70	2	1	. 0	0	3	174
3:15 PM	72	0	0	0	72	0	82	0	0	82	0	0	0	0	0	154
3:30 PM	97	1	0	0	98	0	82	0	0	82	1	0	. 0	0	1	181
3:45 PM	100	0	0	0	100	0	91	0	0	91	1	0	0	0	. 1	192
Hourly Total	370	1	0	0	371	0	325	0	0	325	4	1	0	0	5	701
4:00 PM	114	0	0	0	114	0	109	0	0	109	1	1	0	0	2	225
4:15 PM	116	1	0	0	117	0	81	0	0	81	0	0	0	0	0	198
4:30 PM	109	0	. 0	0	109	0	104	0	0	104	0	0	0	0	. 0	213

4: 45 DM	00					_	00		0		_					407
4:45 PM	93	0	0	0	93	0	88	0	0	88	5	11	0	0	6	187
Hourly Total	432	1	0	. 0	433	0	382	0	0	382	6	2	0	0	. 8	823
5:00 PM	94	0	0	0	94	0	88	0	0	88	0	0	0	0	0	182
5:15 PM	92	0	0	0	92	0	61	0	0	61	0	0	0	0	0	153
5:30 PM	77	0	0	0	77	0	78	0	0	78	0	0	0	0	0	155
5:45 PM	56	0	0	0	56	0	63	0	0	63	0	0	0	0	0	119
Hourly Total	319	0	0	0	319	0	290	0	0	290	0	0	0	0	0	609
Grand Total	2356	8	0	0	2364	5	2334	0	0	2339	14	5	0	0	19	4722
Approach %	99.7	0.3	0.0	-	-	0.2	99.8	0.0	-	-	73.7	26.3	0.0	-	-	-
Total %	49.9	0.2	0.0	-	50.1	0.1	49.4	0.0	-	49.5	0.3	0.1	0.0	-	0.4	-
Motorcycles	3	0	0	-	3	0	7	0	-	7	0	0	0	-	0	10
% Motorcycles	0.1	0.0	-	-	0.1	0.0	0.3	-	-	0.3	0.0	0.0	-	-	0.0	0.2
Cars & Light Goods	2165	8	0	-	2173	5	2094	0	-	2099	12	4	0	-	16	4288
% Cars & Light Goods	91.9	100.0	-	-	91.9	100.0	89.7	-	-	89.7	85.7	80.0	-	-	84.2	90.8
Buses	7	0	0	-	7	0	28	0	-	28	2	0	0	-	2	37
% Buses	0.3	0.0	-	-	0.3	0.0	1.2	-	-	1.2	14.3	0.0	-	-	10.5	0.8
Single-Unit Trucks	70	0	0	-	70	0	87	0	-	87	0	1	0	-	1	158
% Single-Unit Trucks	3.0	0.0	-	-	3.0	0.0	3.7	-	-	3.7	0.0	20.0	-	-	5.3	3.3
Articulated Trucks	111	0	0	-	111	0	116	0	-	116	0	0	0	-	0	227
% Articulated Trucks	4.7	0.0	-	-	4.7	0.0	5.0	-	-	5.0	0.0	0.0	-	-	0.0	4.8
Bicycles on Road	0	0	0	-	0	0	2	0	-	2	0	0	0	-	0	2
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.1	-	-	0.1	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
				-	-					-					-	-



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Turning Movement Data Plot



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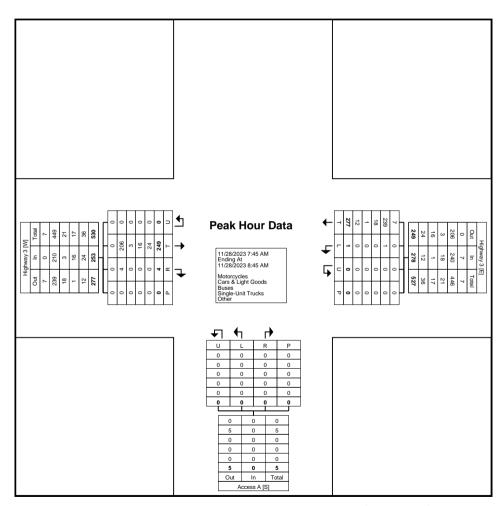
Turning Movement Peak Hour Data (7:45 AM)

,					rumni	j woven	nent Pe	ak nour i	Jaia (7.	.43 AIVI)						
			Highway 3					Highway 3					Access A			
Start Time	Eastbound				Westbound				Northbound							
	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
7:45 AM	66	3	. 0	0	69	1	74	0	0	75	0	. 0	. 0	0	. 0	144
8:00 AM	65	0	0	0	65	0	65	0	0	65	0	0	0	0	0	130
8:15 AM	57	0	0	0	57	0	58	0	0	58	0	0	0	0	0	115
8:30 AM	61	1	0	0	62	0	80	0	0	80	0	0	0	0	0	142
Total	249	4	0	0	253	1	277	0	0	278	0	0	0	0	0	531
Approach %	98.4	1.6	0.0	-	-	0.4	99.6	0.0	-	-	0.0	0.0	0.0	-	-	-
Total %	46.9	0.8	0.0	-	47.6	0.2	52.2	0.0	-	52.4	0.0	0.0	0.0	-	0.0	-
PHF	0.943	0.333	0.000	-	0.917	0.250	0.866	0.000	-	0.869	0.000	0.000	0.000	-	0.000	0.922
Motorcycles	0	0	0	-	0	0	7	0	-	7	0	0	0	-	0	7
% Motorcycles	0.0	0.0	-	-	0.0	0.0	2.5	-	-	2.5	-	_	-	-	-	1.3
Cars & Light Goods	206	4	0	-	210	1	239	0	-	240	0	0	0	-	0	450
% Cars & Light Goods	82.7	100.0	-	-	83.0	100.0	86.3	-	-	86.3	-	-		-	-	84.7
Buses	3	0	0	-	3	0	18	0	-	18	0	0	0	-	0	21
% Buses	1.2	0.0	-	-	1.2	0.0	6.5	-	-	6.5	-	-	-	-	-	4.0
Single-Unit Trucks	16	0	0	-	16	0	1	0	-	1	0	0	0	-	0	17
% Single-Unit Trucks	6.4	0.0	-	-	6.3	0.0	0.4	-	-	0.4	-	-	-	-	-	3.2
Articulated Trucks	24	0	0	-	24	0	12	0	-	12	0	0	0	-	0	36
% Articulated Trucks	9.6	0.0	-	-	9.5	0.0	4.3	-	-	4.3	-	-	-	-	-	6.8
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (7:45 AM)



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Access A Site Code: 230661 Start Date: 11/28/2023 Page No: 6

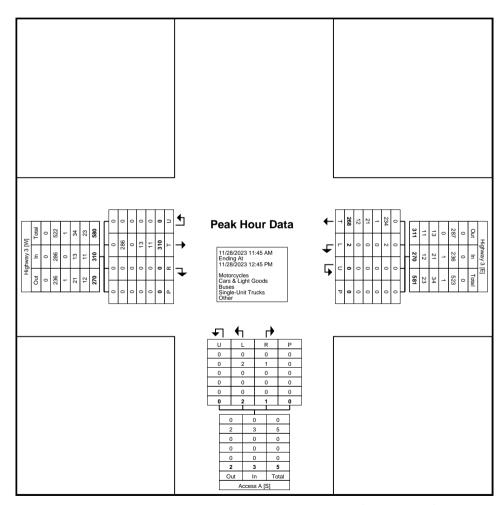
Turning Movement Peak Hour Data (11:45 AM)

Start Time	Int. Total 143 143 158 139 583 - 0.922
Start Time	143 143 158 139 583 - - 0.922
Thru Right U-Turn Peds App. Total Left Thru U-Turn Peds App. Total Left Right U-Turn Peds App. Total 11:45 AM 81 0 0 81 0 62 0 <td>143 143 158 139 583 - - 0.922</td>	143 143 158 139 583 - - 0.922
12:00 PM 67 0 0 0 67 1 75 0 0 76 0 0 0 0 0 1 12:15 PM 91 0 0 0 91 0 64 0 0 64 2 1 0 0 3 12:30 PM 71 0 0 0 71 1 67 0 0 68 0 0 0 0 0 Total 310 0 0 0 310 2 268 0 0 270 2 1 0 0 3 Approach % 100.0 0.0 0.0 - - 0.7 99.3 0.0 - - 66.7 33.3 0.0 - - Total % 53.2 0.0 0.0 - 53.2 0.3 46.0 0.0 - 46.3 0.3 0.2 0.0 -	143 158 139 583 - - 0.922
12:15 PM 91 0 0 91 0 64 0 0 64 2 1 0 0 3 12:30 PM 71 0 0 0 71 1 67 0 0 68 0 3 0	158 139 583 - - 0.922
12:30 PM 71 0 0 0 71 1 67 0 0 68 0 3 Approach % 100.0 0.0 0.0 - - - 0.7 99.3 0.0 - - 66.7 33.3 0.0 - <td>139 583 - - 0.922</td>	139 583 - - 0.922
Total 310 0 0 0 310 2 268 0 0 270 2 1 0 0 3 Approach % 100.0 0.0 0.0 - - 0.7 99.3 0.0 - - 66.7 33.3 0.0 - - Total % 53.2 0.0 0.0 - 53.2 0.3 46.0 0.0 - 46.3 0.3 0.2 0.0 - 0.5 PHF 0.852 0.000 0.802 0.500 0.893 0.000 - 0.888 0.250 0.250 0.000 - 0.250	583 - - 0.922
Approach % 100.0 0.0 0.0 - - 0.7 99.3 0.0 - - 66.7 33.3 0.0 - - Total % 53.2 0.0 0.0 - 53.2 0.3 46.0 0.0 - 46.3 0.3 0.2 0.0 - 0.5 PHF 0.852 0.000 - 0.852 0.500 0.893 0.000 - 0.888 0.250 0.250 0.000 - 0.250	- 0.922
Total % 53.2 0.0 0.0 - 53.2 0.3 46.0 0.0 - 46.3 0.3 0.2 0.0 - 0.5 PHF 0.852 0.000 0.000 - 0.852 0.500 0.893 0.000 - 0.888 0.250 0.250 0.000 - 0.250	0.922
PHF 0.852 0.000 0.000 - 0.852 0.500 0.893 0.000 - 0.888 0.250 0.250 0.000 - 0.250	0.922
Motorcycles 0 0 0 - 0 0 0 0 - 0 0 0 0 - 0	
	0
% Motorcycles 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0
Cars & Light Goods 286 0 0 - 286 2 234 0 - 236 2 1 0 - 3	525
% Cars & Light Goods 92.3 92.3 100.0 87.3 87.4 100.0 100.0 100.0	90.1
Buses 0 0 0 - 0 0 1 0 - 1 0 0 0 - 0	1
% Buses 0.0 0.0 0.0 0.4 0.4 0.0 0.0 0.0	0.2
Single-Unit Trucks 13 0 0 - 13 0 21 0 - 21 0 0 0 - 0	34
% Single-Unit Trucks 4.2 4.2 0.0 7.8 7.8 0.0 0.0 0.0	5.8
Articulated Trucks 11 0 0 - 11 0 12 0 - 12 0 0 0 - 0	23
% Articulated Trucks 3.5 3.5 0.0 4.5 4.4 0.0 0.0 0.0	3.9
Bicycles on Road 0 0 0 - 0 0 0 - 0 0 0 0 - 0	0
% Bicycles on Road 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0
Bicycles on Crosswalk 0 0 0 0 -	-
% Bicycles on Crosswalk	-
Pedestrians 0 0 0 0 0 -	-
% Pedestrians	-



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Turning Movement Peak Hour Data Plot (11:45 AM)



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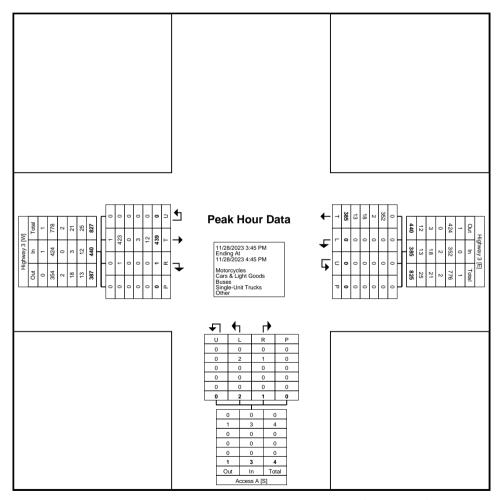
Turning Movement Peak Hour Data (3:45 PM)

Highway 3 Highway 3 Eastbound Westbound							
Monthound				Access A			
Start Time Eastbourid vvestbourid				Northbound			
Thru Right U-Turn Peds App. Total Left Thru U-Turn Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	Int. Total
3:45 PM 100 0 0 0 100 0 91 0 0	91	1	0	0	0	. 1	192
4:00 PM 114 0 0 0 114 0 109 0 0	109	1	1	0	0	2	225
4:15 PM 116 1 0 0 117 0 81 0 0	81	0	0	0	0	0	198
4:30 PM 109 0 0 0 109 0 104 0 0	104	0	0	. 0	0	. 0	213
	385	2	1	0	0	3	828
Approach % 99.8 0.2 0.0 0.0 100.0 0.0 -	-	66.7	33.3	0.0	-	-	-
Total % 53.0 0.1 0.0 - 53.1 0.0 46.5 0.0 -	46.5	0.2	0.1	0.0	-	0.4	-
PHF 0.946 0.250 0.000 - 0.940 0.000 0.883 0.000 -	0.883	0.500	0.250	0.000	-	0.375	0.920
Motorcycles 1 0 0 - 1 0 0 0 -	0	0	0	0	-	0	1
% Motorcycles 0.2 0.0 0.2 - 0.0	0.0	0.0	0.0	<u>-</u>	-	0.0	0.1
Cars & Light Goods 423 1 0 - 424 0 352 0 -	352	2	1	0	-	3	779
% Cars & Light Goods 96.4 100.0 96.4 - 91.4	91.4	100.0	100.0	-	-	100.0	94.1
Buses 0 0 0 - 0 0 2 0 -	2	0	0	0	-	0	2
% Buses 0.0 0.0 0.0 - 0.5	0.5	0.0	0.0	-	-	0.0	0.2
Single-Unit Trucks 3 0 0 - 3 0 18 0 -	18	0	0	0	-	0	21
% Single-Unit Trucks 0.7 0.0 0.7 - 4.7	4.7	0.0	0.0	-	-	0.0	2.5
Articulated Trucks 12 0 0 - 12 0 13 0 -	13	0	0	0	-	0	25
% Articulated Trucks 2.7 0.0 2.7 - 3.4	3.4	0.0	0.0	-	-	0.0	3.0
Bicycles on Road 0 0 0 - 0 0 0 - 0 - 0 0 0	0	0	0	0	-	0	0
% Bicycles on Road 0.0 0.0 0.0 - 0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk 0 0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	<u>-</u>	-
Pedestrians 0 0	-	-	-	-	0	<u>-</u>	-
% Pedestrians	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (3:45 PM)



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Count Name: Bilger Access Site Code: 230661 Start Date: 11/28/2023

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Turning Movement Data

	1						90		- 4.64							
			Bilger Access					Goshen Road					Goshen Road			
Start Time			Westbound					Northbound					Southbound			
Start Time	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
7:00 AM	0	0	0	0	0	2	0	0	0	2	1	3	0	0	4	6
7:15 AM	0	0	0	0	0	4	0	0	0	4	1	1	0	0	2	6
7:30 AM	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7
7:45 AM	0	0	0	0	0	7	0	0	0	7	0	1	0	0	1	8
Hourly Total	0	0	0	0	0	17	0	0	0	17	2	8	0	0	10	27
8:00 AM	0	0	0	0	0	1	0	0	0	1	0	1	1	0	2	3
8:15 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	7	0	0	0	7	2	1	0	0	3	10
8:45 AM	1	0	0	0	1	5	0	0	0	5	0	3	0	0	3	9
Hourly Total	1	0	0	0	1	14	0	0	0	14	2	5	1	0	8	23
9:00 AM	0	2	0	0	2	2	0	0	0	2	0	0	0	0	0	4
9:15 AM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
9:30 AM	1	0	0	0	1	3	0	0	0	3	1	1	0	0	2	6
9:45 AM	1	0	0	0	1	1	0	0	0	1	1	4	0	0	5	7
Hourly Total	2	2	0	0	4	7	0	0	0	7	2	6	0	0	8	19
*** BREAK ***	-	-	-	-	-	-	_	-	-	-	-	<u>-</u>	-	-	-	-
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
11:45 AM	0	0	0	0	0	4	0	0	0	4	0	2	0	0	2	6
Hourly Total	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7
12:00 PM	0	0	0	0	0	3	0	0	0	3	2	1	0	0	3	6
12:15 PM	0	2	0	0	2	3	0	0	0	3	0	3	0	0	3	8
12:30 PM	0	0	0	0	0	4	0	0	0	4	0	4	0	0	4	8
12:45 PM	0	0	0	0	0	1	0	0	0	. 1	0	3	0	0	3	4
Hourly Total	0	2	0	0	2	11	0	0	0	11	2	11	0	0	13	26
1:00 PM	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	2
1:15 PM	0	0	0	0	0	3	0	0	0	3	0	1	0	0	. 1	4
*** BREAK ***	-	-	-	-	-	-			-	-	-		-	-	-	-
Hourly Total	0	0	0	0	0	5	0	0	0	5	0	1	0	0	1	6
3:00 PM	0	. 1	. 0	0	. 1	3	0	0	0	. 3	2	0	0	0	2	6
3:15 PM	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	2
3:30 PM	0	0	. 0	0	0	2	0	0	0	2	0	1	0	0	1	3
3:45 PM	0	1	0	0	1	1	0	0	0	. 1	2	1	0	0	3	5
Hourly Total	0	2	0	0	2	8	0	0	0	8	4	2	0	0	6	16
4:00 PM	1	1	0	0	2	1	0	0	0	1	0	0	0	0	0	3
4:15 PM	0	0	0	0	0	5	3	0	0	8	1	3	0	0	4	12
4:30 PM	0	0	. 0	0	0	11	0	0	0	. 11	1	2	. 0	0	3	14

