

Mr. Andrew Wallace Planner Norfolk County 50 Colborne Street South Simcoe, ON N3Y 4H3

Date: February 24, 2025

Our Ref: 148728

Subject: Site Plan Approval - 25 St. Andrew St., Port Dover - 1000033566

Ontario Inc.

Dear Mr. Wallace,

Arcadis Professional Services (Canada) Inc. 360 James Street North Suite 200 Hamilton, Ontario L8L 1H5 Canada

Phone: 905 546 1010 www.arcadis.com

On behalf of our client, 1000033566 Ontario Inc., please accept this letter and the following materials as part of our complete submission of a Site Plan Application ("SPA") for the lands municipally known as 25 St. Andrews St. ("Subject Lands")

The subject lands are legally identified as PDOV PLAN 207 BLK 50 LOT 35. The subject lands are currently occupied by a one-storey residential dwelling. Surrounding land uses include residential to the North, East commercial (Dairy Bar) to the west and a vacant parcel to the South.

The intent of the proposed site plan approval is to redevelop the lands for a surface parking lot which will ultimately owned and operated by the County of Norfolk. The proposed parking lot will replace the existing Clinton Municipal Parking lot located at 33 Clinton Street. The new St. Andrews parking lot proposes twenty-eight (28) standard parking spaces and two (2) accessible parking spaces for a total of thirty (30) parking spaces to replace the 14 spaces within the Clinton Municipal Parking Lot.

The St. Andrews parking lot is planned to provide pedestrian access to Main Street via a proposed pedestrian walkway proposed on the adjacent property to the south which is subject to a separate SPA application process. The relocation of hydro and other utility will be coordinated with the 200 Main Street SPA Application process.

In terms of the current planning framework, the Norfolk County Official Plan ("OP") designates the subject lands as 'Downtown' under Schedule B.17 of the Plan. The proposed municipal parking lot is a permitted land use under the 'Downtown' designation. It is the intent of the new municipal parking lot to improve public parking thereby contributing to the economic viability of the Downtown area. The connection to Main Street via the proposed walkway will provide convenient parking in proximity to the main shopping area. The proposed use is permitted under the OP and no amendment is required.

The subject lands are designated as Central Business District Zone ("CBD") according to Zoning By-Law Norfolk County 1-Z-2014 ("ZBL") which lists 'a parking lot' as one of the permitted uses. As required under the County's by-law, the site plan meets the regulations for Off-Street Parking providing for spaces that are 5.8m x 3.0m with minimum aisle width of 7.3m. Two accessible parking stalls meets the Type A (5.8m x 3.4m) and Type B (5.8m x 2.4m) dimensions with a 1.5m access aisle between the accessible stalls. Therefore, the proposed development does not require an amendment to the ZBL.

Mr. Andrew Wallace Norfolk County February 24, 2025

Please accept the items listed below as part of our complete submission for Site Plan Approval. These have been submitted electronically. If hard copies are required, please advise.

- A Signed Site Plan Application Form
- One (1) copy of the Architectural Package prepared by VanGroll & Associates Inc.
- One (1) copy of the Pre-Development Drainage Area Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Post-Development Drainage Area Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Downstream Analysis Memo prepared by Arcadis Professional Services
- One (1) copy of the Erosion and Sediment Control Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Grading Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Site Servicing Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Cross Section Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the General Notes and Details prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of Storm Water Management Design Report prepared by Arcadis Professional Services (Canada) Inc.

We trust that the attached materials provide you with sufficient information for the processing of this application. Should you have any further questions or require anything more for the processing of this file, please do not hesitate to contact the undersigned.

Sincerely,

Arcadis Professional Services (Canada) Inc.

Carmen Jandu, MCIP RPP Planner – Senior Associate Nickee Digman Planning Technician

CC. Ms. Brooke Hayward, via email.

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Planning Department Development Application Form

Complete Application

A complete development application consists of the following:

- 1. A completed, signed, and notarized application form
- 2. Supporting information adequate to illustrate your proposal as indicated in **Section**H of this application form
- 3. Written authorization from the registered owner of the subject lands where the applicant is not the owner as per Section N
- 4. Cash, debit, credit or cheque payable to Norfolk County in the amount set out in the user fees By-Law that will be accepted and deposited once the application has been deemed complete.

Pre-Submission Consultation:

Norfolk County requires a Pre-Consultation Meeting for all applications; however, minor applications may be exempted depending on the nature of the proposal. The purpose of a Pre-Consultation Meeting is to provide the applicant with an opportunity to present the proposed application, discuss potential issues, and for the Norfolk County and Agency staff to identify the application requirements. Application requirements, as detailed in the Pre-Consultation Meeting Comments, are valid for one year after the meeting date.