4:45 PM	0	3	0	0	3	3	0	0	0	3	1	3	0	0	4	10
Hourly Total	1	4	0	0	5	20	3	0	0	23	3	8	0	0	11	39
5:00 PM	0	2	0	0	2	0	0	0	0	0	0	1	0	0	1	3
5:15 PM	0	0	0	0	0	2	0	0	0	2	1	2	0	0	3	5
5:30 PM	1	0	0	0	1	0	0	0	0	0	2	2	0	0	4	5
5:45 PM	0	1	0	0	1	4	0	0	0	4	2	2	0	0	4	9
Hourly Total	1	3	0	0	4	6	0	0	0	6	5	7	0	0	12	22
Grand Total	5	13	0	0	18	92	3	0	0	95	20	51	1	0	72	185
Approach %	27.8	72.2	0.0	-	<u>-</u>	96.8	3.2	0.0	-	-	27.8	70.8	1.4	-	-	-
Total %	2.7	7.0	0.0	-	9.7	49.7	1.6	0.0	-	51.4	10.8	27.6	0.5	-	38.9	-
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Cars & Light Goods	4	10	0	-	14	77	3	0	-	80	9	38	1	-	48	142
% Cars & Light Goods	80.0	76.9	-	-	77.8	83.7	100.0	-	-	84.2	45.0	74.5	100.0	-	66.7	76.8
Buses	0	0	0	-	0	2	0	0	-	2	0	3	0	-	3	5
% Buses	0.0	0.0	-	-	0.0	2.2	0.0	-	-	2.1	0.0	5.9	0.0	-	4.2	2.7
Single-Unit Trucks	1	2	0	-	3	10	0	0	-	10	9	6	0	-	15	28
% Single-Unit Trucks	20.0	15.4	-	-	16.7	10.9	0.0	-	-	10.5	45.0	11.8	0.0	-	20.8	15.1
Articulated Trucks	0	1	0	-	1	3	0	0	-	3	2	4	0	-	6	10
% Articulated Trucks	0.0	7.7	-	-	5.6	3.3	0.0	-	-	3.2	10.0	7.8	0.0	-	8.3	5.4
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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 Goshen Road [N]

 Out
 In
 Total
 L U P 11/28/2023 7:00 AM Ending At 11/28/2023 6:00 PM Motorcycles Cars & Light Goods Buses Single-Unit Trucks Other 56 95 151 Goshen Road [S]

Turning Movement Data Plot



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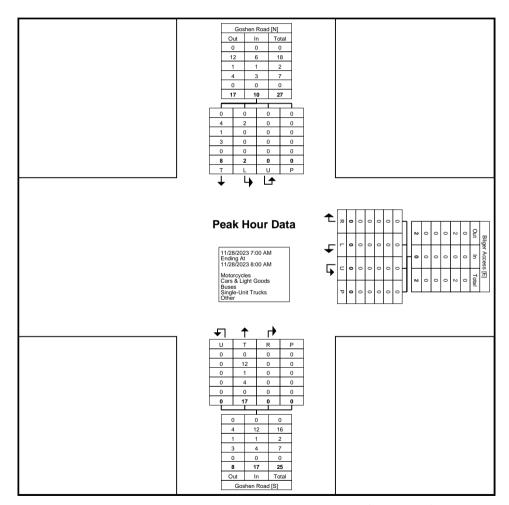
Turning Movement Peak Hour Data (7:00 AM)

					runni	i ivioveii	ICIIL I C	ak moui i	Jaia (1.	.00 AIVI) _,						ı
			Bilger Access					Goshen Road					Goshen Road			
Start Time			Westbound					Northbound					Southbound			
Start Time	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
7:00 AM	0	0	0	0	0	2	0	0	0	2	1	3	0	0	4	6
7:15 AM	0	0	0	0	0	4	0	0	0	4	1	1	0	0	2	6
7:30 AM	0	0	0	0	0	4	0	0	0	4	0	3	0	0	3	7
7:45 AM	0	0	0	0	0	7	0	0	0	7	0	1	0	0	1	8
Total	0	0	0	0	0	17	0	0	0	17	2	8	0	0	10	27
Approach %	0.0	0.0	0.0	-	-	100.0	0.0	0.0	-	-	20.0	80.0	0.0	-	-	-
Total %	0.0	0.0	0.0	-	0.0	63.0	0.0	0.0	-	63.0	7.4	29.6	0.0	-	37.0	-
PHF	0.000	0.000	0.000	-	0.000	0.607	0.000	0.000	-	0.607	0.500	0.667	0.000	-	0.625	0.844
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	-	_	-	-	-	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	0	0	0	-	0	12	0	0	-	12	2	4	0	-	6	18
% Cars & Light Goods	-	-	-	-	-	70.6	-	-	-	70.6	100.0	50.0	-	-	60.0	66.7
Buses	0	0	0	-	0	1	0	0	-	1	0	1	0	-	1	2
% Buses	-	-	-	-	-	5.9	-	-	-	5.9	0.0	12.5	-	-	10.0	7.4
Single-Unit Trucks	0	0	0	-	0	4	0	0	-	4	0	3	0	-	3	7
% Single-Unit Trucks	-	<u>-</u>	-	-	-	23.5	-	-	-	23.5	0.0	37.5	-	-	30.0	25.9
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	-	-	-	-	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	-	-	-	-	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	i	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-



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Turning Movement Peak Hour Data Plot (7:00 AM)



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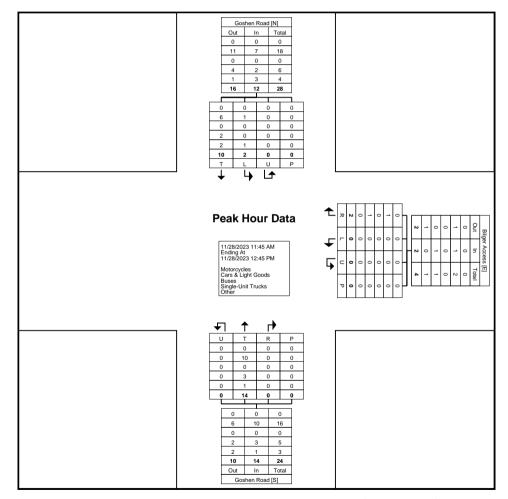
Turning Movement Peak Hour Data (11:45 AM)

	1				i urning	iviovem	ient Pea	k Hour L)ata (11	:45 AM)						1
			Bilger Access					Goshen Road					Goshen Road			1
Start Time			Westbound					Northbound					Southbound			1
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
11:45 AM	0	. 0	0	0	. 0	4	. 0	. 0	0	4	0	2	. 0	0	. 2	6
12:00 PM	0	0	0	0	0	3	0	0	0	3	2	1	0	0	3	6
12:15 PM	0	2	0	0	2	3	0	0	0	3	0	3	0	0	3	8
12:30 PM	0	. 0	0	0	. 0	4	. 0	. 0	0	4	0	. 4	. 0	0	. 4	8
Total	0	2	0	0	2	14	0	0	0	14	2	10	0	0	12	28
Approach %	0.0	100.0	0.0	-	-	100.0	0.0	0.0	-	-	16.7	83.3	0.0	-	-	-
Total %	0.0	7.1	0.0	-	7.1	50.0	0.0	0.0	-	50.0	7.1	35.7	0.0	-	42.9	-
PHF	0.000	0.250	0.000	-	0.250	0.875	0.000	0.000	-	0.875	0.250	0.625	0.000	-	0.750	0.875
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	-	0.0		-	0.0	0.0			-	0.0	0.0	0.0		-	0.0	0.0
Cars & Light Goods	0	1	0	-	1	10	0	0	-	10	1	6	0	-	7	18
% Cars & Light Goods	-	50.0	-	-	50.0	71.4		-	-	71.4	50.0	60.0	-	-	58.3	64.3
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	-	0.0	-	-	0.0	0.0	_	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	1	0	-	1	3	0	0	-	3	0	2	0	-	2	6
% Single-Unit Trucks	-	50.0		-	50.0	21.4			-	21.4	0.0	20.0		-	16.7	21.4
Articulated Trucks	0	0	0	-	0	1	0	0	-	1	1	2	0	-	3	4
% Articulated Trucks	-	0.0	-	-	0.0	7.1	_	-	-	7.1	50.0	20.0	-	-	25.0	14.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	ı	-	-	-	-	-	-	-	-	-	-



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Turning Movement Peak Hour Data Plot (11:45 AM)



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Turning Movement Peak Hour Data (4:00 PM)

ı					Turning	j ivioven	nent Pea	ak Hour I	Jata (4)	(UU PIVI)						İ
			Bilger Access					Goshen Road					Goshen Road			
Start Time			Westbound					Northbound					Southbound			
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Int. Total
4:00 PM	1	1	. 0	0	2	1	0	. 0	0	. 1	0	0	0	0	. 0	3
4:15 PM	0	0	0	0	0	5	3	0	0	8	1	3	0	0	4	12
4:30 PM	0	0	0	0	0	11	0	0	0	11	1	2	0	0	3	14
4:45 PM	0	3	. 0	0	3	3	0	. 0	0	3	1	3	. 0	0	. 4	10
Total	1	4	0	0	5	20	3	0	0	23	3	8	0	0	11	39
Approach %	20.0	80.0	0.0	-	-	87.0	13.0	0.0	-	-	27.3	72.7	0.0	-	-	-
Total %	2.6	10.3	0.0	-	12.8	51.3	7.7	0.0	-	59.0	7.7	20.5	0.0	-	28.2	
PHF	0.250	0.333	0.000	-	0.417	0.455	0.250	0.000	-	0.523	0.750	0.667	0.000	-	0.688	0.696
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	0	3	0	-	3	20	3	0	-	23	0	8	0	-	8	34
% Cars & Light Goods	0.0	75.0	-	-	60.0	100.0	100.0	-	-	100.0	0.0	100.0	-	-	72.7	87.2
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	1	1	0	-	2	0	0	0	-	0	3	0	0	-	3	5
% Single-Unit Trucks	100.0	25.0	-	-	40.0	0.0	0.0	-	-	0.0	100.0	0.0	-	-	27.3	12.8
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-		0	-	-	-	-	0	_	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	_	-	-	-	-	_	-	-	-	-	_	-	-	-



Cambridge, Ontario, Canada N1R 8J8 519-896-3163 cbowness@ptsl.com

Count Name: Bilger Access Site Code: 230661 Start Date: 11/28/2023 Page No: 9

L U P **Peak Hour Data** 11/28/2023 4:00 PM Ending At 11/28/2023 5:00 PM Motorcycles Cars & Light Goods Buses Single-Unit Trucks Other 9 23 32 Goshen Road [S]

Turning Movement Peak Hour Data Plot (4:00 PM)

Appendix C

Existing Traffic Operations Reports

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	•	7	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	Y	
Traffic Volume (vph)	233	1	5	273	1	18
Future Volume (vph)	233	1	5	273	1	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1666	0	0	1787	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1666	0	0	1787	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	253	1	5	297	1	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	254	0	0	302	21	0
Sign Control	Free			Free	Stop	
l-t						

Intersection Summary Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.4%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	233	1	5	273	1	18
Future Vol. veh/h	233	1	5	273	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	
Storage Length		-		-	0	-
Veh in Median Storage	e.# 0	_	_	0	0	_
Grade. %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	253	1	5	297	1	20
WWIT Flow	253	- 1	0	297	- 1	20
	Major1	1	Major2	1	Minor1	
Conflicting Flow All	0	0	254	0	561	254
Stage 1	-	-	-	-	254	-
Stage 2	-	-	-	-	307	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-		3.453
Pot Cap-1 Maneuver	_	-	1213	-	492	749
Stage 1	-	-	-	-	793	
Stage 2	_	_	_	-	751	-
Platoon blocked, %		-		-	701	
Mov Cap-1 Maneuver			1213	-	490	749
Mov Cap-1 Maneuver	-		1213	-	490	149
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	747	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.1	
HCM LOS	U		0.1		В	
TIOW LOO						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		729	-	-	1213	-
HCM Lane V/C Ratio		0.028	-	-	0.004	-
HCM Control Delay (s)	10.1	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh	1)	0.1	-	-	0	-
	,					

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	*	•	•	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.			ર્ન	¥		
Traffic Volume (vph)	249	4	1	277	0	0	
Future Volume (vph)	249	4	1	277	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.998						
Flt Protected							
Satd. Flow (prot)	1638	0	0	1810	1900	0	
Flt Permitted							
Satd. Flow (perm)	1638	0	0	1810	1900	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%	
Adj. Flow (vph)	271	4	1	301	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	275	0	0	302	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary							

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 18.7%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3

Paradigm Transportation Solutions Limited

Existing AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	¥	
Traffic Vol, veh/h	249	4	1	277	0	0
Future Vol, veh/h	249	4	1	277	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	271	4	1	301	0	0
WWITETIOW	211	7		001	U	U
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	275	0	576	273
Stage 1	-	-	-	-	273	-
Stage 2	-	-	-	-	303	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1300	-	482	771
Stage 1	-	-	-	-	778	-
Stage 2	-	-	-	-	754	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1300	-	482	771
Mov Cap-2 Maneuver	-	-	-	-	482	-
Stage 1	-	_	_	_	778	-
Stage 2	-	-	-	-	753	-
5.030 L						
Approach	EB		WB		NB	
	0					
HCM Control Delay, s	U		0		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1300	-
HCM Lane V/C Ratio		-	-	-	0.001	-
HCM Control Delay (s)		0	_	_	7.8	0
HCM Lane LOS		A	-	-	Α.	A
HCM 95th %tile Q(veh)		-	_	_	0	-
TIOM JOHN JUHIC Q(VEII)					J	

Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	422	0	16	371	2	18
Future Volume (vph)	422	0	16	371	2	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.877	
Flt Protected				0.998	0.995	
Satd. Flow (prot)	1827	0	0	1772	1658	0
Flt Permitted				0.998	0.995	
Satd. Flow (perm)	1827	0	0	1772	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	459	0	17	403	2	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	459	0	0	420	22	0
Sign Control	Free			Free	Stop	

Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 42.5%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	,,,,,,
Traffic Vol, veh/h	422	0	16	371	2	18
Future Vol. veh/h	422	0	16	371	2	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		riee -	None	Stop -	None
Storage Length		NOTICE -	-	NOTICE -	0	NOHE
Veh in Median Storage	e. # 0	-	-	0	0	
Grade, %	0		-	0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mvmt Flow	459	0	17	403	2	20
Major/Minor I	Major1	1	Major2	1	Minor1	
Conflicting Flow All	0	0	459	0	896	459
Stage 1	-	-	-	-	459	-
Stage 2	-	-	-	-	437	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	_	-	_	5.4	_
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	965	_	313	606
Stage 1			300		641	-
Stage 2	-	-		-	655	
Platoon blocked, %			_		055	_
	-	-	005	-	200	000
Mov Cap-1 Maneuver	-	-	965	-	306	606
Mov Cap-2 Maneuver	-	-	-	-	306	-
Stage 1	-	-	-	-	641	-
Stage 2	-	-	-	-	640	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.8	
HCM LOS	U		0.4		11.0 B	
HUM LUS					В	
Minor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		552	-	-	965	-
HCM Lane V/C Ratio		0.039			0.018	
HCM Control Delay (s)		11.8	_	-	8.8	0
HCM Lane LOS		В	-	-	A	A
HCM 95th %tile Q(veh)	١	0.1	_	_	0.1	

Page 1

Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	€	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			ર્ન	Y	
Traffic Volume (vph)	439	1	0	385	2	1
Future Volume (vph)	439	1	0	385	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	477	1	0	418	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	478	0	0	418	3	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 33.2%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3 Existing PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.1					
• •	EDT	EDD	WDI	WDZ	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	¥	
Traffic Vol, veh/h	439	1	0	385	2	1
Future Vol, veh/h	439	1	0	385	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	477	1	0	418	2	1
					_	
	lajor1		/lajor2		Minor1	
Conflicting Flow All	0	0	478	0	896	478
Stage 1	-	-	-	-	478	-
Stage 2	-	-	-	-	418	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	_	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	1095	_	313	591
Stage 1			-	-	628	-
Stage 2	-	_	_	_	669	-
Platoon blocked, %	-				000	
Mov Cap-1 Maneuver	-	-	1095	-	313	591
					313	- 180
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	628	-
Stage 2	-	-	-	-	669	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		14.8	
HCM LOS	U		0		14.0 B	
I IOWI LUO					٥	
Minor Lane/Major Mvmt	I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		371	-	-	1095	-
HCM Lane V/C Ratio		0.009			-	
HCM Control Delay (s)		14.8	_	-	0	-
HCM Lane LOS		В	-		A	
HCM 95th %tile Q(veh)		0	-		0	-
HOW SOUL WILL CA (VELL)		U		_	U	

Appendix D

2024 Background Traffic Operations Reports

2024 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

2024 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Lane Group EBT EBR WBL WBT NBL NBR Lane Configurations ♣ ♣ ♣ ▼ ★ ▼ ▼ ■ 1 18 ■ 1 18 ■ 1 18 ■ 1 18 ■ 1 18 ■ ■ ■ 1 18 ■ ■ ■ 1 18 ■
Traffic Volume (vph) 238 1 5 278 1 18 Future Volume (vph) 238 1 5 278 1 18 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00
Future Volume (vph) 238 1 5 278 1 18 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 1900 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00
E
Frt 0.999 0.871
Flt Protected 0.999 0.998
Satd. Flow (prot) 1666 0 0 1787 1421 0
Flt Permitted 0.999 0.998
Satd. Flow (perm) 1666 0 0 1787 1421 0
Link Speed (k/h) 50 50
Link Distance (m) 256.2 158.5 186.1
Travel Time (s) 18.4 11.4 13.4
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92
Heavy Vehicles (%) 14% 0% 20% 6% 0% 17%
Adj. Flow (vph) 259 1 5 302 1 20
Shared Lane Traffic (%)
Lane Group Flow (vph) 260 0 0 307 21 0
Sign Control Free Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.6% ICU Level of Service A
Analysis Period (min) 15

Inters	section						
	elay, s/veh	0.4					
Movo	ement	EBT	EBR	WBL	WBT	NBL	NBR
			EDI	WDL	₩DI	NDL W	NDI
	Configurations	}	4				40
	ic Vol, veh/h	238	1	5	278	1	18
	e Vol, veh/h	238	1	5	278	1	18
	licting Peds, #/hr	_ 0	_ 0	0	0	0	0
	Control	Free	Free	Free	Free	Stop	Stop
	hannelized	-		-	None	-	None
	ige Length	-	-	-	-	0	-
	n Median Storage		-	-	0	0	-
Grade	-,	0	-	-	0	0	-
	Hour Factor	92	92	92	92	92	92
Heav	y Vehicles, %	14	0	20	6	0	17
Mvmt	t Flow	259	1	5	302	1	20
Majar	r/Minor	Major1		Aniar?		Minor1	
		Major1 0	0	Major2 260	0	572	260
Conti	licting Flow All						
	Stage 1	-	-	-	-	260	-
	Stage 2	-	-	-	-	312	-
	al Hdwy	-	-	4.3	-	6.4	6.37
	al Hdwy Stg 1	-	-	-	-	5.4	-
	al Hdwy Stg 2	-	-	-	-	5.4	-
Follov	w-up Hdwy	-	-	2.38	-	3.5	3.453
Pot C	Cap-1 Maneuver	-	-	1207	-	485	744
	Stage 1	-	-	-	-	788	-
	Stage 2	-	-	_	-	747	-
Plato	on blocked. %	-	-		-		
Mov (Cap-1 Maneuver	-	-	1207	-	483	744
	Cap-2 Maneuver	-	-	-	-	483	-
	Stage 1	-	_	_	-	788	_
	Stage 2	-				743	
	Staye 2					140	
Appro		EB		WB		NB	
HCM	Control Delay, s	0		0.1		10.1	
HCM	LOS					В	
Minor	r Lane/Major Mvm	,	NBLn1	EBT	EBR	WBL	WBT
		it i					
	city (veh/h)		723	-	-	1207	-
	Lane V/C Ratio		0.029	-		0.005	-
	Control Delay (s)		10.1	-	-	8	0
	Lane LOS		В	-	-	Α	Α
HCM	95th %tile Q(veh)	0.1	-	-	0	-

Synchro 11 Report Page 1

	-	\rightarrow	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^			ર્ન	Y	
Traffic Volume (vph)	254	4	1	283	0	0
Future Volume (vph)	254	4	1	283	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1638	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1638	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	276	4	1	308	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	280	0	0	309	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 19.0%			IC	CU Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
			14/51	14/DT	N.D.	ND.
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			4	Y	
Traffic Vol, veh/h	254	4	1	283	0	0
Future Vol, veh/h	254	4	1	283	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mvmt Flow	276	4	1	308	0	0
Major/Minor N	Major1		Major2		Minor1	
Conflicting Flow All	0	0	280	0	588	278
Stage 1	U	·	200	-	278	210
Stage 2		-			310	_
Critical Hdwy	-	-	4.1		6.4	6.2
			4.1	-	5.4	0.2
Critical Hdwy Stg 1	-				5.4	-
Critical Hdwy Stg 2	-	-	-	-		
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1294	-	475	766
Stage 1	-	-	-	-	774	-
Stage 2	-	-	-	-	748	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1294	-	475	766
Mov Cap-2 Maneuver	-	-	-	-	475	-
Stage 1	-	-	-	_	774	-
Stage 2	-	-	-	-	747	-
Annraaah	EB		WB		NB	
Approach	0		0		0	
HCM Control Delay, s	0		0		•	
HCM LOS					Α	
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-		-	1294	-
HCM Lane V/C Ratio				_	7.8	0
HCM Lane V/C Ratio		0	_			
HCM Control Delay (s)		0 A	-			_
		0 A	-	-	7.0 A	A

HCM 6th TWSC

2: Access A & Highway 3

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2024 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	→	*	1	•	7	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	430	0	16	378	2	18
Future Volume (vph)	430	0	16	378	2	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.877	
Flt Protected				0.998	0.995	
Satd. Flow (prot)	1827	0	0	1772	1658	0
Flt Permitted				0.998	0.995	
Satd. Flow (perm)	1827	0	0	1772	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	467	0	17	411	2	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	467	0	0	428	22	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 42.9%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2024 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized	0.5 EBT \$	EBR	WBL	MOT		
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized	(1	EBR	WBL	MOT		
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized	(1			WBT	NBL	NBR
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized				4	¥	
Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized		0	16	378	2	18
Conflicting Peds, #/hr Sign Control RT Channelized	430	0	16	378	2	18
Sign Control RT Channelized	0	0	0	0	0	0
RT Channelized	Free	Free	Free	Free	Stop	Stop
	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e.# 0	_	-	0	0	_
Grade, %	0			0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	467	0	17	411	2	20
IVIVIIIL I IOW	401	U	17	411		20
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	467	0	912	467
Stage 1	-	-	-	-	467	-
Stage 2	-	-	-	-	445	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	958	-	307	600
Stage 1	-	-	-	-	635	-
Stage 2	-	-	-	-	650	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	958	-	300	600
Mov Cap-2 Maneuver	-	-	-	-	300	-
Stage 1	_	_	_	_	635	-
Stage 2	-				635	
Stage 2					000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.9	
HCM LOS					В	
Minor Lane/Major Mvn	nt N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	110 1	545	LDI	LDIX	958	-
HCM Lane V/C Ratio		0.04			0.018	
	,		-		8.8	-
HCM Control Delay (s)	11.9	-	-		0
HCM Lane LOS HCM 95th %tile Q(veh	,	В	-	-	A	Α
	1)	0.1	-	-	0.1	-

	-	•	•	-	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	¥	
Traffic Volume (vph)	448	1	0	393	2	1
Future Volume (vph)	448	1	0	393	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	487	1	0	427	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	488	0	0	427	3	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 33.6%			IC	CU Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
•			MD:	MIDT	ND	NDC
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	448	1	0	393	2	1
Future Vol, veh/h	448	1	0	393	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mymt Flow	487	1	0	427	2	1
WWW.CTIOW	101		U	121	_	
Major/Minor N	Major1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	488	0	915	488
Stage 1	-	-	-	-	488	-
Stage 2	-	-	-	-	427	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1		-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy		-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver		-	1086	-	305	584
Stage 1			-	-	621	-
Stage 2		_		_	662	_
Platoon blocked. %	-	-	_	-	002	_
Mov Cap-1 Maneuver			1086	-	305	584
Mov Cap-1 Maneuver			1000		305	304
			-			
Stage 1	-	-	-	-	621	-
Stage 2	-	-	-	-	662	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		15	
HCM LOS	U		U		C	
HOW LOS					U	
Minor Lane/Major Mvm	it I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		363	-	-	1086	-
HCM Lane V/C Ratio		0.009		-	-	-
HCM Control Delay (s)		15	-	-	0	_
HCM Lane LOS		C			A	
HCM 95th %tile Q(veh)	١	0	_	_	0	_
TION JOHN JUNE Q(VEII)		0			0	

HCM 6th TWSC

2: Access A & Highway 3

Appendix E

2024 Total Traffic Operations Reports

2024 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

Page 1

	-	•	•	•	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			ર્ન	¥		
Traffic Volume (vph)	242	1	5	278	1	18	
Future Volume (vph)	242	1	5	278	1	18	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.999				0.871		
Flt Protected				0.999	0.998		
Satd. Flow (prot)	1666	0	0	1787	1421	0	
Flt Permitted				0.999	0.998		
Satd. Flow (perm)	1666	0	0	1787	1421	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	256.2			158.5	186.1		
Travel Time (s)	18.4			11.4	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%	
Adj. Flow (vph)	263	1	5	302	1	20	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	264	0	0	307	21	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize							
Intersection Capacity Utili	zation 28.6%			IC	CU Level	of Service A	Α
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	LDIN	TTDL	4	¥	HUIT
Traffic Vol, veh/h	242	1	5	278	1	18
Future Vol. veh/h	242	1	5	278	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length		None -	-	INOHE -	0	NOHE -
Veh in Median Storage,		-		0	0	-
Grade, %	# 0	-	-	0	0	-
	92			92	-	
Peak Hour Factor		92	92		92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mvmt Flow	263	1	5	302	1	20
Major/Minor N	lajor1	N	Major2	. 1	Minor1	
Conflicting Flow All	0	0	264	0	576	264
Stage 1	-	-		-	264	
Stage 2	-		-	-	312	-
Critical Hdwy			4.3	-	6.4	6.37
Critical Hdwy Stg 1			4.5		5.4	0.01
Critical Hdwy Stg 2					5.4	-
Follow-up Hdwy			2.38			3.453
Pot Cap-1 Maneuver			1203	_	482	740
Stage 1		_	1203		785	740
Stage 2	-	-		-	747	-
Platoon blocked. %		-		-	141	
	-		1203		480	740
Mov Cap-1 Maneuver	-	-		-		
Mov Cap-2 Maneuver	-	-	-	-	480	-
Stage 1	-	-	-	-	785	-
Stage 2	-	-	-	-	743	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.2	
HCM LOS			0.1		В	
TIOM EGO						
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		719	-	-	1203	-
HCM Lane V/C Ratio		0.029	-	-	0.005	-
HCM Control Delay (s)		10.2	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.1	-	-	0	-
,						

2024 Total AM Peak Hour

(230661) 494 Highway 3, Courtland TIS

	-	•	•	←	1	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	^			ર્ન	Y		
Traffic Volume (vph)	254	8	2	283	0	0	
Future Volume (vph)	254	8	2	283	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.996						
Flt Protected							
Satd. Flow (prot)	1639	0	0	1810	1900	0	
Flt Permitted							
Satd. Flow (perm)	1639	0	0	1810	1900	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%	
Adj. Flow (vph)	276	9	2	308	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	285	0	0	310	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 19.8%			IC	CU Level of	of Service	èΑ
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	¥	
Traffic Vol, veh/h	254	8	2	283	0	0
Future Vol, veh/h	254	8	2	283	0	0
Conflicting Peds, #/hr	204	0	0	203	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length		None -		None -	0	None -
Veh in Median Storage,		-	-	0	0	-
	# 0		-	0	0	-
Grade, %	-	-		-	-	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mvmt Flow	276	9	2	308	0	0
Major/Minor N	lajor1	ı	Major2		Minor1	
Conflicting Flow All	0	0	285	0	593	281
Stage 1	-	-		-	281	-
Stage 2					312	
Critical Hdwy	-		4.1	-	6.4	6.2
		_	4.1		5.4	
Critical Hdwy Stg 1	-	-		-		-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1289	-	472	763
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	747	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1289	-	471	763
Mov Cap-2 Maneuver	-	-	-	-	471	-
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	746	-
Annroach	EB		WB		ND	
Approach					NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	- LDIX	1289	-
					0.002	-
HCM Cantral Dalay (a)		-			7.8	
HCM Control Delay (s)		0	-	-		0
HCM Lane LOS		Α	-	-	A	Α
HCM 95th %tile Q(veh)		-	-	-	0	-

Paradigm Transportation Solutions Limited

HCM 6th TWSC

3: Access B & Highway 3

Paradigm Transportation Solutions Limited

	-	•	•	•	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			र्स	¥	
Traffic Volume (vph)	254	0	2	285	0	0
Future Volume (vph)	254	0	2	285	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1863	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1863	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	276	0	2	310	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	276	0	0	312	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 19.9%			IC	U Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
	• • • •					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	Y	
Traffic Vol, veh/h	254	0	2	285	0	0
Future Vol, veh/h	254	0	2	285	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	_	0	0	_
Grade. %	. 0			0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	276	0	2	310	0	0
Major/Minor N	lajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	276	0	590	276
Stage 1	-	U	210	-	276	210
		-		-		-
Stage 2	-	-	- 40		314	
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1287	-	470	763
Stage 1	-	-	-	-	771	-
Stage 2		_	_	_	741	-
Platoon blocked. %			_		741	_
	-	-	4007	-	400	700
Mov Cap-1 Maneuver			1287		469	763
Mov Cap-2 Maneuver	-	-	-	-	469	-
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	740	-
A	EB		WD		ND	
Approach			WB		NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minar Lana/Maiar Muma	. ,	UDI 51	FDT	EDD	WDI	WBT
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	
Capacity (veh/h)		-	-	-	1287	-
HCM Lane V/C Ratio		-	-	-	0.002	-
HCM Control Delay (s)		0	-	-	7.8	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)		-	_	-	0	_
σσαι /σαισ α(νοιι)						

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	*	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ની	Y	
Traffic Volume (vph)	431	0	17	380	2	21
Future Volume (vph)	431	0	17	380	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.876	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1658	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	468	0	18	413	2	23
Shared Lane Traffic (%)						
Lane Group Flow (vph)	468	0	0	431	25	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 43.8%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

Paradigm Transportation Solutions Limited

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
•		EDD	MD	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	431	0	17	380	2	21
Future Vol, veh/h	431	0	17	380	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	468	0	18	413	2	23
IVIVIIIL FIUW	400	U	10	413	2	23
Major/Minor I	Major1	I	Major2	- 1	Minor1	
Conflicting Flow All	0	0	468	0	917	468
Stage 1	_	_	_	_	468	_
Stage 2	-	-		-	449	
Critical Hdwy	_	-	4.41	_	6.4	6.2
Critical Hdwy Stg 1				-	5.4	0.2
Critical Hdwy Stg 2					5.4	
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
					304	599
Pot Cap-1 Maneuver	-	-	958	-		
Stage 1	-	-	-	-	634	-
Stage 2	-	-	-	-	647	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	958	-	297	599
Mov Cap-2 Maneuver	-	-	-	-	297	-
Stage 1	-	-	-	-	634	-
Stage 2	-	-	-	-	631	-
olago 2						
			1475			
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		11.9	
HCM LOS					В	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
	it I				958	
Capacity (veh/h)		550	-	-		-
HCM Lane V/C Ratio		0.045	-		0.019	-
HCM Control Delay (s)		11.9	-	-	8.8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh))	0.1	-	-	0.1	-

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	€	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	451	2	0	394	4	2
Future Volume (vph)	451	2	0	394	4	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1843	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1843	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	490	2	0	428	4	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	492	0	0	428	6	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 33.9%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 2: Access A & Highway 3

2024 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Intersection Int Delay, s/veh	0.1					
•						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			ની	¥	
Traffic Vol, veh/h	451	2	0	394	4	2
Future Vol, veh/h	451	2	0	394	4	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	490	2	0	428	4	2
Major/Minor	Major1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	492	0	919	491
Stage 1	-	-		-	491	
Stage 2			-		428	
Critical Hdwy	-	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1					5.4	- 0.2
Critical Hdwy Stg 2		_		_	5.4	_
Follow-up Hdwy		-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	1082	_	304	582
Stage 1	-	-	-	-	619	-
Stage 2	-	-	_	-	662	-
Platoon blocked, %		-		-		
Mov Cap-1 Maneuver	-	-	1082	-	304	582
Mov Cap-2 Maneuver	-	-	-	-	304	-
Stage 1	-	-	-	-	619	-
Stage 2		-	-	-	662	-
·						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		15.1	
HCM LOS	U		U		13.1 C	
IICWI LOS					U	
Minor Lane/Major Mvm	nt I	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		362	-	-	1082	-
HCM Lane V/C Ratio		0.018	-	-	-	-
HCM Control Delay (s))	15.1	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)	0.1	-	-	0	-
,	,					

	-	\rightarrow	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	450	3	3	393	1	4
Future Volume (vph)	450	3	3	393	1	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.892	
Flt Protected					0.990	
Satd. Flow (prot)	1861	0	0	1863	1645	0
Flt Permitted					0.990	
Satd. Flow (perm)	1861	0	0	1863	1645	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	489	3	3	427	1	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	492	0	0	430	5	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ration 33.9%			IC	CU Level of	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	TTUL	₩ <u>₩</u>	₩.	ADI
Traffic Vol, veh/h	450	3	3	393	T	4
	450	3	3	393		4
Future Vol, veh/h		-	-		1	
Conflicting Peds, #/hr	0	_ 0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	489	3	3	427	1	4
	Major1		/lajor2		Minor1	
Conflicting Flow All	0	0	492	0	924	491
Stage 1	-	-	-	-	491	-
Stage 2	-	-	-	-	433	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	_	_	1071	-	299	578
Stage 1	-	-	-	-	615	-
Stage 2	_	_		_	654	-
Platoon blocked, %		-			001	
Mov Cap-1 Maneuver		_	1071	_	298	578
Mov Cap-1 Maneuver			1071		298	3/0
	-		-	-		-
Stage 1	-	-	-		615	-
Stage 2	-	-	-	-	651	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		12.5	
HCM LOS			0.1		B	
110111 200						
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		487	-	-	1071	-
HCM Lane V/C Ratio		0.011	-	-	0.003	-
)	12.5	-	-	8.4	0
HCM Control Delay (s)						
		В	_	-	Α	Α
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)	B 0	-	-	A 0	Α -

HCM 6th TWSC

Appendix F

2029 Background Traffic Operations Reports



Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2029 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	262	1	6	307	1	20
Future Volume (vph)	262	1	6	307	1	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	285	1	7	334	1	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	286	0	0	341	23	0
Sign Control	Free			Free	Stop	
Intersection Cummens						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 31.0%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2029 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

0.5					
0.5					
EBT	EBR	WBL	WBT	NBL	NBR
	LDI	TTUL			ADI
	1	6			20
					20
		-			0
	•	-	-	_	•
					Stop
					None
				-	-
	-				-
-	-	-		-	-
92	92	92	92	92	92
14	0	20	6	0	17
285	1	7	334	1	22
0	0	286	0		286
-	-	-	-	286	-
-	-	-	-	348	-
-	-	4.3	-	6.4	6.37
-	-	-	-	5.4	-
_	_	_	-		_
					3.453
					719
					113
	_				-
	-	-		7 19	
		4400		110	740
					719
-	-	-	-		-
-	-	-	-	767	-
-	-	-	-	714	-
ED		MD		ND	
0		0.2			
				В	
nt I	MRI n1	FRT	FRP	WRI	WBT
it I					WDI -
					-
	10.3	-	-	8.1	0
)					
) ı)	B 0.1	-	-	A 0	A
	262 262 262 2 0 0 Free e, # 0 0 0 92 144 285 Major1 0	262 1 262 1 262 1 0 0 Free Free - None 1 6 262 1 6 6 262 1 6 6 6 6 6 6 6 6 6	262 1 6 307 262 1 6 307 262 1 6 307 0 0 0 0 0 Free Free Free Free - None e,# 0 0 92 92 92 92 14 0 20 6 285 1 7 334 Major1 Major2 0 0 286 0	262 1 6 307 1 262 1 6 307 1 262 1 6 307 1 0 0 0 0 0 0 Free Free Free Free Stop - None - None - O 0 e, # 0 0 0 0 0 0 0 292 92 92 92 14 0 20 6 0 285 1 7 334 1 Major1 Major2 Minor1 0 0 286 0 634 286 348 4.3 - 6.4 5.4 5.4 1180 - 446 1180 - 446 1180 - 443 1180 - 443 1180 - 443 777 719 719 719 719 714 EB WB NB 0 0.2 10.3 B nt NBLn1 EBT EBR WBL 698 1180	

	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĥ			ર્ન	¥	
Traffic Volume (vph)	280	4	1	312	0	0
Future Volume (vph)	280	4	1	312	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1638	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1638	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	304	4	1	339	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	308	0	0	340	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 20.5%			IC	CU Level	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	TVDL	₩DI	NDL W	NUI
Traffic Vol, veh/h	280	4	1	312	'T'	0
Future Vol. veh/h	280	4	1	312	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	- 100	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	_	0	0	-
Grade. %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	304	4	1	339	0	0
Major/Minor M	ajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	308	0	647	306
Stage 1	-	-	300	-	306	300
Stage 2	-		-		341	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-		4.1		5.4	0.2
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	-		2.2		3.5	3.3
Pot Cap-1 Maneuver	_	_	1264	_	439	739
Stage 1	-		-		751	-
Stage 2	-	_	_	_	725	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	1264	_	439	739
Mov Cap-2 Maneuver	-	-	-	-	439	-
Stage 1	-	-	_	-	751	-
Stage 2	-	-	-	-	724	-
J						
Annroach	EB		WB		NB	
Approach	0		0 0		0	
HCM Control Delay, s HCM LOS	U		U		A	
FIGNI LOS					А	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
		-	-	-	1264	-
Capacity (veh/h)					0.001	-
Capacity (veh/h) HCM Lane V/C Ratio		-	-			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0	-	-	7.9	0
Capacity (veh/h) HCM Lane V/C Ratio						0 A