Development Application Process

Once an application has been deemed complete by a Planner, Norfolk County staff will circulate the application to adjacent landowners, public agencies, and internal departments for comment. The time involved in application processing varies depending on its complexity, acceptability to the other agencies, and statutory Planning Act decision time-frames.

Payment is required once your application is deemed complete. Pre-payments will not be accepted.



Norfolk County collects personal information submitted through this form under the Municipal Freedom of Information and Protection Act's authority. Norfolk County will use this information for the purposes indicated or implied by this form. You can direct questions about collecting personal information to Norfolk GIS Services at NorfolkGIS@norfolkcounty.ca.

Additional studies required for the complete application shall be at the applicant's sole expense. Sometimes, peer reviews may be necessary to review particular studies at the applicant's expense. In these caseds, Norfolk County staff will select the company to complete the peer review.

Norfolk County will refund the original fee if applicants withdraw their applications before circulation. If Norfolk County must recirculate your drawings, there will be an additional fee. If Norfolk County must do more than three reviews of engineering drawings due to revisions by the owner or failure to revise engineering drawings as requested, Norfolk County will charge an additional fee. Full refunds are only available before Norfolk County has circulated the application.

Notification Sign Requirements

For public notification, Norfolk County will provide you with a sign to indicate the intent and purpose of your development application. It is your responsibility to:

- 1. Post one sign per frontage in a conspicuous location on the subject lands.
- 2. Ensure one sign is posted at the front of the subject lands at least three feet above ground level and not on a tree.
- 3. Notify the Planner when the sign is in place.
- 4. Maintain the sign until the development application is finalized and, after that, remove it.

Contact Us

For additional information or assistance completing this application, please contact a Planner at 519-426-5870 or 519-875-4485 extension 1842 or planning@norfolkcounty.ca. Please submit the completed application and fees to the attention of the Planning Department at 185 Robinson Street, Suite 200, Simcoe, ON N3Y 5L6.



File N Relate Pre-co Applic	fice Use Only: Imber	
Chec	k the type of planning application(s) you are submitting.	
	Official Plan Amendment	
	Zoning By-Law Amendment	
	Temporary Use By-law	
	Draft Plan of Subdivision/Vacant Land Condominium	
	Condominium Exemption	
	Site Plan Application	
	Extension of a Temporary Use By-law	
	Part Lot Control	
	Cash-in-Lieu of Parking	
	Renewable Energy Project or Radio Communication Tower	
provi	e summarize the desired result of this application (for example, a special zoning sion on the subject lands to include additional use(s), changing the zone or offici lesignation of the subject lands, creating a certain number of lots, or similar)	_
_		
_		
-		
_		
_		
Prop	erty Assessment Roll Number:	



A. Applicant Information Name of Owner			
Name of Owner			
Address			
Town and Postal Code			
Phone Number			
Cell Number			
Email			
Name of Applicant			
Address			
Town and Postal Code			
Phone Number			
Cell Number			
Email			
Name of Agent			
Address			
Town and Postal Code			
Phone Number			
Cell Number			
Email			
Unless otherwise directed regarding this application	•	ll forward all correspondence and n agent noted above.	otices
□ Owner	☐ Agent	☐ Applicant	
Names and addresses of encumbrances on the sub		nortgagees, charges or other	



B. Location, Legal Description and Property Information

1.	Legal Description (include Geographic Township, Concession Number, Lot Number, Block Number and Urban Area or Hamlet):		
	Municipal Civic Address:		
	Present Official Plan Designation(s):		
	Present Zoning:		
2.	Is there a special provision or site specific zone on the subject lands?		
	\square Yes \square No If yes, please specify corresponding number:		
3.	Present use of the subject lands:		
4.	Please describe all existing buildings or structures on the subject lands and whether they will be retained, demolished or removed. If retaining the buildings or structures, please describe the type of buildings or structures, and illustrate the setback, in metric units, from the front, rear and side lot lines, ground floor area, gross floor area, lot coverage, number of storeys, width, length, and height on your attached sketch which must be included with your application:		
5.	If an addition to an existing building is being proposed, please explain what it will be used for (for example: bedroom, kitchen, or bathroom). If new fixtures are proposed please describe.		
6.	Please describe all proposed buildings or structures/additions on the subject lands Describe the type of buildings or structures/additions, and illustrate the setback, in metric units, from front, rear and side lot lines, ground floor area, gross floor area, locoverage, number of storeys, width, length, and height on your attached sketch which must be included with your application:		