HCM 6th TWSC

2: Access A & Highway 3

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	*	•	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ર્ન	¥	
Traffic Volume (vph)	475	0	18	418	2	20
Future Volume (vph)	475	0	18	418	2	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.876	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1658	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1658	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	516	0	20	454	2	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	516	0	0	474	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 46.6%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			ની	¥	
Traffic Vol, veh/h	475	0	18	418	2	20
Future Vol, veh/h	475	0	18	418	2	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	516	0	20	454	2	22
WWW.CT IOW	010	U	20	101	_	
	Major1		Major2		Minor1	=10
Conflicting Flow All	0	0	516	0		516
Stage 1	-	-	-	-	516	-
Stage 2	-	-	-	-	494	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-		-	3.5	3.3
Pot Cap-1 Maneuver	-	-	917	-	268	563
Stage 1	-	-	-	-	603	-
Stage 2	-	-	-	-	617	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	917	-	260	563
Mov Cap-2 Maneuver	-	-	-	-	260	-
Stage 1	-	_	_	-	603	-
Stage 2	-	-	-	-	599	
Stage 2		_	_	_	333	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		12.4	
HCM LOS					В	
Minor Lane/Major Mvm	t 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	. 1	509	EDI	- EDR	917	WDI -
					0.021	
HCM Cartes Dalay (2)		0.047	-			-
HCM Control Delay (s)		12.4	-	-	9	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.1		_	0.1	_

Lanes, Volumes, Timings 2: Access A & Highway 3

Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph)
Ideal Flow (vphpl)
Lane Util. Factor

Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)

Link Speed (k/h)
Link Distance (m)
Travel Time (s)
Peak Hour Factor Heavy Vehicles (%) Adj. Flow (vph)
Shared Lane Traffic (%)
Lane Group Flow (vph)

Sign Control

2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

→	*	•	+	1	<i>></i>
EBT	EBR	WBL	WBT	NBL	NBR
₽			4	¥	
494	1	0	434	2	1
494	1	0	434	2	1
1900	1900	1900	1900	1900	1900
1.00	1.00	1.00	1.00	1.00	1.00
				0.955	
				0.968	
1845	0	0	1759	1756	0
				0.968	
1845	0	0	1759	1756	0
50			50	50	
158.5			70.1	130.3	
11.4			5.0	9.4	
0.92	0.92	0.92	0.92	0.92	0.92
3%	0%	0%	8%	0%	0%
537	1	0	472	2	1
538	0	0	472	3	0

Intersection Summary

Area Type: Control Type: Unsignalized

Intersection Capacity Utilization 36.1% Analysis Period (min) 15

Free

ICU Level of Service A

Stop Free

HCM 6th TWSC 2: Access A & Highway 3 2029 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

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Intersection						
Int Delay, s/veh	0.1					
•		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	♣		^	<u>4</u>	À	
Traffic Vol, veh/h	494	1	0	434	2	1
Future Vol, veh/h	494	1	0	434	2	1
Conflicting Peds, #/hr	_ 0	0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	537	1	0	472	2	1
Major/Minor N	/lajor1		Major2		Minor1	
Conflicting Flow All	0	0	538	0	1010	538
Stage 1	U	-	-	U	538	-
Stage 2					472	_
Critical Hdwy			4.1		6.4	6.2
	-	-		-		
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1040	-	268	547
Stage 1	-	-	-	-	589	-
Stage 2	-	-	-	-	632	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1040	-	268	547
Mov Cap-2 Maneuver	-	-	-	-	268	-
Stage 1	-	-	_	-	589	-
Stage 2	-	-	-	-	632	-
Olugo Z					002	
			14/5			
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		16.3	
HCM LOS					С	
	t 1	NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Maior Mymi					1040	-
Minor Lane/Major Mvmt		323				
Capacity (veh/h)		323				_
Capacity (veh/h) HCM Lane V/C Ratio		0.01	-		-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.01 16.3	-	-	0	-
Capacity (veh/h) HCM Lane V/C Ratio		0.01	-			-

Appendix G

2029 Total Traffic Operations Reports

2029 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»			ર્ન	¥	
Traffic Volume (vph)	266	1	6	307	1	20
Future Volume (vph)	266	1	6	307	1	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.871	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1421	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1421	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	289	1	7	334	1	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	290	0	0	341	23	0
Sign Control	Free			Free	Stop	
Intersection Summany						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 31.0%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

2029 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	W	
Traffic Vol. veh/h	266	1	6	307	1	20
Future Vol. veh/h	266	1	6	307	1	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -	None	-		Stop -	None
Storage Length		None -		None -	0	None
Veh in Median Storage		-	-	0	0	-
Grade. %	0.	-	-	0	0	-
	•			-	•	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mvmt Flow	289	1	7	334	1	22
Major/Minor N	Major1	ı	Major2		Minor1	
Conflicting Flow All	0	0	290	0	638	290
Stage 1	-	_		_	290	-
Stage 2		-	-	-	348	
Critical Hdwy	_	-	4.3	_	6.4	6.37
Critical Hdwy Stg 1	-	-	7.0	-	5.4	0.01
Critical Hdwy Stg 2		-			5.4	
Follow-up Hdwy			2.38		3.5	3.453
Pot Cap-1 Maneuver		-	1176	-	444	715
	-				764	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	719	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1176	-	441	715
Mov Cap-2 Maneuver	-	-	-	-	441	-
Stage 1	-	-	-	-	764	-
Stage 2	-	-	-	-	714	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.4	
HCM LOS	U		0.2		10.4 B	
I IOW LOS					0	
		NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Major Mvm	it I	VDLIII				
	nt I	694	-	-	1176	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	nt I				1176 0.006	-
Capacity (veh/h) HCM Lane V/C Ratio		694	-			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		694 0.033 10.4	-	-	0.006	- 0
Capacity (veh/h) HCM Lane V/C Ratio		694 0.033	-	-	0.006	-

Lane Group EBT EBR WBL WBT NBL NBR Lane Configurations ♣ ♣ ♣ ✔ ✔ ✔ ✓
Traffic Volume (vph) 280 8 2 312 0 0
Traffic Volume (vph) 280 8 2 312 0 0
Firture Values (inh) 000 0 0 240 0 0
Future Volume (vph) 280 8 2 312 0 0
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00
Frt 0.996
FIt Protected
Satd. Flow (prot) 1638 0 0 1810 1900 0
Flt Permitted
Satd. Flow (perm) 1638 0 0 1810 1900 0
Link Speed (k/h) 50 50
Link Distance (m) 158.5 70.1 130.3
Travel Time (s) 11.4 5.0 9.4
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92
Heavy Vehicles (%) 16% 0% 0% 5% 0% 0%
Adj. Flow (vph) 304 9 2 339 0 0
Shared Lane Traffic (%)
Lane Group Flow (vph) 313 0 0 341 0 0
Sign Control Free Free Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 21.3% ICU Level of Service A
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	0.1					
	EBT	EDD	WDi	MDT	ND	NDD
Movement		EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	•	^	4	À	^
Traffic Vol, veh/h	280	8	2	312	0	0
Future Vol, veh/h	280	8	2	312	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mymt Flow	304	9	2	339	0	0
M-i/Mi	4-14	_	4-:0	_	M:4	
	Major1		Major2		Minor1	200
Conflicting Flow All	0	0	313	0	652	309
Stage 1	-	-	-	-	309	-
Stage 2	-	-	-	-	343	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1259	-	436	736
Stage 1	-	-	-	-	749	-
Stage 2	-	-	-	-	723	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1259	-	435	736
Mov Cap-2 Maneuver	-	-	-	-	435	-
Stage 1	-	-	-	-	749	-
Stage 2	-	-	-	-	722	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		0	
HCM LOS					Α	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		NDLIII -	EDI	EDK -	1259	WDI
HCM Lane V/C Ratio					0.002	
		0	-	-	7.9	0
HCM Control Delay (s) HCM Lane LOS		A	-		7.9 A	A
		А	-	-		
HCM 95th %tile Q(veh)		-	-	-	0	-

	-	\rightarrow	•	•	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ _e			ર્ન	¥	
Traffic Volume (vph)	280	0	2	314	0	0
Future Volume (vph)	280	0	2	314	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1863	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1863	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	304	0	2	341	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	304	0	0	343	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 21.5%			IC	U Level o	of Service
Analysis Period (min) 15						

Intersection Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/m Sign Control RT Channelized Storage Length Voland In Median Storag Grade, % Peak Hour Factor	0 EBT 280 280 280	EBR 0	WBL	WBT	NDI	
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280	0	WBL		NDI	
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280	0	VVDL			NBR
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280 280				NBL	NDK
Future Vol, veh/h Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	280		0	4	À	٥
Conflicting Peds, #/hr Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor			2	314	0	0
Sign Control RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor	r 0	0	2	314	0	0
RT Channelized Storage Length Veh in Median Storag Grade, % Peak Hour Factor		0	0	0	0	0
Storage Length Veh in Median Storag Grade, % Peak Hour Factor	Free	Free	Free	Free	Stop	Stop
Veh in Median Storag Grade, % Peak Hour Factor	-	None	-	None	-	None
Grade, % Peak Hour Factor	-	-	-	-	0	-
Peak Hour Factor	qe,# 0	-	-	0	0	-
	0	-	-	0	0	-
	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	304	0	2	341	0	0
INIVITIE I TOW	304	U		J 4 I	U	U
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	304	0	649	304
Stage 1	-	_	_	_	304	-
Stage 2		-	-	-	345	-
Critical Hdwy	-	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	-	-	-1.12	-	5.42	0.22
Critical Hdwy Stg 1					5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
		-				
Pot Cap-1 Maneuver		-	1257	-	434	736
Stage 1	-	-	-	-	748	-
Stage 2	-	-	-	-	717	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	r -	-	1257	-	433	736
Mov Cap-2 Maneuver	r -	-	-	-	433	-
Stage 1	_	_	_	_	748	_
Stage 2		-	-	-	716	-
otago 2						
Approach	EB		WB		NB	
HCM Control Delay, s	s 0		0		0	
HCM LOS					Α	
Minor Lane/Major Mv	mt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-			1257	
HCM Lane V/C Ratio		-				
HCM Control Delay (s	S)	0	-	-	7.9	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(ve	h)	-	-	-	0	-

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 1: Goshen Road & Highway 3

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f a			ર્ન	¥	
Traffic Volume (vph)	476	0	19	420	2	23
Future Volume (vph)	476	0	19	420	2	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.875	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	517	0	21	457	2	25
Shared Lane Traffic (%)						
Lane Group Flow (vph)	517	0	0	478	27	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized
Intersection Capacity Utilization 47.5%
Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			4	¥	
Traffic Vol, veh/h	476	0	19	420	2	23
Future Vol, veh/h	476	0	19	420	2	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	- 100	None	-	None
Storage Length	-	-		-	0	-
Veh in Median Storage,		_	_	0	0	_
Grade, %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mymt Flow	517	0	21	457	2	25
IVIVMT FIOW	517	U	21	457	2	25
Major/Minor M	1ajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	517	0	1016	517
Stage 1	-	-	-	-	517	-
Stage 2	-	-	-	-	499	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1		-	-	-	5.4	-
Critical Hdwy Stg 2		-	_	-	5.4	_
Follow-up Hdwy	-	_	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	916	_	266	562
Stage 1	-		310		603	- 502
Stage 2				-	614	
Platoon blocked. %	-		_		014	_
	-		040		050	562
Mov Cap-1 Maneuver	-	-	916	-	258	
Mov Cap-2 Maneuver	-	-	-	-	258	-
Stage 1	-	-	-	-	603	-
Stage 2	-	-	-	-	595	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		12.4	
HCM LOS	U		0.4		12.4 B	
TIOW LOO						
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		514	-	-	916	-
HCM Lane V/C Ratio		0.053	-	-	0.023	-
HCM Control Delay (s)		12.4	-	-	9	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.2	_	-	0.1	_

2029 Total PM Peak Hour

(230661) 494 Highway 3, Courtland TIS

Paradigm Transportation Solutions Limited

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

HCM 6th TWSC 2: Access A & Highway 3 2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	—	1	-	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			ર્ન	¥		
Traffic Volume (vph)	497	2	0	435	4	2	
Future Volume (vph)	497	2	0	435	4	2	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.955		
Flt Protected					0.968		
Satd. Flow (prot)	1845	0	0	1759	1756	0	
Flt Permitted					0.968		
Satd. Flow (perm)	1845	0	0	1759	1756	0	
Link Speed (k/h)	50			50	50		
Link Distance (m)	158.5			70.1	130.3		
Travel Time (s)	11.4			5.0	9.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%	
Adj. Flow (vph)	540	2	0	473	4	2	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	542	0	0	473	6	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	ization 36.3%			IC	CU Level of	of Service	e A
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	¥	
Traffic Vol, veh/h	497	2	0	435	4	2
Future Vol. veh/h	497	2	0	435	4	2
Conflicting Peds, #/hr	497	0	0	435	0	0
	-	Free		_	_	_
Sign Control	Free		Free	Free	Stop	Stop
RT Channelized		None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	540	2	0	473	4	2
Maiaa/Miaaa	N4=:==4		M-:0		Aire and	
	Major1		Major2 542		Minor1	544
Conflicting Flow All	0	0		0	1014	541
Stage 1	-	-	-	-	541	-
Stage 2	-	-	-	-	473	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1037	-	267	545
Stage 1	-	-	-	-	588	-
Stage 2	_	-	_	-	631	-
Platoon blocked. %	-	-		-		
Mov Cap-1 Maneuver	-	_	1037	_	267	545
Mov Cap-1 Maneuver		-	1007	-	267	-
					588	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	631	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		16.4	
HCM LOS					C	
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		322	-	-	1037	-
HCM Lane V/C Ratio		0.02	-	-	-	-
HCM Control Delay (s)	16.4	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh	1)	0.1	-	-	0	-
	,					

2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS HCM 6th TWSC 3: Access B & Highway 3 2029 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	→	\rightarrow	•	←	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f			ર્ન	¥	
Traffic Volume (vph)	497	3	3	434	1	4
Future Volume (vph)	497	3	3	434	1	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.892	
Flt Protected					0.990	
Satd. Flow (prot)	1861	0	0	1863	1645	0
Flt Permitted					0.990	
Satd. Flow (perm)	1861	0	0	1863	1645	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	540	3	3	472	1	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	543	0	0	475	5	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utiliz	zation 36.3%			IC	CU Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	¥	
Traffic Vol, veh/h	497	3	3	434	1	4
Future Vol, veh/h	497	3	3	434	1	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-		-	0	-
Veh in Median Storage	. # 0	-	_	0	0	-
Grade. %	0	-		0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	540	3	3	472	1	4
IVIVIIIL I IOW	340	J	J	412	- 1	7
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	543	0	1020	542
Stage 1	-	-	-	-	542	-
Stage 2	-	-	-	-	478	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1026	-	262	540
Stage 1	-	-	-	-	583	-
Stage 2	-	-	-	-	624	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	1026	_	261	540
Mov Cap-2 Maneuver		-	-		261	-
Stage 1	_	_	_	_	583	_
Stage 2	-	-	-	-	622	
Stage 2					022	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		13.2	
HCM LOS					В	
Min I /M-i M		NDI 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvm	It I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		445	-	-	1026	-
HCM Lane V/C Ratio		0.012	-		0.003	-
HCM Control Delay (s)		13.2	-	-	8.5	0
HCM Lane LOS HCM 95th %tile Q(veh)		B 0	-	-	A	Α
			_	_	0	_

Appendix H

2034 Background Traffic Operations Reports

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2034 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	Y	
Traffic Volume (vph)	290	1	6	339	1	22
Future Volume (vph)	290	1	6	339	1	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.870	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1418	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1418	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	315	1	7	368	1	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	316	0	0	375	25	0
Sign Control	Free			Free	Stop	
Intersection Commons						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 32.6%
Analysis Period (min) 15

ICU Level of Service A

1: Goshen Road & Highway 3

HCM 6th TWSC

2034 Background AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	290	1	6	339	1	22
Future Vol. veh/h	290	1	6	339	1	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	_	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	.# 0	_	-	0	0	_
Grade. %	0			0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mymt Flow	315	1	7	368	1	24
WIVIII I IOW	313		'	300	- 1	24
Major/Minor N	Major1	1	Major2	- 1	Minor1	
Conflicting Flow All	0	0	316	0	698	316
Stage 1	-	-	-	-	316	-
Stage 2	-	-	-	-	382	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-	3.5	3.453
Pot Cap-1 Maneuver	-	-	1149	-	410	691
Stage 1	-	-	-	-	744	-
Stage 2	-	_	-	_	694	-
Platoon blocked, %		-		-		
Mov Cap-1 Maneuver	_	_	1149	_	407	691
Mov Cap-2 Maneuver		-	- 1110		407	-
Stage 1	_	_	_	_	744	
Stage 2		-			688	-
Stage 2	-	-	-		000	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.6	
HCM LOS					В	
					14/51	WDT
Minor Lane/Major Mvm	t I	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		671	-	-	1149	-
HCM Lane V/C Ratio		0.037	-		0.006	-
HCM Control Delay (s)		10.6	-	-	8.2	0
		В	_	_	Α	Α
HCM Lane LOS HCM 95th %tile Q(veh)		0.1			0	,,

HCM 6th TWSC

2: Access A & Highway 3

Paradigm Transportation Solutions Limited

	-	\rightarrow	•	•	1	*
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	¢î			ર્ન	¥	
Traffic Volume (vph)	310	4	1	344	0	0
Future Volume (vph)	310	4	1	344	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998					
Flt Protected						
Satd. Flow (prot)	1637	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1637	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	337	4	1	374	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	341	0	0	375	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	ization 22.2%			IC	CU Level of	of Service A
Analysis Deried (min) 15						

intoroccion cummary			
Area Type:	Other		
Control Type: Unsignalized	t de la companya de la companya de la companya de la companya de la companya de la companya de la companya de		
Intersection Capacity Utiliz	ation 22.2%	ICU Level of Service A	
Analysis Period (min) 15			

Intersection						
Int Delay, s/veh	0					
	EDT	EDD	MD	MOT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$			4	À	^
Traffic Vol, veh/h	310	4	1	344	0	0
Future Vol, veh/h	310	4	1	344	0	0
Conflicting Peds, #/hr	_ 0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	16	0	0	5	0	0
Mvmt Flow	337	4	1	374	0	0
		_		_		
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	341	0	715	339
Stage 1	-	-	-	-	339	-
Stage 2	-	-	-	-	376	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1229	-	400	708
Stage 1	-	-	-	-	726	-
Stage 2	-	-	-	-	699	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	1229	-	400	708
Mov Cap-2 Maneuver	-	-	-	-	400	-
Stage 1	-	-	-	_	726	-
		-			698	
Stage 2	-	-	-	-	698	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		0	
HCM LOS	•		·		Ā	
TIOW LOO						
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1229	-
HCM Lane V/C Ratio		-	-	-	0.001	-
HCM Control Delay (s)		0	-	-	7.9	0
HCM Lane LOS		Ā	-		A	Ā
HCM 95th %tile Q(veh)		-	_	_	0	-
TOM JOHN JOHN (VEII)					J	

Lanes, Volumes, Timings 1: Goshen Road & Highway 3

2034 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Page 1

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	Y	
Traffic Volume (vph)	525	0	20	461	2	22
Future Volume (vph)	525	0	20	461	2	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.875	
Flt Protected				0.998	0.996	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.996	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	571	0	22	501	2	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	571	0	0	523	26	0
Sign Control	Free			Free	Stop	
Interception Cummens						

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 50.5%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3 2034 Background PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1≽			4	¥	
Traffic Vol, veh/h	525	0	20	461	2	22
Future Vol, veh/h	525	0	20	461	2	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length		-		-	0	-
Veh in Median Storage	.# 0	-	_	0	0	-
Grade, %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
	571	-	22	501	2	24
Mvmt Flow	5/1	0	22	501	2	24
Major/Minor N	/lajor1	ı	Major2	- 1	Minor1	
Conflicting Flow All	0	0	571	0	1116	571
Stage 1	-	-	-	-	571	-
Stage 2	-	-	-	-	545	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	_	-	-	_	5.4	_
Follow-up Hdwy	-		2.479	-	3.5	3.3
Pot Cap-1 Maneuver	-	_	873	_	232	524
Stage 1	-		-		569	- 02-1
Stage 2	_			_	585	_
Platoon blocked. %	-				303	
Mov Cap-1 Maneuver	-	-	873	-	224	524
					224	
Mov Cap-2 Maneuver	-	-	-	-		-
Stage 1	-	-	-	-	569	-
Stage 2	-	-	-	-	565	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		13.1	
HCM LOS			0.1		В	
Minor Lane/Major Mvm	t i	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		471	-	-	873	-
HCM Lane V/C Ratio		0.055	-	-	0.025	-
HCM Control Delay (s)		13.1	-	-	9.2	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.2	-	-	0.1	-
,						

	-	•	€	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	546	1	0	479	2	1
Future Volume (vph)	546	1	0	479	2	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	593	1	0	521	2	1
Shared Lane Traffic (%)						
Lane Group Flow (vph)	594	0	0	521	3	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliz	ation 38.8%			IC	CU Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK	WDL			NDK
Lane Configurations	\$	4	٥	470	Å	1
Traffic Vol, veh/h	546	1	0	479	2	
Future Vol, veh/h	546	1	0	479	2	1
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	593	1	0	521	2	1
Major/Minor N	Major1		Aniar?		Minor1	
			Major2			504
Conflicting Flow All	0	0	594	0	1115	594
Stage 1	-	-	-	-	594	-
Stage 2	-	-	-	-	521	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	992	-	232	509
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	600	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	992	-	232	509
Mov Cap-2 Maneuver	-	-	-	-	232	-
Stage 1	-	_	_	-	555	-
Stage 2					600	_
Olage 2					000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		17.9	
HCM LOS					С	
Minor Lane/Major Mvm	1 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	•	283			992	-
HCM Lane V/C Ratio		0.012			332	
ACIVI Lane V/C Ratio		17.9	-	-	0	
UCM Control Doloy (a)		17.9	-	-	-	-
HCM Control Delay (s)		^				
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		C 0	-	-	A 0	-

HCM 6th TWSC

2: Access A & Highway 3

Appendix I

2034 Total Traffic Operations Reports

2034 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Lanes, Volumes, Timings
1: Goshen Road & Highway 3

	-	•	•	_	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	294	1	6	339	1	22
Future Volume (vph)	294	1	6	339	1	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.870	
Flt Protected				0.999	0.998	
Satd. Flow (prot)	1667	0	0	1786	1418	0
Flt Permitted				0.999	0.998	
Satd. Flow (perm)	1667	0	0	1786	1418	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	0%	20%	6%	0%	17%
Adj. Flow (vph)	320	1	7	368	1	24
Shared Lane Traffic (%)						
Lane Group Flow (vph)	321	0	0	375	25	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 32.6%
Analysis Period (min) 15

ICU Level of Service A

HCM 6th TWSC 1: Goshen Road & Highway 3

Paradigm Transportation Solutions Limited

2034 Total AM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.4					
	•••	EDD.	WDI	MOT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$		•	4	Y	00
Traffic Vol, veh/h	294	1	6	339	1	22
Future Vol, veh/h	294	1	6	339	1	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	14	0	20	6	0	17
Mvmt Flow	320	1	7	368	1	24
Maiaa/Miaaa	4-14	_	4-:	_	M:4	
	Major1		Major2		Minor1	00:
Conflicting Flow All	0	0	321	0	703	321
Stage 1	-	-	-	-	321	-
Stage 2	-	-	-	-	382	-
Critical Hdwy	-	-	4.3	-	6.4	6.37
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.38	-	3.5	3.453
Pot Cap-1 Maneuver	-	-	1144	-	407	686
Stage 1	-	-	-	-	740	-
Stage 2	-	-	-	_	694	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1144	-	404	686
Mov Cap-2 Maneuver	-	-	-	-	404	-
Stage 1	-	_	_	-	740	-
Stage 2			_		688	_
Olago Z					000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		10.6	
HCM LOS					В	
Minor Lane/Major Mvm	it I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		666	-	-	1144	-
HCM Lane V/C Ratio		0.038	-	-	0.000	-
HCM Control Delay (s)		10.6	-	-	8.2	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh))	0.1	-	-	0	-

	-	•	€	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	Y	
Traffic Volume (vph)	310	8	2	344	0	0
Future Volume (vph)	310	8	2	344	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.996					
Flt Protected						
Satd. Flow (prot)	1637	0	0	1810	1900	0
Flt Permitted						
Satd. Flow (perm)	1637	0	0	1810	1900	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	0%	0%	5%	0%	0%
Adj. Flow (vph)	337	9	2	374	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	346	0	0	376	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utili	zation 23.0%			IC	U Level	of Service
Analysis Period (min) 15						

0					
0					
0					
EBT	EBR	WBL	WBT	NBL	NBR
f)			ર્ની	Y	
310	8	2	344	0	0
310	8	2	344	0	0
. 0	0	0	0	0	0
Free	Free	Free	Free	Stop	Stop
-	None	-	None	-	None
-	-	-	-	0	-
ie.# 0	_	-	0	0	_
	-		-	-	
-				-	92
					0
					0
331	9	2	3/4	U	U
Major1	N	Major2	N	Minor1	
0			0	720	342
-					
_	_	_			_
_	_	11	_		6.2
-	-	7.1		5.4	- 0.2
		_			
				E 1	
-	-	-	-	5.4	- 2.2
- -	-	2.2	-	3.5	3.3
- -	-	2.2 1224	-	3.5 398	3.3 705
- - -	-	2.2	-	3.5 398 724	3.3 705
- - - -	- - -	2.2 1224	- - -	3.5 398	3.3 705
-	-	2.2 1224 -	-	3.5 398 724 697	3.3 705 -
- - - - -	- - -	2.2 1224	- - -	3.5 398 724 697	3.3 705
-	-	2.2 1224 -	-	3.5 398 724 697 397 397	3.3 705 -
- - - - -	-	2.2 1224 - - 1224	-	3.5 398 724 697	3.3 705 - - 705
- - - - - - -	-	2.2 1224 - - 1224	-	3.5 398 724 697 397 397	3.3 705 - - 705
- - - - - - - -	-	2.2 1224 - - 1224 -	-	3.5 398 724 697 397 397 724	3.3 705 - - 705 -
- - - - - - - -	-	2.2 1224 - - 1224 - -	-	3.5 398 724 697 397 397 724 696	3.3 705 - - 705 -
- - - - - - - -	-	2.2 1224 - - 1224 - - - WB	-	3.5 398 724 697 397 397 724 696	3.3 705 - - 705 -
- - - - - - - -	-	2.2 1224 - - 1224 - -	-	3.5 398 724 697 397 397 724 696 NB	3.3 705 - - 705 -
- - - - - - - -	-	2.2 1224 - - 1224 - - - WB	-	3.5 398 724 697 397 397 724 696	3.3 705 - - 705 -
- - - - - - - -	-	2.2 1224 - - 1224 - - - WB	-	3.5 398 724 697 397 397 724 696 NB	3.3 705 - - 705 -
- - - - - - - - - - - - - - - - - - -	-	2.2 1224 - - 1224 - - - WB	-	3.5 398 724 697 397 397 724 696 NB 0 A	3.3 705 - - 705 - -
- - - - - - - - - - - - - - - - - - -	- - - - - - -	2.2 1224 - 1224 - - - WB 0	- - - - - - - -	3.5 398 724 697 397 724 696 NB 0 A	3.3 705 - - 705 - - -
- - - - - - - - - - - - - - - - - - -	- - - - - - - - - -	2.2 1224 - 1224 - - - WB 0	- - - - - - - -	3.5 398 724 697 397 724 696 NB 0 A	3.3 705 - - 705 - - - - - WBT
	- - - - - - - - - NBLn1	2.2 1224 - - 1224 - - - - WB 0	- - - - - - - - -	3.5 398 724 697 397 397 724 696 NB 0 A	3.3 705 - - 705 - - - - - - -
- - - - - - - - - - - - - - - - - - -	- - - - - - - - - - - - - - - - - - -	2.2 1224 1224 WB 0	EBR -	3.5 398 724 697 397 724 696 NB 0 A WBL 1224 0.002 7.9	3.3 705 - - 705 - - - - - - WBT - 0
	- - - - - - - - - NBLn1	2.2 1224 - - 1224 - - - - WB 0	EBR -	3.5 398 724 697 397 724 696 NB 0 A WBL 1224 0.002 7.9 A	3.3 705
	- - - - - - - - - - - - - - - - - - -	2.2 1224 1224 WB 0	EBR -	3.5 398 724 697 397 724 696 NB 0 A WBL 1224 0.002 7.9	3.3 705 - 705 - - - - - - WBT - 0
)	310 0 Free e, # 0 0 92 16 337 Major1 0	310 8 0 0 0 Free Free - None e, # 0 - 92 92 16 0 0 337 9 Major1 1 0 0	310 8 2 0 0 0 0 Free Free Free - None 92 92 92 16 0 0 0 337 9 2 Major1 Major2 0 0 346 4.1	310 8 2 344 0 0 0 0 0 0 Free Free Free Free Free - None 0 0 0 0 92 92 92 92 16 0 0 5 337 9 2 374 Major1 Major2 1 0 0 346 0 4.1 -	310

HCM 6th TWSC

3: Access B & Highway 3

	-	•	•	•	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^			ની	Y	
Traffic Volume (vph)	310	0	2	347	0	0
Future Volume (vph)	310	0	2	347	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1863	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1863	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	337	0	2	377	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	337	0	0	379	0	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 23.2%			IC	U Level o	of Service A
Analysis Period (min) 15						

Intersection Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/l Sign Control	0 EBT					
Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/l Sign Control	EBT					
Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/l Sign Control						
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/I Sign Control		EDD	WD	WDT	ND	NDD
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/I Sign Control		EBR	WBL	WBT	NBL	NBR
Future Vol, veh/h Conflicting Peds, #/I Sign Control		^	^	4	À	•
Conflicting Peds, #/I Sign Control	310	0	2	347	0	0
Sign Control	310	0	2	347	0	0
		_ 0	_ 0	_ 0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Stora		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	337	0	2	377	0	0
					-	-
Major/Minor	Major1		Major2	1	Minor1	
Conflicting Flow All	0	0	337	0	718	337
Stage 1	-	-	-	-	337	-
Stage 2	-	-	-	-	381	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	-	5.42	-
Follow-up Hdwy		-	2.218	-	3.518	3.318
Pot Cap-1 Maneuve		-	1222	-	396	705
Stage 1	-		-		723	-
Stage 2				_	691	_
Platoon blocked. %			_		031	_
Mov Cap-1 Maneuv			1222	-	395	705
Mov Cap-1 Maneuv		-	1222			
		-		-	395	-
	-	-	-	-	723	-
Stage 1	-	-	-	-	690	-
Stage 1						
Stage 1 Stage 2	ER		W/R			
Stage 1 Stage 2 Approach	EB		WB		NB	
Stage 1 Stage 2 Approach HCM Control Delay,			WB 0		0	
Stage 1 Stage 2 Approach						
Stage 1 Stage 2 Approach HCM Control Delay,					0	
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS	, s 0	NRI n1	0	FBR	0 A	WRT
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M	, s 0	NBLn1	0 EBT	EBR	0 A WBL	WBT
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M Capacity (veh/h)	, s 0 Ivmt	-	0 EBT	-	0 A WBL 1222	-
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M Capacity (veh/h) HCM Lane V/C Rati	y, s 0	-	0 EBT	-	0 A WBL 1222 0.002	-
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M Capacity (veh/h) HCM Lane V/C Rati HCM Control Delay	y, s 0	- - 0	0 EBT - -	-	0 A WBL 1222 0.002 8	- - 0
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M Capacity (veh/h) HCM Lane V/C Rati HCM Control Delay HCM Lane LOS	fys 0	-	0 EBT - -	- - -	0 A WBL 1222 0.002 8 A	- 0 A
Stage 1 Stage 2 Approach HCM Control Delay, HCM LOS Minor Lane/Major M Capacity (veh/h) HCM Lane V/C Rati HCM Control Delay	fys 0	- - 0	0 EBT - -	-	0 A WBL 1222 0.002 8	- - 0

2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ર્ન	¥	
Traffic Volume (vph)	526	0	21	463	2	25
Future Volume (vph)	526	0	21	463	2	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.874	
Flt Protected				0.998	0.997	
Satd. Flow (prot)	1827	0	0	1771	1656	0
Flt Permitted				0.998	0.997	
Satd. Flow (perm)	1827	0	0	1771	1656	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.2			158.5	186.1	
Travel Time (s)	18.4			11.4	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	4%	0%	31%	6%	0%	0%
Adj. Flow (vph)	572	0	23	503	2	27
Shared Lane Traffic (%)						
Lane Group Flow (vph)	572	0	0	526	29	0
Sign Control	Free			Free	Stop	

ICU Level of Service A

Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 51.4%
Analysis Period (min) 15

HCM 6th TWSC 1: Goshen Road & Highway 3

Paradigm Transportation Solutions Limited

2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

Intersection						
Int Delay, s/veh	0.5					
• .	EDT	EDD	MDi	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	^	0.4	4	À	٥٢
Traffic Vol, veh/h	526	0	21	463	2	25
Future Vol, veh/h	526	0	21	463	2	25
Conflicting Peds, #/hr	_ 0	_ 0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	- " 0	-	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	4	0	31	6	0	0
Mvmt Flow	572	0	23	503	2	27
Major/Minor I	Major1		Major2		Minor1	
Conflicting Flow All	0	0	572	0	1121	572
Stage 1	_	-	-	-	572	_
Stage 2	-	-	-	-	549	-
Critical Hdwy	-	-	4.41	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	_	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.479	-	3.5	3.3
Pot Cap-1 Maneuver	_	-	872	-	230	523
Stage 1	-	-	-	-	569	-
Stage 2	_	-	_	-	583	_
Platoon blocked. %	-	-		-		
Mov Cap-1 Maneuver	_	-	872	-	221	523
Mov Cap-2 Maneuver	-	-	-	-	221	-
Stage 1	-	_	_	_	569	-
Stage 2	-				561	
Olugo 2					001	
			MD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		13.1	
HCM LOS					В	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		475	-	-	872	-
HCM Lane V/C Ratio		0.062	-	-	0.026	-
HCM Control Delay (s))	13.1	-	_	9.2	0
HCM Lane LOS	,	В	-		Α	A
HCM 95th %tile Q(veh	1)	0.2	-	-	0.1	_
304.7743 3(1011	'/	0.2			0.1	

	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	549	2	0	480	4	2
Future Volume (vph)	549	2	0	480	4	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.955	
Flt Protected					0.968	
Satd. Flow (prot)	1845	0	0	1759	1756	0
Flt Permitted					0.968	
Satd. Flow (perm)	1845	0	0	1759	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	158.5			70.1	130.3	
Travel Time (s)	11.4			5.0	9.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	0%	0%	8%	0%	0%
Adj. Flow (vph)	597	2	0	522	4	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	599	0	0	522	6	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 39.0%			IC	CU Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>⊏БІ</u>	EDI	WDL	₩D1	INDL	NDI
Traffic Vol, veh/h	549	2	0	480	Υ 4	2
Future Vol. veh/h		2	0	480	4	2
	549 0	0	0	480	0	0
Conflicting Peds, #/hr		-	-			_
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized				None		None
Storage Length	- 4 0	-	-	0	0	
Veh in Median Storage		-	-	-	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	0	0	8	0	0
Mvmt Flow	597	2	0	522	4	2
Major/Minor N	Major1	P	Major2	- 1	Minor1	
Conflicting Flow All	0	0	599	0	1120	598
Stage 1	-	-	333	-	598	-
Stage 2					522	_
Critical Hdwy		-	4.1	_	6.4	6.2
Critical Hdwy Stg 1			4.1	-	5.4	0.2
	-					
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	988	-	231	506
Stage 1	-	-	-	-	553	-
Stage 2	-	-	-	-	599	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	988	-	231	506
Mov Cap-2 Maneuver	-	-	-	-	231	-
Stage 1	-	-	-	-	553	-
Stage 2	-	-	-	-	599	-
·						
Annraach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		0		18.1	
HCM LOS					С	
Minor Lane/Major Mvm	it I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		282	-	-	988	-
		0.023		-	-	-
HCM Lane V/C Ratio			_	_	U	_
HCM Lane V/C Ratio HCM Control Delay (s)		18.1	-		0 A	
HCM Lane V/C Ratio			-	-	0 A 0	-

2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

	-	•	1	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)			ર્ન	¥	
Traffic Volume (vph)	548	3	3	479	1	4
Future Volume (vph)	548	3	3	479	1	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999				0.892	
Flt Protected					0.990	
Satd. Flow (prot)	1861	0	0	1863	1645	0
Flt Permitted					0.990	
Satd. Flow (perm)	1861	0	0	1863	1645	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	70.1			184.9	93.4	
Travel Time (s)	5.0			13.3	6.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	596	3	3	521	1	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	599	0	0	524	5	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