7.	Are any existing buildings on the subject lands designated under the <i>Ontario</i> Heritage Act as being architecturally and/or historically significant? Yes \Box No \Box
	If yes, identify and provide details of the building:
8.	If known, the length of time the existing uses have continued on the subject lands:
9.	Existing use of abutting properties:
10	Are there any easements or restrictive covenants affecting the subject lands?
	☐ Yes ☐ No If yes, describe the easement or restrictive covenant and its effect:
C.	Purpose of Development Application
No	te: Please complete all that apply.
1.	Please explain what you propose to do on the subject lands/premises which makes this development application necessary:
2.	Please explain why it is not possible to comply with the provision(s) of the Zoning By-law/and or Official Plan:
2	Does the requested amendment alter all or any part of the boundary of an area of
J.	settlement in the municipality or implement a new area of settlement in the municipality? Yes No If yes, describe its effect:
4.	Does the requested amendment remove the subject land from an area of employment? ☐ Yes ☐ No If yes, describe its effect:



	•	d amendment alter, replace, or delete a policy of the Official Plan? s, identify the policy, and also include a proposed text of the
p	olicy amendment	(if additional space is required, please attach a separate sheet):
-		
D	Description of land	intended to be severed in metric units:
F	rontage:	
D	epth:	
٧	Vidth:	
L	ot Area:	
Р	resent Use:	
Ρ	roposed Use:	
Ρ	roposed final lot	size (if boundary adjustment):
lf	a boundary adjus	stment, identify the assessment roll number and property owner o
		the parcel will be added:
		•
D	escription of land	intended to be retained in metric units:
F	rontage:	
D	epth:	
V	Vidth:	
L	ot Area:	
Р	resent Use:	
Р	roposed Use:	
В	Buildings on retain	ed land:
	escription of proprontage:	osed right-of-way/easement:
D	epth:	
V	Vidth:	
Α	irea:	
Р	roposed use:	
Ν	·	, if known, to whom lands or interest in lands to be transferred, (if known):



9.	Site Information	Zoning	Proposed
PΙθ	ease indicate unit of measurem	ent, for example: m, m ² or %	
Lo	t frontage		
Lo	t depth		
Lo	t width		
Lo	t area		
Lo	t coverage		
Fro	ont yard		
Re	ear yard		
Le	ft Interior side yard		
Ri	ght Interior side yard		
Ex	terior side yard (corner lot)		
La	ndscaped open space		
En	trance access width		
Ex	it access width		
Siz	ze of fencing or screening		
Ту	pe of fencing		
10	.Building Size		
Νu	ımber of storeys		
Bu	ilding height		
То	tal ground floor area		
То	tal gross floor area		
То	tal useable floor area		
11	.Off Street Parking and Loading	g Facilities	
Nu	ımber of off street parking spac	es	
Νu	ımber of visitor parking spaces		
Νu	ımber of accessible parking spa	aces	
Nι	ımber of off street loading facilit	ies	



12. Residential (if applicable)		
Number of buildings existing:		
Number of buildings propose	d:	
Is this a conversion or addition	on to an existing building	? □ Yes □ No
If yes, describe:		
Туре	Number of Units	Floor Area per Unit in m2
Single Detached _		
Semi-Detached _		
Duplex _		
Triplex _		
Four-plex _		
Street Townhouse _		
Stacked Townhouse _		
Apartment - Bachelor		
Apartment - One bedroom		
Apartment - Two bedroom		
Apartment - Three bedroom		
Other facilities provided (for e or swimming pool):	example: play facilities, ι	underground parking, games room,
13. Commercial/Industrial Use	es (if applicable)	
Number of buildings existing:		
Number of buildings propose	d:	
Is this a conversion or addition	on to an existing building	? □ Yes □ No
If yes, describe:		
Indicate the gross floor area	by the type of use (for ex	xample: office, retail, or storage):



Seating Capacity (for assembly halls or similar):
Total number of fixed seats:
Describe the type of business(es) proposed:
Total number of staff proposed initially:
Total number of staff proposed in five years:
Maximum number of staff on the largest shift:
Is open storage required: ☐ Yes ☐ No
Is a residential use proposed as part of, or accessory to commercial/industrial use?
☐ Yes ☐ No If yes please describe:
14. Institutional (if applicable)
Describe the type of use proposed:
Seating capacity (if applicable):
Number of beds (if applicable):
Total number of staff proposed initially:
Total number of staff proposed in five years:
Maximum number of staff on the largest shift:
Indicate the gross floor area by the type of use (for example: office, retail, or storage):
15. Describe Recreational or Other Use(s) (if applicable)



D.	Previous Use of the Property
1.	Has there been an industrial or commercial use on the subject lands or adjacent lands? \Box Yes \Box No \Box Unknown
	If yes, specify the uses (for example: gas station or petroleum storage):
^	
2.	Is there reason to believe the subject lands may have been contaminated by former uses on the site or adjacent sites? \square Yes \square No \square Unknown
3.	Provide the information you used to determine the answers to the above questions:
4.	If you answered yes to any of the above questions in Section D, a previous use inventory showing all known former uses of the subject lands, or if appropriate, the adjacent lands, is needed. Is the previous use inventory attached? \square Yes \square No
E.	Provincial Policy
1.	Is the requested amendment consistent with the provincial policy statements issued under subsection 3(1) of the <i>Planning Act, R.S.O. 1990, c. P. 13</i> ? \square Yes \square No
	If no, please explain:
2.	It is owner's responsibility to be aware of and comply with all relevant federal or provincial legislation, municipal by-laws or other agency approvals, including the Endangered Species Act, 2007. Have the subject lands been screened to ensure that development or site alteration will not have any impact on the habitat for endangered or threatened species further to the provincial policy statement subsection 2.1.7? \square Yes \square No
	If no, please explain:



3.	Have the subject lands been screened to ensure that development or site alteration will not have any impact on source water protection? \square Yes \square No
	If no, please explain:
	Note: If in an area of source water Wellhead Protection Area (WHPA) A, B or C please attach relevant information and approved mitigation measures from the Risk Manager Official.
4.	Are any of the following uses or features on the subject lands or within 500 metres of the subject lands, unless otherwise specified? Please check boxes, if applicable.
	Livestock facility or stockyard (submit MDS Calculation with application)
	□ On the subject lands or □ within 500 meters – distance Wooded area □ On the subject lands or □ within 500 meters – distance Municipal Landfill □ On the subject lands or □ within 500 meters – distance Sewage treatment plant or waste stabilization plant □ On the subject lands or □ within 500 meters – distance Provincially significant wetland (class 1, 2 or 3) or other environmental feature □ On the subject lands or □ within 500 meters – distance Floodplain □ On the subject lands or □ within 500 meters – distance Rehabilitated mine site □ On the subject lands or □ within 500 meters – distance Non-operating mine site within one kilometre
	□ On the subject lands or □ within 500 meters – distance Active mine site within one kilometre □ On the subject lands or □ within 500 meters – distance Industrial or commercial use (specify the use(s)) □ On the subject lands or □ within 500 meters – distance Active railway line □ On the subject lands or □ within 500 meters – distance
	Seasonal wetness of lands ☐ On the subject lands or ☐ within 500 meters – distance Erosion ☐ On the subject lands or ☐ within 500 meters – distance Abandoned gas wells ☐ On the subject lands or ☐ within 500 meters – distance



F. Servicing and Access 1. Indicate what services are available or proposed: Water Supply ☐ Municipal piped water □ Communal wells ☐ Individual wells ☐ Other (describe below) Sewage Treatment ☐ Municipal sewers ☐ Communal system ☐ Septic tank and tile bed in good working order ☐ Other (describe below) Storm Drainage ☐ Storm sewers □ Open ditches ☐ Other (describe below) 2. Existing or proposed access to subject lands: ☐ Municipal road ☐ Provincial highway ☐ Unopened road ☐ Other (describe below) Name of road/street: G. Other Information 1. Does the application involve a local business? \square Yes \square No If yes, how many people are employed on the subject lands? 2. Is there any other information that you think may be useful in the review of this

application? If so, explain below or attach on a separate page.



H. Supporting Material to be submitted by Applicant

In order for your application to be considered complete, **folded** hard copies (number of paper copies as directed by the planner) and an **electronic version (PDF) of the properly named site plan drawings, additional plans, studies and reports** will be required, including but not limited to the following details:

- 1. Concept/Layout Plan
- 2. All measurements in metric
- 3. Key map
- 4. Scale, legend and north arrow
- 5. Legal description and municipal address
- 6. Development name
- 7. Drawing title, number, original date and revision dates
- 8. Owner's name, address and telephone number
- 9. Engineer's name, address and telephone number
- 10. Professional engineer's stamp
- 11. Existing and proposed easements and right of ways
- 12. Zoning compliance table required versus proposed
- 13. Parking space totals required and proposed
- 14. All entrances to parking areas marked with directional arrows
- 15. Loading spaces, facilities and routes (for commercial developments)
- 16. All dimensions of the subject lands
- 17. Dimensions and setbacks of all buildings and structures
- 18. Location and setbacks of septic system and well from all existing and proposed lot lines, and all existing and proposed structures
- 19. Gross, ground and useable floor area
- 20. Lot coverage
- 21. Floor area ratio
- 22. Building entrances, building type, height, grades and extent of overhangs
- 23. Names, dimensions and location of adjacent streets including daylighting triangles
- 24. Driveways, curbs, drop curbs, pavement markings, widths, radii and traffic directional signs
- 25. All exterior stairways and ramps with dimensions and setbacks
- 26. Retaining walls including materials proposed
- 27. Fire access and routes
- 28. Location, dimensions and number of parking spaces (including visitor and accessible) and drive aisles
- 29. Location of mechanical room, and other building services (e.g. A/C, HRV)
- 30. Refuse disposal and storage areas including any related screening (if indoors, need notation on site plan)
- 31. Winter snow storage location