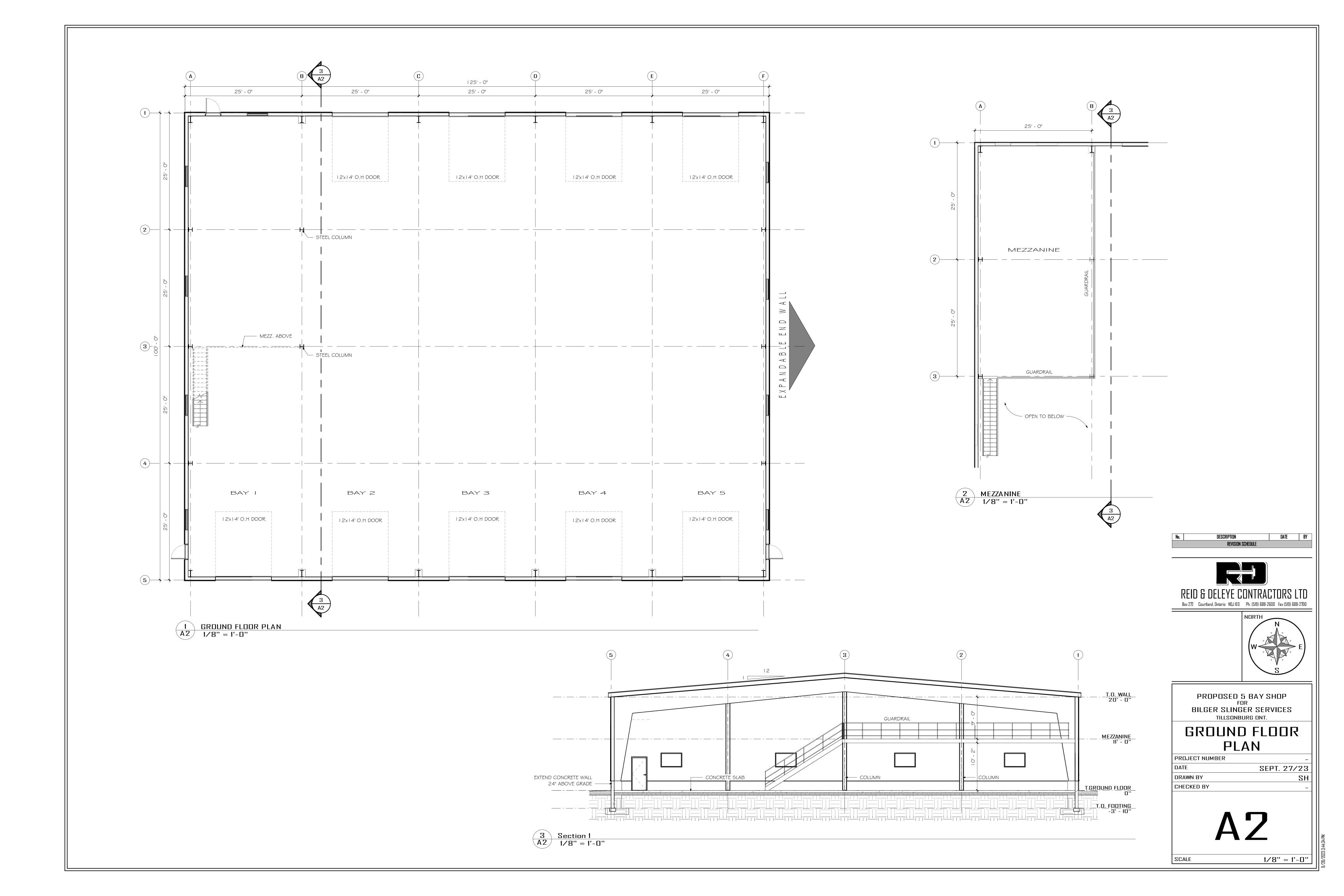
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 39.0%
Analysis Period (min) 15

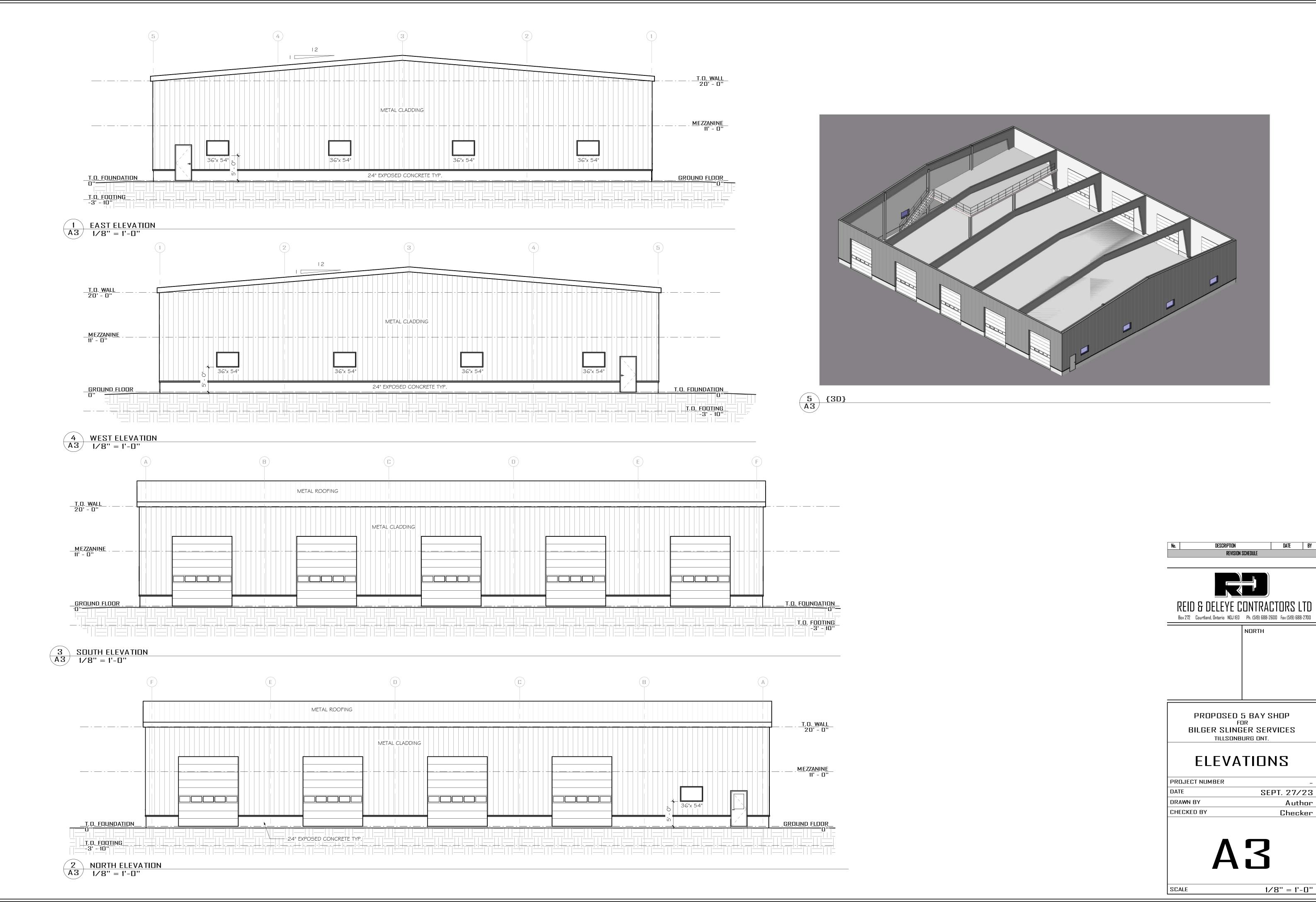
ICU Level of Service A

HCM 6th TWSC 3: Access B & Highway 3

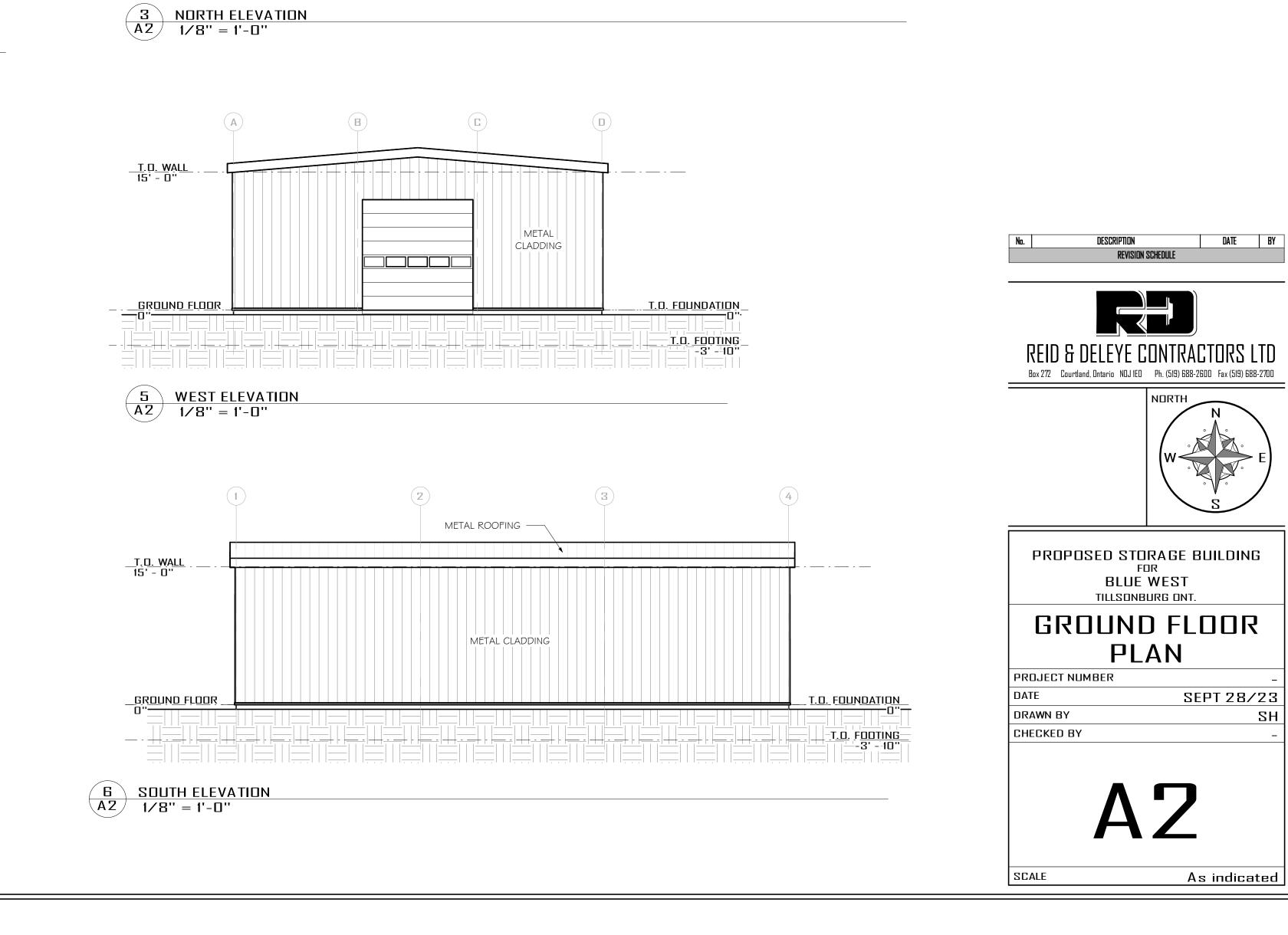
2034 Total PM Peak Hour (230661) 494 Highway 3, Courtland TIS

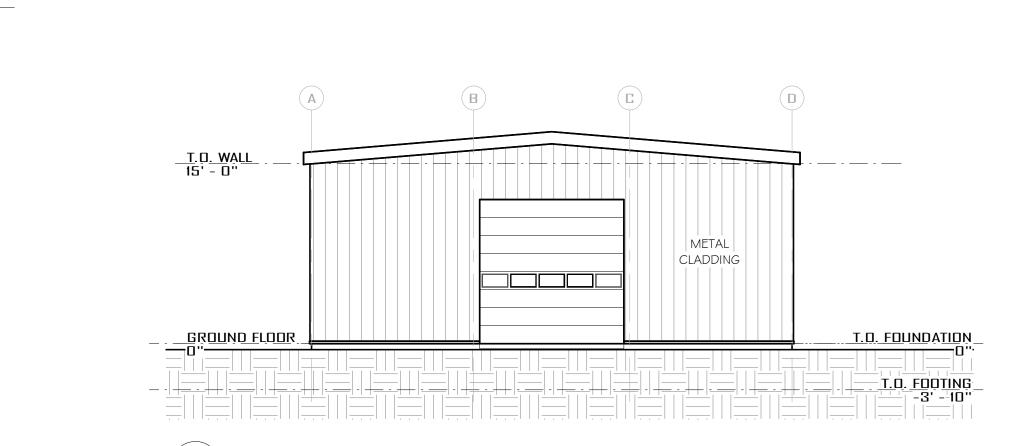
Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		EDK	WDL		INDL	NDK
	F 40	3	2	470		4
Traffic Vol, veh/h	548	_	3	479	1	
Future Vol, veh/h	548	3	3	479	1	4
Conflicting Peds, #/hr	0	_ 0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	596	3	3	521	1	4
Major/Minor N	/lajor1		Major2		Minor1	
Conflicting Flow All	0	0	599	0	1125	598
Stage 1		-	-	U	598	-
Stage 2					527	
Critical Hdwy	-		4.12		6.42	6.22
		-		-		
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-		2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	978	-	227	502
Stage 1	-	-	-	-	549	-
Stage 2	-	-	-	-	592	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	978	-	226	502
Mov Cap-2 Maneuver	-	-	-	-	226	-
Stage 1	-	-	-	_	549	_
Stage 2		-	-	_	590	-
Olage 2					550	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		14.1	
HCM LOS					В	
Minor Lane/Major Mym	t I	NRI n1	FRT	FRP	WRI	WRT
Minor Lane/Major Mvmt	t I	NBLn1	EBT	EBR	WBL 078	WBT
Capacity (veh/h)	t I	403	-	-	978	-
Capacity (veh/h) HCM Lane V/C Ratio	t I	403 0.013	-	-	978 0.003	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t I	403 0.013 14.1	-	-	978 0.003 8.7	- - 0
Capacity (veh/h) HCM Lane V/C Ratio		403 0.013	-	-	978 0.003	-

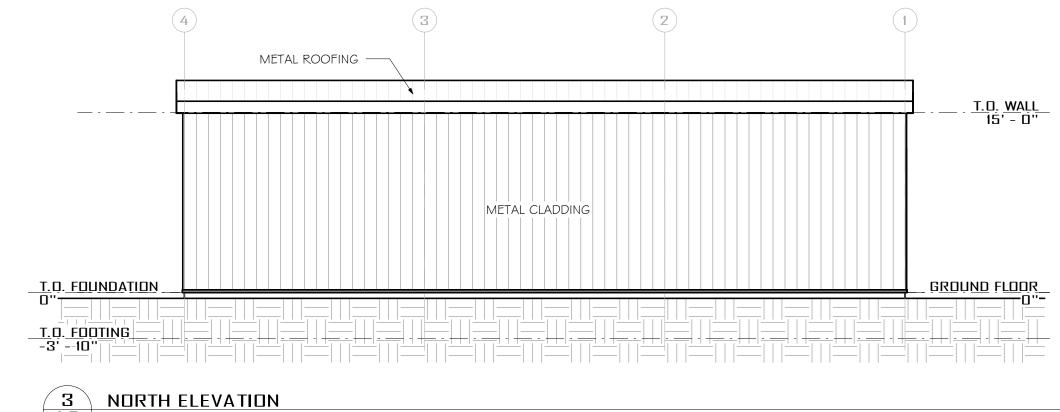


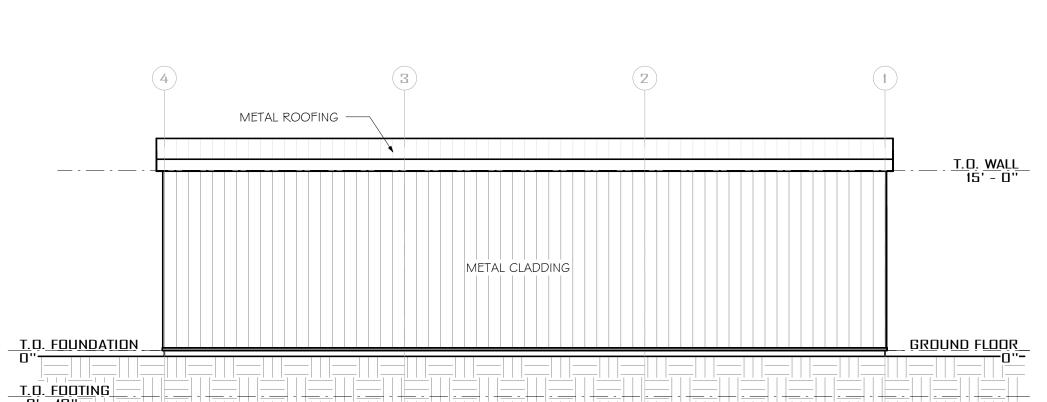


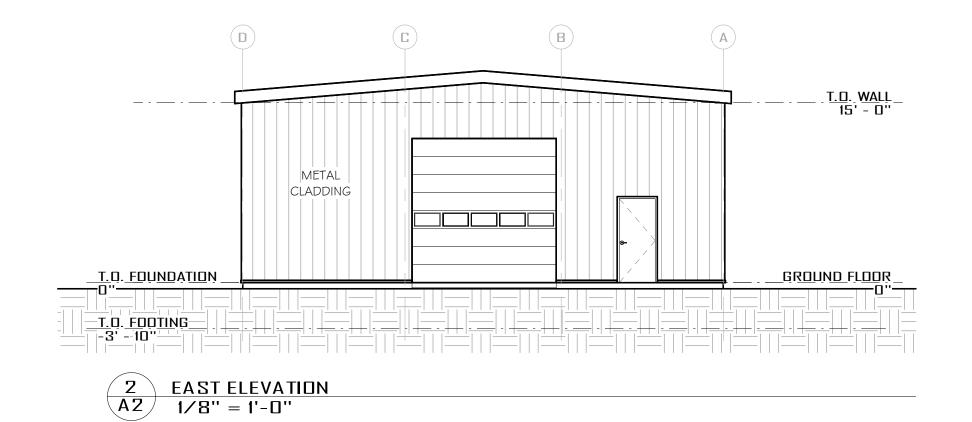
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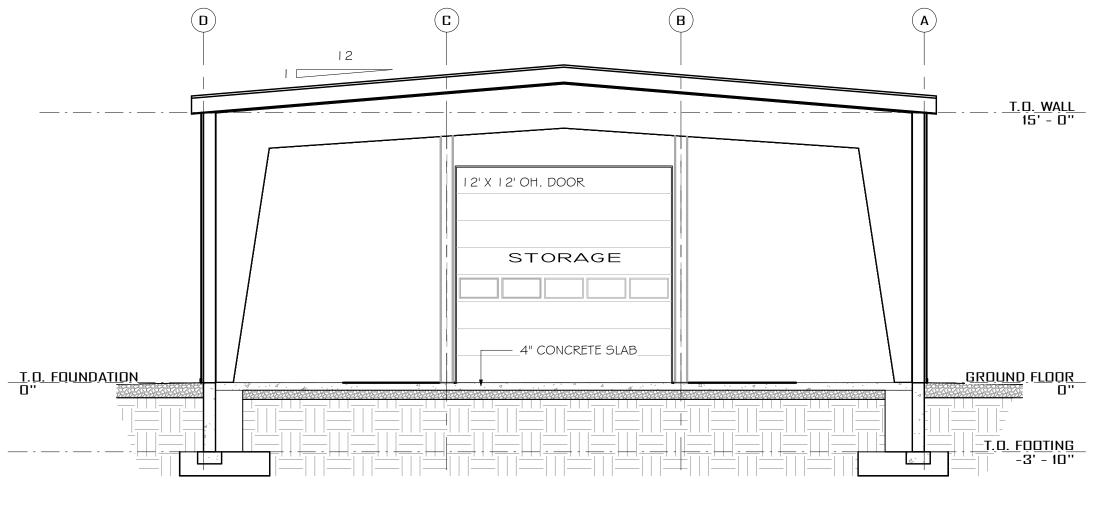