- 32. Landscape areas with dimensions
- 33. Natural features, watercourses and trees
- 34. Fire hydrants and utilities location
- 35. Fencing, screening and buffering size, type and location
- 36. All hard surface materials
- 37. Light standards and wall mounted lights (plus a note on the site plan that all outdoor lighting is to be dark sky compliant)
- 38. Business signs (make sure they are not in sight lines)
- 39. Sidewalks and walkways with dimensions
- 40. Pedestrian access routes into site and around site
- 41. Bicycle parking
- 42. Architectural elevations of all building sides
- 43. All other requirements as per the pre-consultation meeting

may also be required as part of the complete application submission:
Zoning Deficiency Form
On-Site Sewage Disposal System Evaluation Form (to verify location and condition)
Architectural Plan
Buildings Elevation Plan
Cut and Fill Plan
Erosion and Sediment Control Plan
Grading and Drainage Control Plan (around perimeter and within site) (existing and proposed)
Landscape Plan
Photometric (Lighting) Plan
Plan and Profile Drawings
Site Servicing Plan
Storm water Management Plan
Street Sign and Traffic Plan
Street Tree Planting Plan
Tree Preservation Plan
Archaeological Assessment
Environmental Impact Study



	Functional Servicing Report		
	Geotechnical Study / Hydrogeological Review		
	Minimum Distance Separation Schedule		
	Noise or Vibration Study		
	Record of Site Condition		
	Storm water Management Report		
	Traffic Impact Study – please contact the Planner to verify the scope required		
Sit	e Plan applications will require the following supporting materials:		
	1. Two (2) complete sets of the site plan drawings folded to 8½ x 11 and an electronic version in PDF format		
	2. Letter requesting that the Holding be removed (if applicable)		
	3. A cost estimate prepared by the applicant's engineer		
	 An estimate for Parkland dedication by a certified land appraiser Property Identification Number (PIN) printout 		
Sta	andard condominium exemptions will require the following supporting materials:		
	Plan of standard condominium (2 paper copies and 1 electronic copy)		
	Draft condominium declaration		
	Property Identification Number (PIN) printout		

Your development approval might also be dependent on other relevant federal or provincial legislation, municipal by-laws or other agency approvals.

All final plans must include the owner's signature as well as the engineer's signature and seal.

I. Development Agreements

A development agreement may be required prior to site plan approval, subdivision and condominium applications. Should this be necessary for your development, you will be contacted by the agreement administrator with further details of the requirements including but not limited to insurance coverage, professional liability for your engineer, additional fees and securities.



J. Transfers, Easements and Postponement of Interest

The owner acknowledges and agrees that if required, it is their solicitor's responsibility on behalf of the owner, to disclose the registration of all transfer(s) of land and/or easement in favour of the County and/or utilities. Also, the owner further acknowledges and agrees that it is their solicitor's responsibility on behalf of the owner for the registration of postponements of any charges in favour of the County.

K. Permission to Enter Subject Lands

Permission is hereby granted to Norfolk County officers, employees or agents, to enter the premises subject to this application for the purposes of making inspections associated with this application, during normal and reasonable working hours.

L. Freedom of Information

For the purposes of the *Municipal Freedom of Information and Protection of Privacy Act*, I authorize and consent to the use by or the disclosure to any person or public body any information that is collected under the authority of the *Planning Act*, *R.S.O.* 1990, c. P. 13 for the purposes of processing this application.

	1000/00
Owner/Applicant Signature	Date
M. Owner's Authorization	
If the applicant/agent is not the registered own application, the owner(s) must complete the au	
I/We 1000033566 Ontario Inc.	am/are the registered owner(s) of the
lands that is the subject of this application.	
I/We authorize Arcadis Professional Services Inc. c/o Carme my/our behalf and to provide any of my/our performs processing of this application. Moreover, this authorization for so doing.	rsonal information necessary for the
	Feb 5/25
Owner	Date
Owner	Date



N. Declaration _{I,} Carmen Jandu	of Town of Oakville, Halton Region
solemnly declare that:	
all of the above statements and the stater transmitted herewith are true and I make believing it to be true and knowing that it i under oath and by virtue of <i>The Canada E</i>	this solemn declaration conscientiously is of the same force and effect as if made
Declared before me at: City of Hamilton	Cfoclee
In	Owner/Applicant)Signature
This 3rd day of February	
A.D., 20 25	
A Commissioner, etc.	Jared Vail Marcus, a Commissioner, etc
V	Province of Ontario,
	for Arcadis Professional Services(Canada) Inc.



Expires June 16, 2026

Memo



SUBJECT

Downstream Sewer Capacity Analysis of Developments at 25 St. Andrew Street and 200 Main Street, Port Dover, Norfolk County DATE

2025-02-19

DEPARTMENT

CA700 CA MOB: D&E Roads & Highways Ontario

COPIES TO Imad Qneibi TO

County of Norfolk

OUR REF

N/A

PROJECT NUMBER

148728

NAME

Rakesh Pandey

Associate Principal - Practice Lead, Municipal Engineering

rakesh.pandey@arcadis.com

1 Objective

This memo documents the storm sewer downstream capacity assessment from the proposed 25 St. Andrew Street and 200 Main Street developments. This memo supports the "DRAFT 25 St. Andrew Street in Port Dover Drainage and Stormwater Management Report" prepared by Arcadis, dated February 12, 2024, and the "DRAFT - 200 Main Street in Port Dover Drainage and Stormwater Management Report", prepared by Arcadis, dated October 10, 2024, under a separate application.

2 Introduction

Arcadis has been retained by the County of Norfolk to assess the downstream for the proposed redevelopment at 200 Main Street and 25 St. Andrew Street in Port Dover, Norfolk County, Ontario, hereafter referred to as "the Site." The Site is located under the jurisdiction of the Long Point Region Conservation Authority, within the Lynn-Black Creek and Dedrick-Young Creek subwatersheds. For the Site's location, existing and proposed downstream sewer legs to the outlets refer to Figures 1 and 2. The native soil was identified as clayey silt and silt as per the 200 Main Street, Port Dover Geotechnical Investigation Report prepared by MTE Consultants Inc. As flows from the proposed developments are only being diverted from the Main Street outlet to the St. Patrick Street outlet, only the storm sewer legs from the Site to the St. Patrick Street outlet will be assessed (as marked in Figures 1 and 2).

2.1 PCSWMM Modelling

A high-level PCSWMM model was used to evaluate the existing and post-development sewer capacity at the downstream sewer legs. Conservative assumptions were made regarding soil characteristics, such as modelling the native site soil as Silty Clay. Various parameters were sourced from PCSWMM for the soil properties and overland Manning's n values. The 5-year Design storm was generated using the Norfolk Design Criteria IDF curves for analysis. An area-wide assumption was made in the existing and proposed conditions, where runoff coefficients were based on a combination of the runoff coefficients from the Stantec Drainage Study Report, and Norfolk County Criteria.

Rakesh Pandey Arcadis Professional Services (Canada) Inc 2025-02-19

Surface data (DEM) was obtained from Ontario GeoHub and various data including drainage areas, the storm sewer lengths, slopes and sizes were based on the proposed storm sewers provided in the "Port Dover Drainage Area Study Report" prepared by Stantec 2001.

Existing inverts around the proposed developments were provided by a survey from the 200 Main Street Report and respective connections were back-calculated based on their slopes from the Stantec Drainage Study Report. The proposed connections to the existing infrastructure were detailed in both the 200 Main Street Report and the 25 St. Andrew Street Report.

3 Results

3.1 Existing Condition Assessment

Please refer to the existing drainage sections in both 25 St. Andrew Street in Port Dover and 200 Main Street Drainage and Stormwater Management Reports. A PCSWMM model has been created for the existing condition 5-year design storm scenario. Table 1 shows the cumulative flow and the maximum flow depth over the full depth represented as a percentage from the PCSWMM results. Refer to the attached Existing Profiles for additional information.

Table 1: Existing Condition: Flow and Percent Full of Storm Sewer Legs to the St. Patrick Street Outlet

Location Manhole No. (1) From To MH-O1-US1 MH11A		Cumulative Flow (L/s)	Percent Full (%)	
		Guindiauve Flow (13)		
		190	61	
MH11A	MH10A	201	48	
MH10A	MH-O1-1	201	59	
MH-O1-1	MH-O1-2	201	71	
MH-O1-2	MH-O1-3	508	58	
MH-O1-3	MH-O1-4	579	69	
MH-O1-4	ST-PATRICK- OUTLET	626	75	

Manhole numbers provided either from surveyed data from 200 Main Street Drainage and Stormwater Management Report, proposed site servicing drawings for 200 Main Street or 25 St Andrew Street or generated for modelling purposes.

2/4

148728 Norfolk County Downstream Assessment Memo

3.1 Proposed Condition Assessment

Please refer to the proposed drainage sections in both 25 St. Andrew Street in Port Dover and 200 Main Street Drainage and Stormwater Management Reports. A PCSWMM model has been created for the proposed condition 5-year design storm scenario. Table 2 shows the cumulative flow and the maximum flow depth over the full depth represented as a percentage from the PCSWMM results. Refer to the attached Proposed Profiles for additional information.