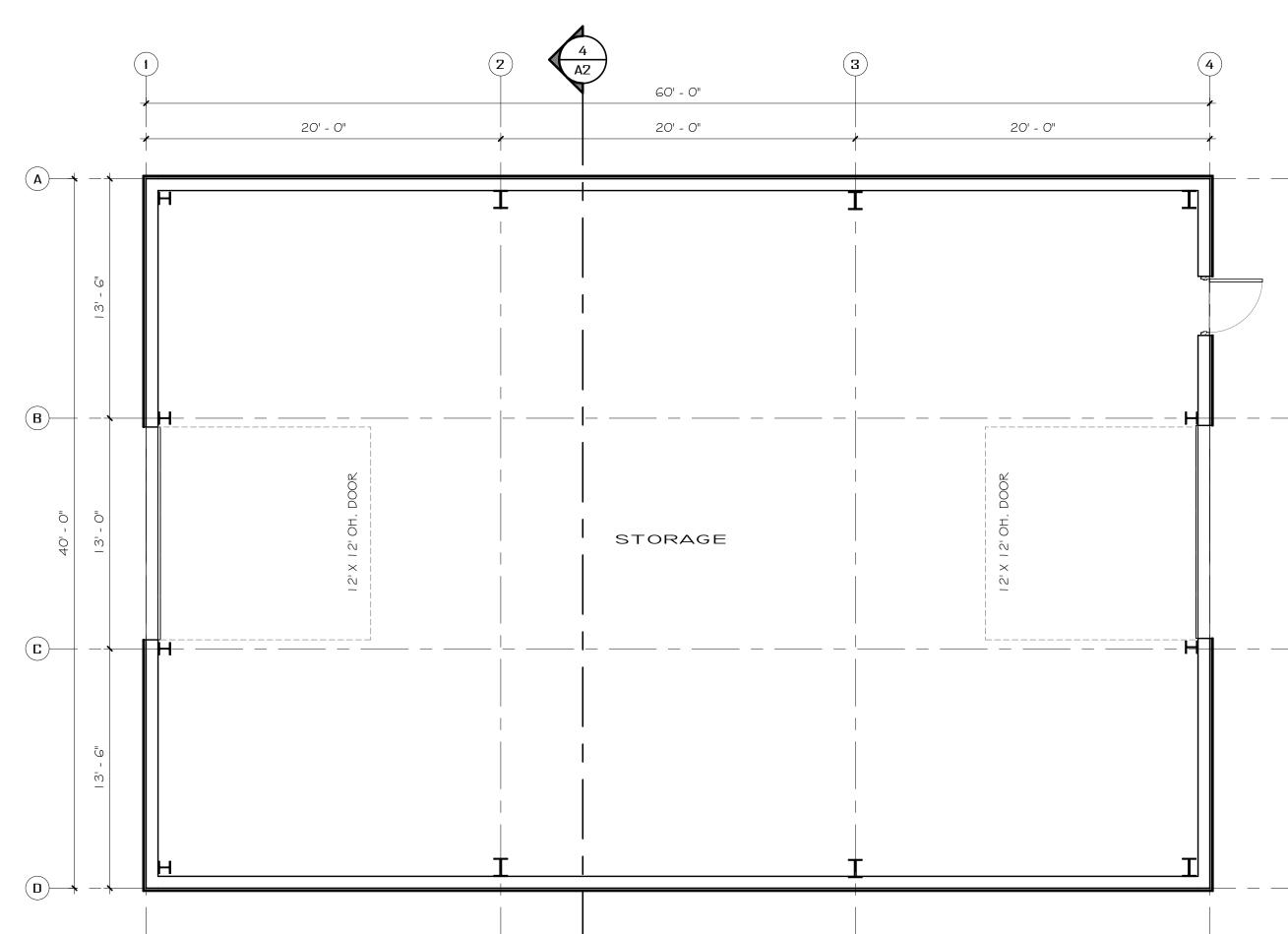


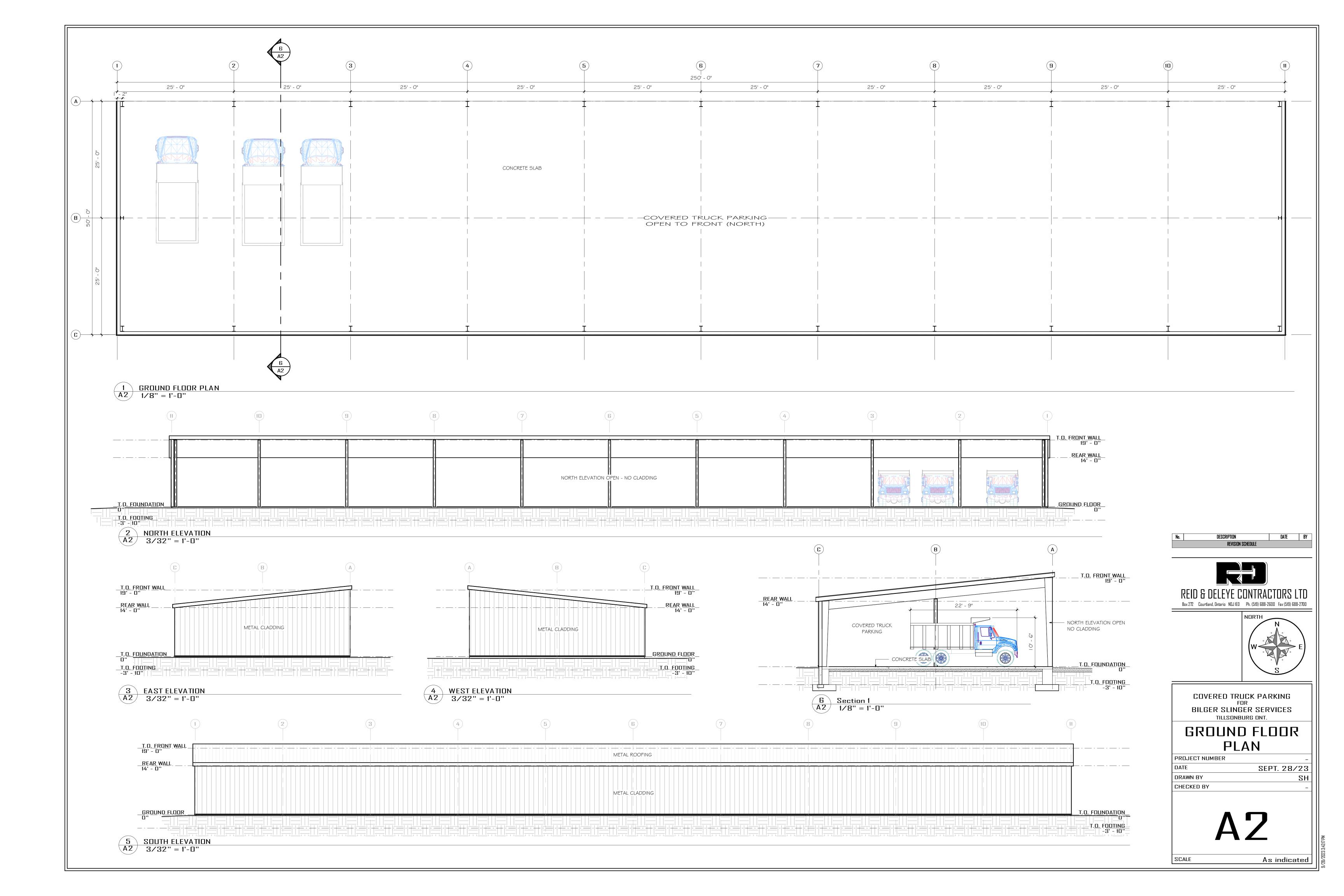


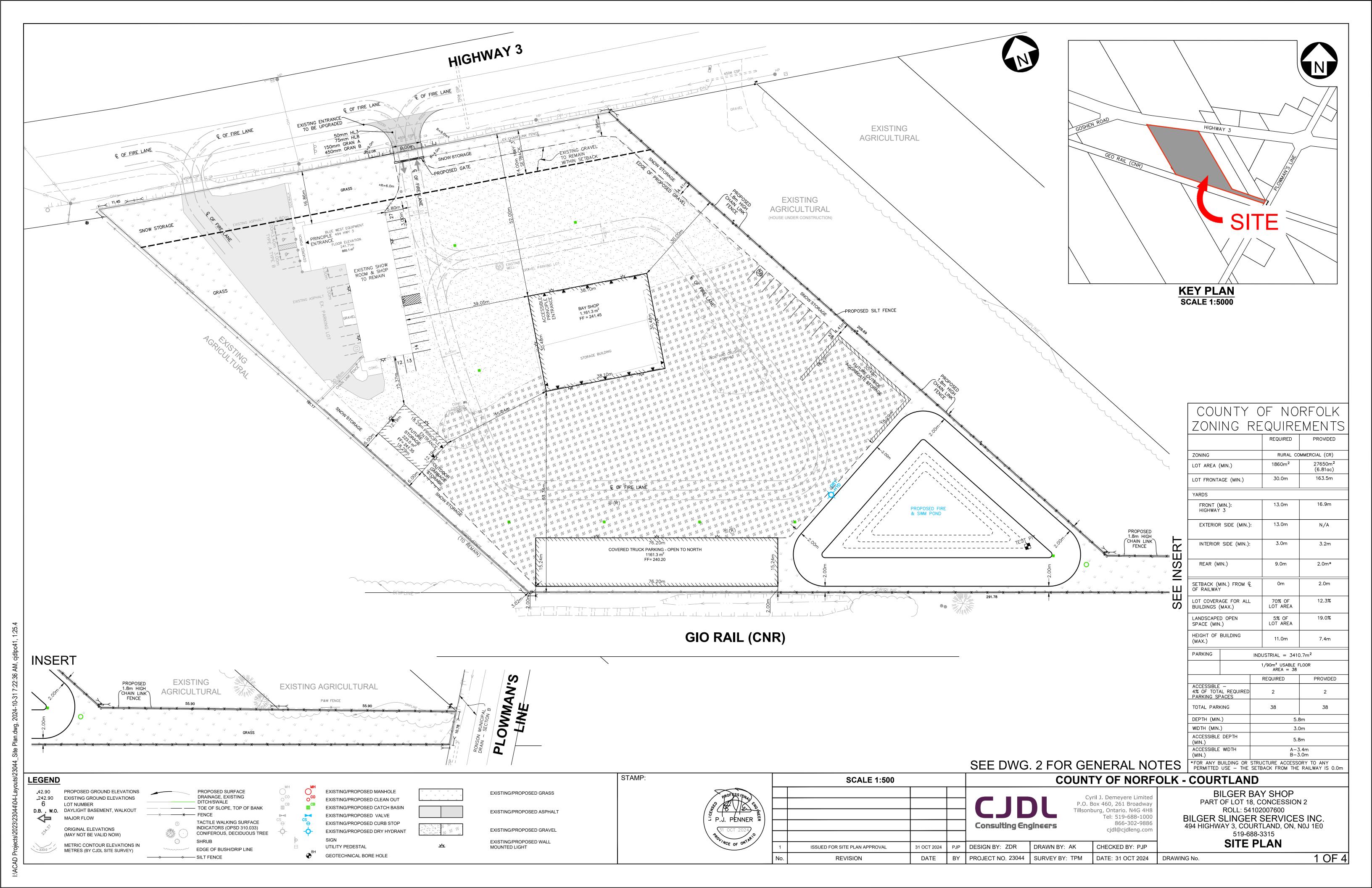
4 Section 1 A2 3/16" = 1'-0"

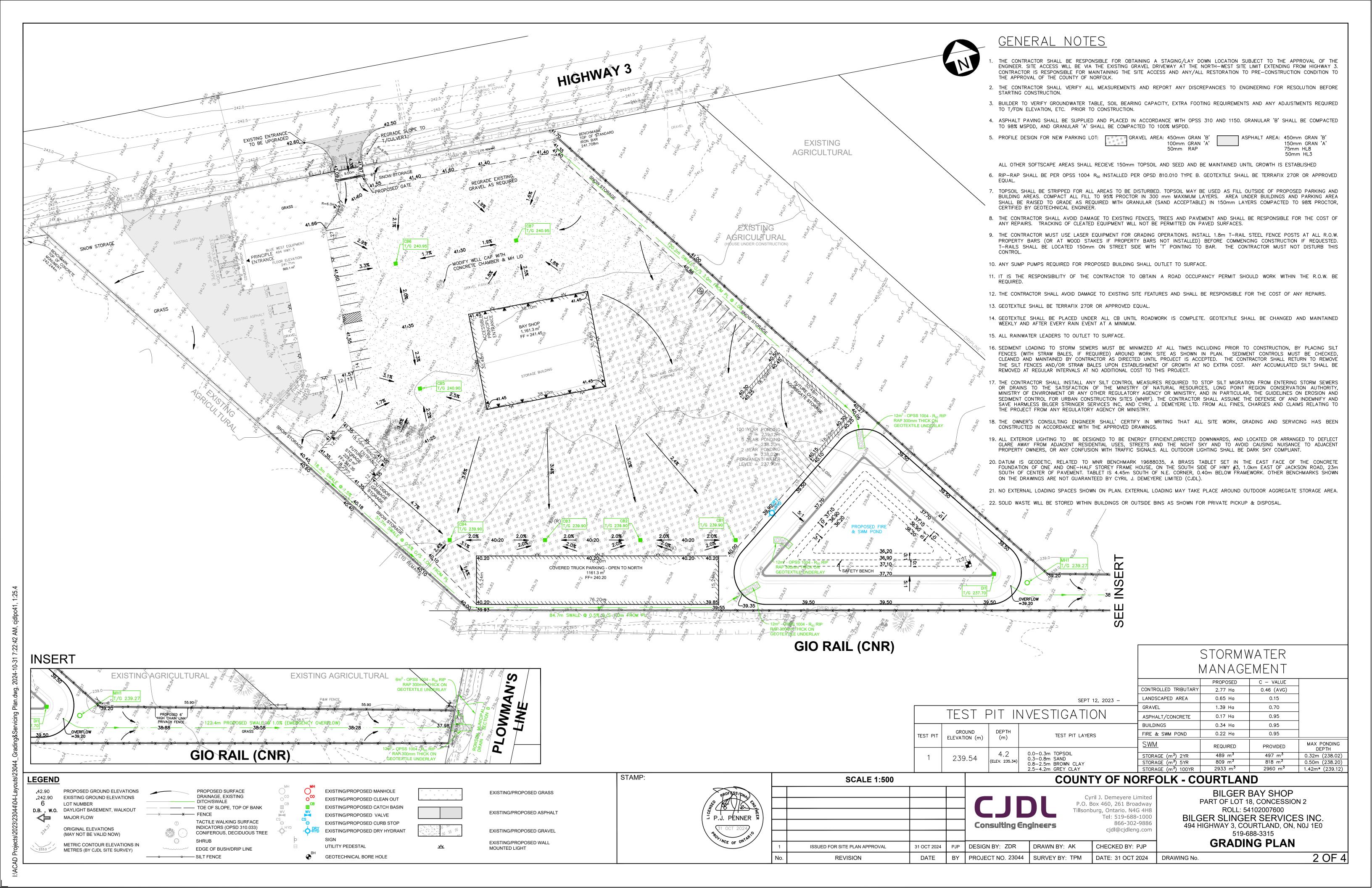


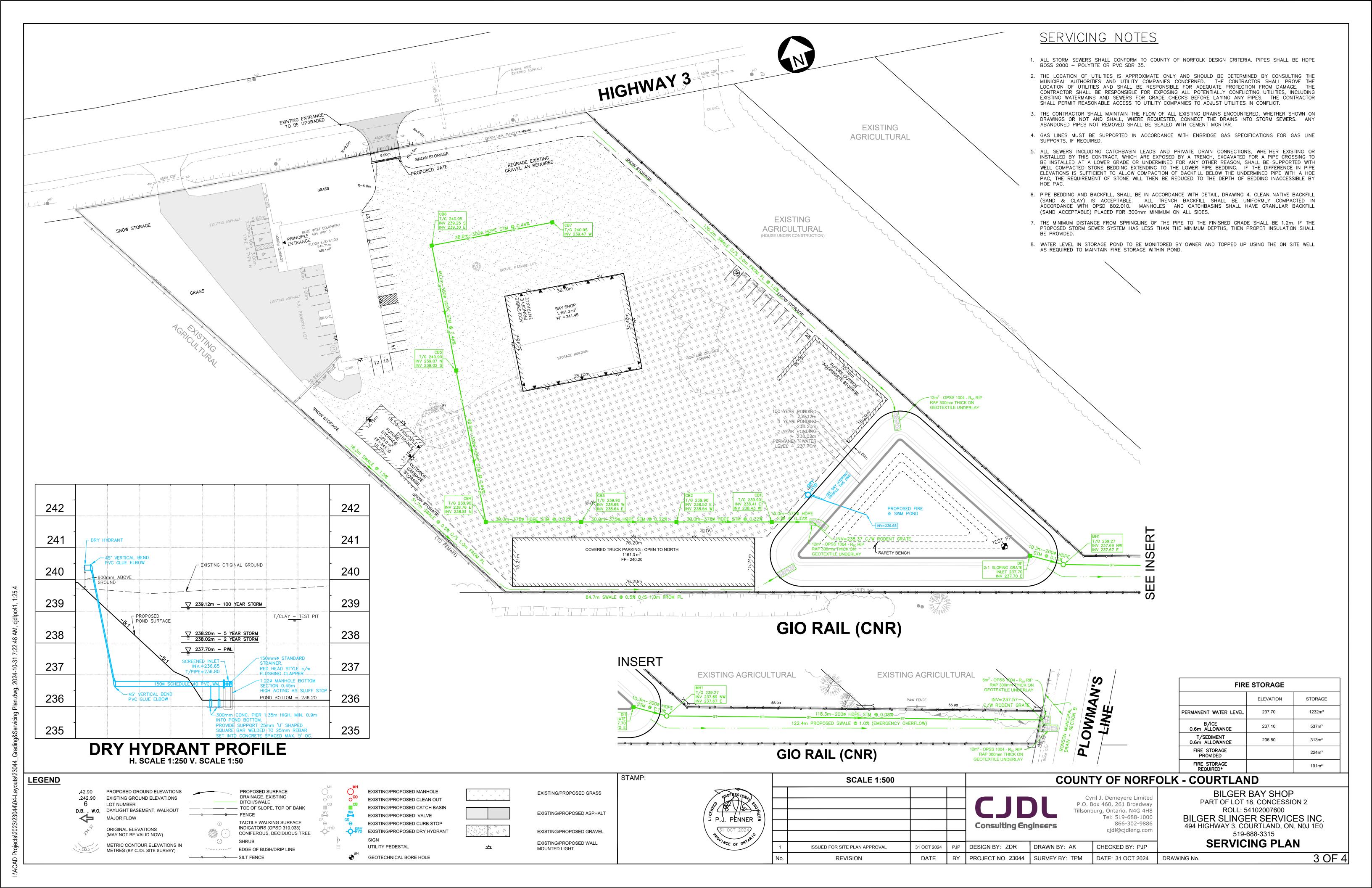
1 GROUND FLOOR PLAN A2 3/16" = 1'-0"