Table 2: Proposed Condition: Flow and Percent Full of Storm Sewer Legs to the St. Patrick Street Outlet

	ation		Percent Full (%)	
Proposed M	anhole No. ⁽¹⁾	Cumulative Flow (L/s)		
From	То			
MH-O1-US1	MH1-Proposed	MH1-Proposed 190		
MH1-Proposed	MH2-Proposed	196	80	
MH2-Proposed	MH2-Proposed MH11A		66	
MH11A	MH10A	230	51	
MH10A	MH-O1-1	230	64	
MH-O1-1	1 MH-O1-2 229	75		
MH-O1-2	MH-O1-3	534	59	
MH-O1-3	MH-O1-4	604	71	
MH-O1-4	ST-PATRICK- OUTLET	650	77	

⁽¹⁾ Manhole numbers provided either from surveyed data from 200 Main Street Drainage and Stormwater Management Report, proposed site servicing drawings for 200 Main Street or 25 St. Andrew Street or generated for modelling purposes.

The upstream connection to MH1-Proposed should be highlighted as the largest percent full depth for the 5-year storm event. No pipe exceeds 95% full capacity as required by the Norfolk County Design Criteria.

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3.2 Capacity Analysis

The storm sewer downstream capacity assessment from the proposed 25 St. Andrew Street and 200 Main Street developments indicated the existing municipal sewer has enough capacity to accommodate flows from the 25 St. Andrew Street and 200 Main Street sites. Table 2 demonstrates the proposed condition results on flow and percent full or surplus capacity of storm sewer legs to the St. Patrick Street Outlet.

4 Conclusions

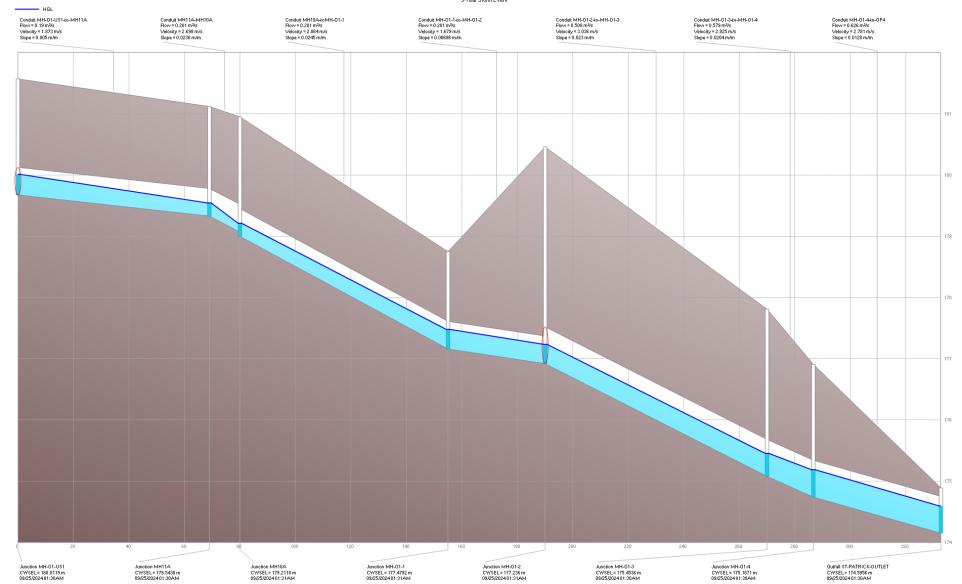
Based on the findings of this assessment, it is concluded that the existing municipal sewer has enough capacity to accommodate flows from the proposed developments at 25 St. Andrew Street and 200 Main Street and no pipe exceeds 95% full capacity as required by the Norfolk County Design Criteria.

Enc. Figures 1 & 2 of Existing and Proposed Drawings, and Existing and Proposed PCSWMM Profiles