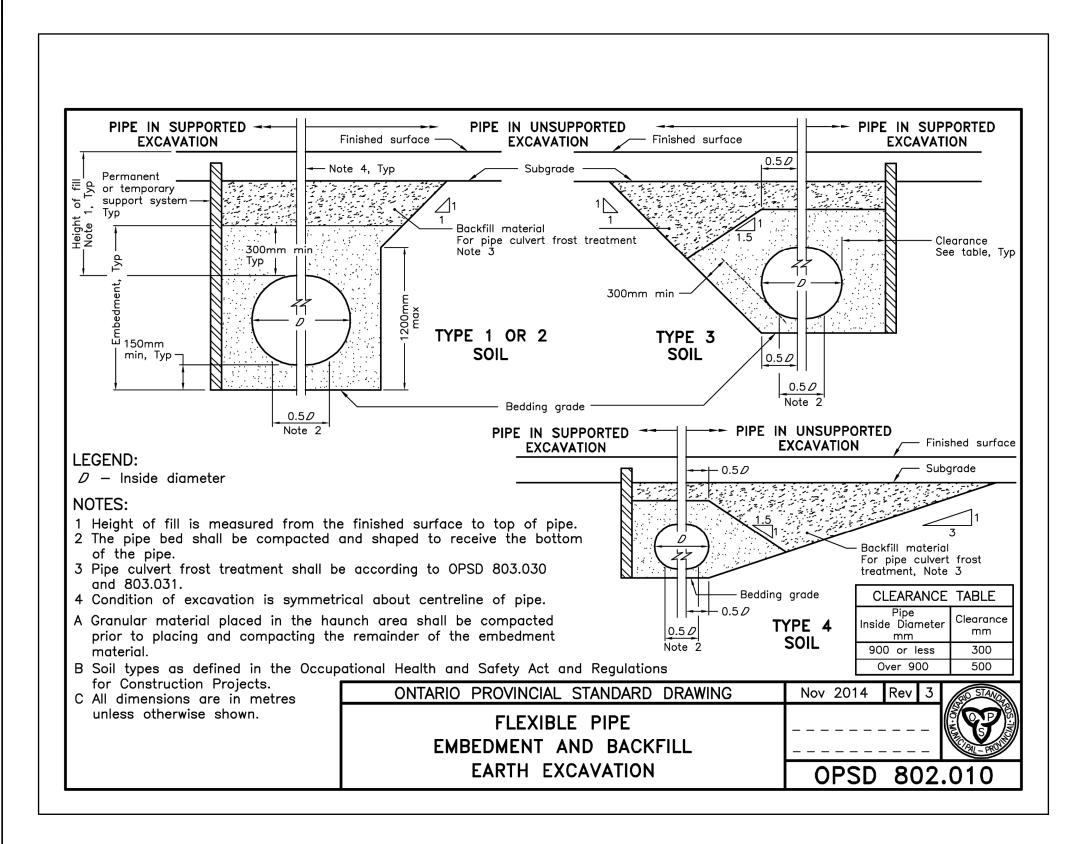


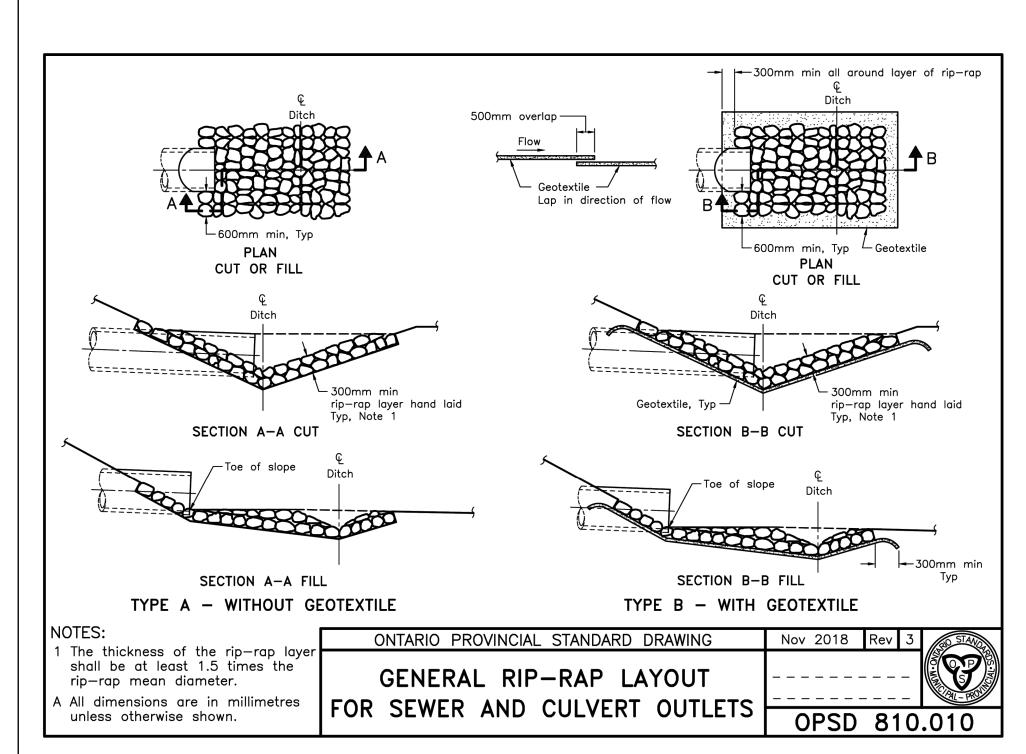


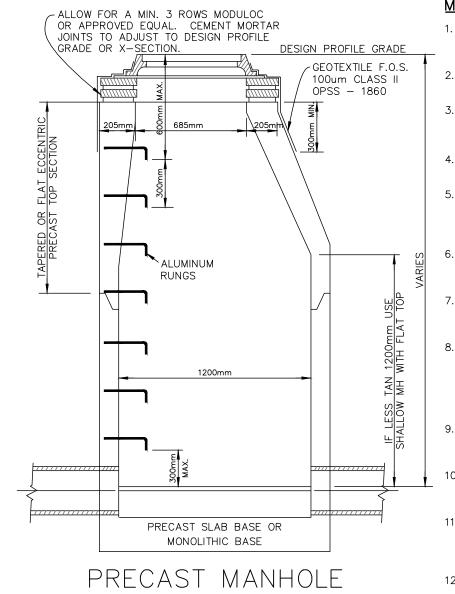








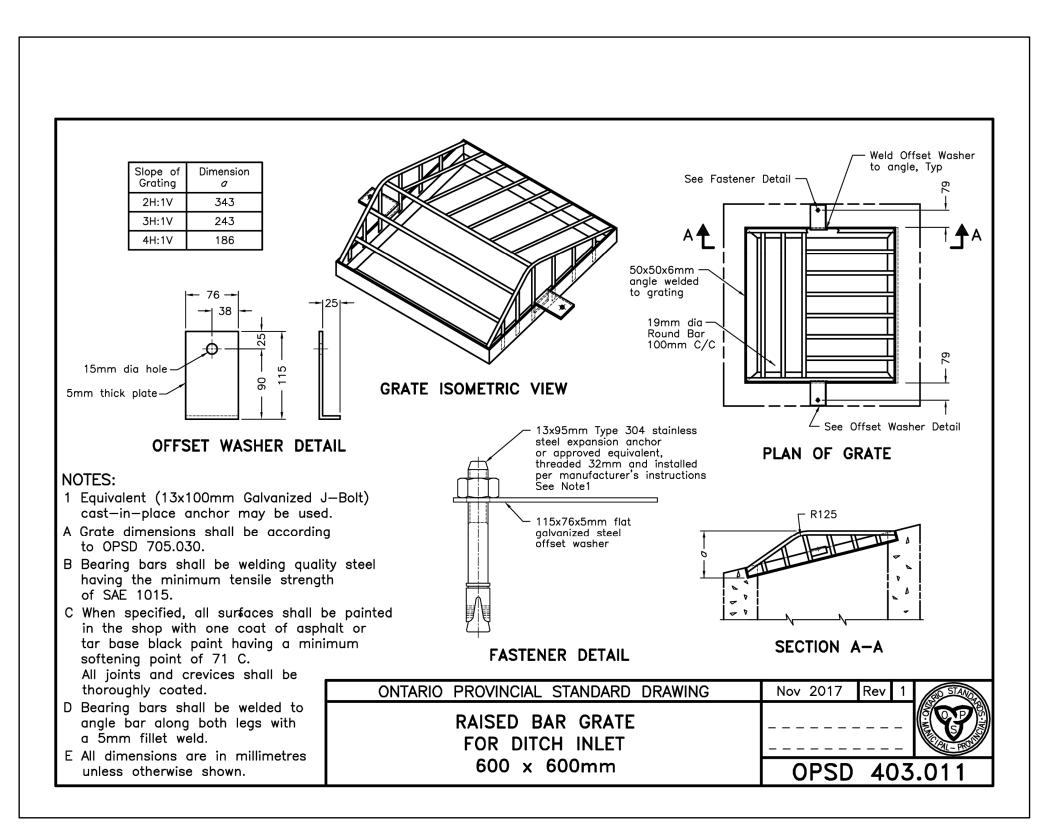


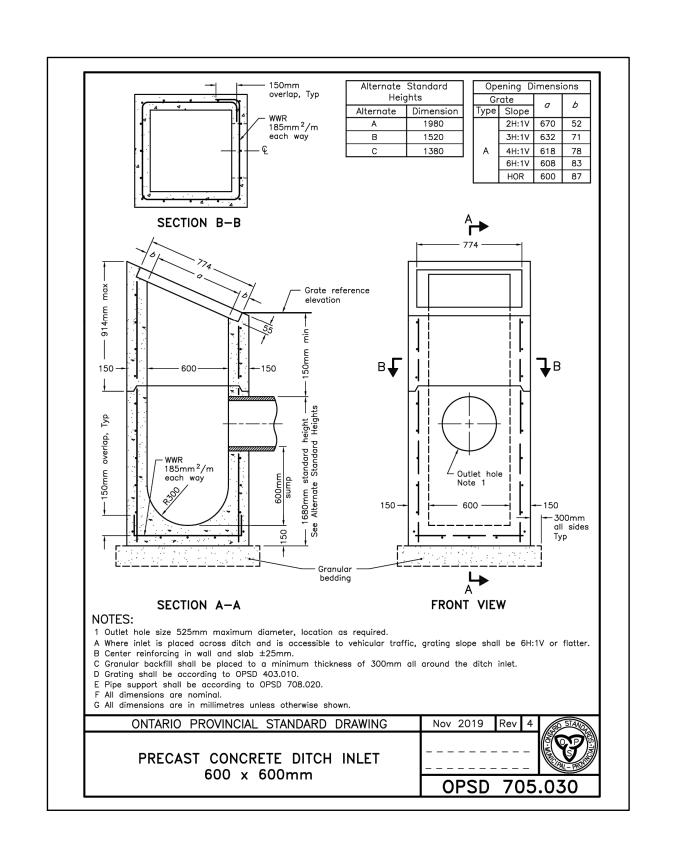


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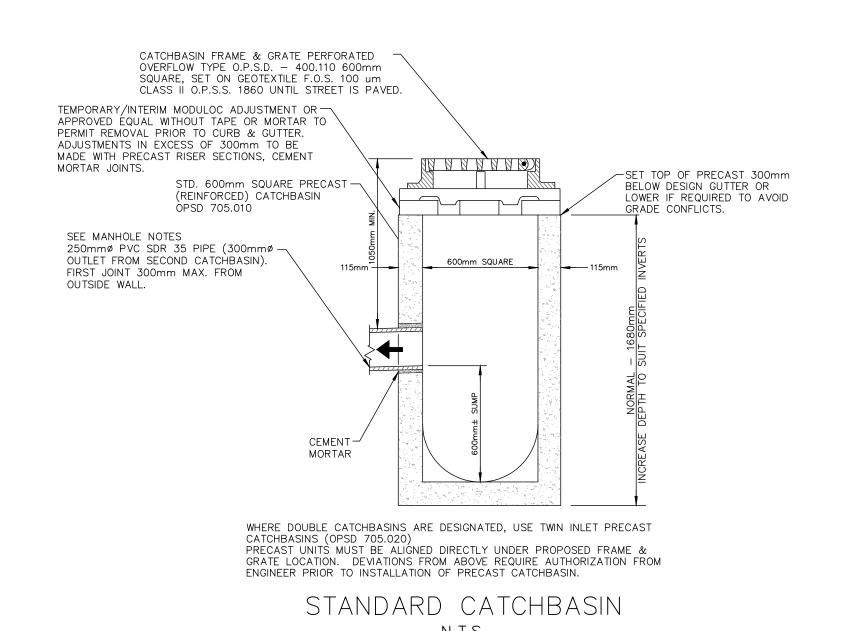
MANHOLE NOTES

- 1. PRECAST SECTIONS SHALL COMPLY WITH CURRENT OPSD 700 SERIES FOR PRECAST CONCRETE MANHOLES. PREBENCHED MANHOLES ARE ACCEPTABLE.
- ON OVERSIZED MANHOLES, THE TRANSITION TO 1200mm SHALL NOT BE LESS THAN 1.5m ABOVE MAINLINE OVERT.
- . FIRST PIPE JOINT TO BE WITHIN 300 mm OF OUTSIDE MANHOLE WALL. OTHERWISE CONCRETE CRADLE SUPPORT TO FIRST JOINT. (CLASS 'B'
- BEDDING ACCEPTABLE ON PIPES 450mmø & LARGER). 4. PVC AND POLYETHYLENE PIPE SHALL BE CONNECTED TO MANHOLES
- WITH FACTORY SAND COATED PVC STUB OR KOR-N-SEAL COUPLING.
- BENCHING ON END MANHOLES TO EXTEND TO FAR INSIDE WALL.
- BENCHING ON LATERAL INLETS SHALL BE 500mm C RADIUS. BENCHING TO CONFORM TO OPSD 701.021. MANHOLES SHALL BE PREBENCHED FOR ALL PDCs / LATERALS.
- TOP LADDER RUNG SHALL NOT EXCEED 600 mm FROM FINISHED GRADE. MANHOLES OVER 5.0m DEEP SHALL INCLUDE SAFETY GRATE
- MANHOLE FRAMES AND GRATES SHALL BE; O.P.S.D. 401.010 -CLOSED COVER MANHOLE COVERS SHALL BE SET ON GEOTEXTILE F.O.S. 250 um CLASS II O.P.S.S. 1860 UNTIL STREET IS PAVED.
- CEMENT MORTAR USED FOR SETTING OF PIPES, BENCHING, ADJUSTMENT & SETTING OF FRAMES FOR CATCHBASINS & MANHOLES, SHALL CONSIST OF ONE PART NORMAL PORTLAND CEMENT TO TWO PARTS MASONRY SAND WITH ONLY SUFFICIENT WATER TO MAKE THE MIXTURE PLASTIC. ADJUSTMENT RINGS TO BE PARGED ON OUTSIDE; INSIDE JOINTS TO BE POINTED ONLY, NOT PARGED.
- GRANULAR BACKFILL SHALL BE PLACED TO A MIN. WIDTH OF 300mm ALL AROUND MANHOLES, IN 300 mm MAX. LIFTS COMPACTED TO 95% STANDARD PROCTOR.
- 10. ADJUSTMENTS IN EXCESS OF 300mm SHALL BE MADE WITH PRECAST RISER SECTIONS. NO STEPS SHALL BE INSTALLED IN CONCRETE ADJUSTMENTS UNITS.
- 11. IN AREAS OF HIGH GROUNDWATER, WATERPROOFING OF MANHOLES AND CHAMBERS IS REQUIRED AS PER COUNTY GUIDELINES. SEALTIGHT MEL-ROL WATERPROOFING SYSTEM AS MANUFACTURED BY W.R. MEADOWS OR APPROVED EQUAL.
- 12. EXTERNAL DROP STRUCTURE PER OPSD 1003.010 SHALL BE PROVIDED IN MANHOLES WHEN THE DIFFERENCE IN ELEVATION BETWEEN INVERTS OF THE INLET PIPE AND AN OUTLET PIPE EXCEED 0.9m FOR STORM AND 0.6m FOR SANITARY.
- 13. ALL CATCHBASINS MANHOLES (CBMH) ARE TO BE BENCHED UNLESS OTHERWISE NOTES ON DESIGN DRAWINGS OR DIRECTED ON-SITE BY





STAMP:



COUNTY OF NORFOLK - COURTLAND BILGER BAY SHOP Cyril J. Demeyere Limited PART OF LOT 18, CONCESSION 2 P.O. Box 460, 261 Broadway ROLL: 54102007600 Tillsonburg, Ontario. N4G 4H8 Tel: 519-688-1000 P.J. PÉNNER BILGER SLINGER SERVICES INC. 866-302-9886 494 HIGHWAY 3, COURTLAND, ON, NOJ 1E0

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GENERAL DETAILS

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