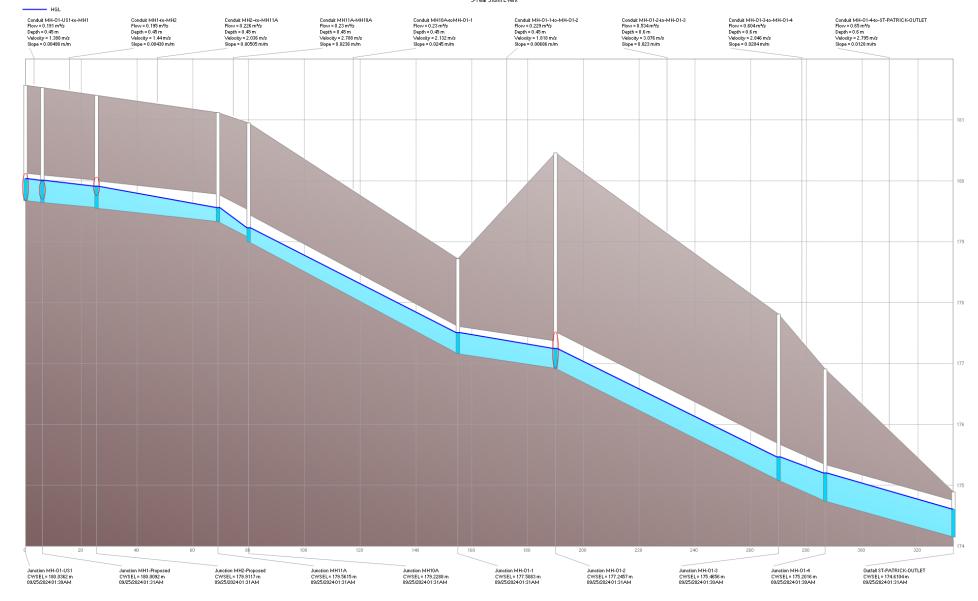




Existing - St. Patrick Street Outlet 5-Year Storm Event



Proposed - St. Patrick Street Outlet 5-Year Storm Event







Drainage and Stormwater Management Report

25 St. Andrew Street, Port Dover, Norfolk County, ON

February 13, 2025

Drainage and Stormwater Management Report

25 St. Andrew Street, Port Dover, Norfolk County, ON

February 13, 2025

Prepared By:

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Version Control

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SWM	00	13 February 2025		Drainage and SWM Report	Imad Qneibi

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Appendix C - Proposed Grading Plan

Appendix D - Site Servicing Plan & General Notes and Details

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Appendix F - Stormwater Calculations

Appendix G - PCSWMM Parameters

Appendix H - Stormceptor OGS Model Specifications

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Preamble

This Drainage and Stormwater Management Report complements the "DRAFT - 200 Main Street in Port Dover Drainage and Stormwater Management Report", prepared by Arcadis, dated October 10, 2024, under a separate application.

1 Introduction

Arcadis has been retained by the County of Norfolk to prepare a Drainage and Stormwater Management Report for the proposed redevelopment at 25 St. Andrew Street in Port Dover, Norfolk County, Ontario, hereafter referred to as "the Site." The Site is located under the jurisdiction of the Long Point Region Conservation Authority, within the Lynn-Black Creek and Dedrick-Young Creek subwatersheds. The site is bounded by the 200 Main Street property to the south, and St. Andrew Street to the east. Residential properties bound the site to the north. Refer to Figure 1 in the Appendix for the site location. The Site is divided into two subcatchments areas of 0.06 ha and 0.04 ha respectively.

The existing Site consists of a residential property with a grassed yard, a two-storey garage, and a residential area to the north of the site. Groundwater data south of the site was encountered at elevations ranging from 174.6 to 178 masl, and the native soil was identified as clayey silt and silt as per the 200 Main Street, Port Dover Geotechnical Investigation Report prepared by MTE Consultants Inc.

The proposed parking lot is to be located northwest and adjacent to the proposed development at 200 Main Street northwest of the site, refer to the "DRAFT - 200 Main Street in Port Dover Drainage and Stormwater Management Report" for that corresponding application. Addressing the drainage of the external area indicated in that report will be included.

The objective of this Drainage and Stormwater Management Report is to verify and summarize recommended drainage and stormwater management strategy while identifying opportunities for improvements that exist within the study limits to mitigate the possible impacts of the proposed redevelopment to the receiving drainage systems. A PCSWMM model was developed to support the findings of this report.

1.1 Background Information and Design Criteria

In preparation for this report, the following relevant documents were reviewed:

- DRAFT 200 Main Street in Port Dover Drainage and Stormwater Management Report, prepared by Arcadis
 October 2024.
- 200 Main Street, Port Dover Geotechnical Investigation Report prepared by MTE Consultants Inc., June 2024.
- Norfolk County Official Plan, January 2021.
- The Corporation of Norfolk County Design Criteria, February 2019.
- Ministry of Environment, Conservation and Parks (MECP) Design Criteria for Sanitary Sewers, Storm Sewers and Forcemains for Alterations Authorized under an Environmental Compliance Approval, May 2023.
- Ministry of Environment, Conservation and Parks (MECP) Stormwater Management Planning and Design Manual, March 2003.

In compliance with Norfolk County's current policies and guidelines, the following design criteria have been applied in developing the drainage and stormwater management strategies for this project:

Water Quality

 Water quality treatment shall conform to MECP requirement for Enhanced Level of Protection (Level 1), i.e., removal of 80% of Total Suspended Solids (TSS) from runoff prior to discharge into the receiving water body.

Water Quantity / Peak Flow Control

According to the County's guidelines, peak flows discharged from the site shall not increase as a result of the
proposed development for the 2-year to the 100-year storm events. All stormwater management measures
must meet the "Enhanced" level of protection as per the MECP Stormwater Management Planning and Design
Manual.

IDF Curves

Norfolk County IDF curves are to be used for hydrological analyses.

It should be noted the Site is not located in the Dingle Creek subwatershed nor greater than 0.40 ha and is therefore not subject to the special policy water management objectives outlined in the Norfolk Design Criteria.

1.2 PCSWMM Modelling

A high-level PCSWMM model was used to evaluate the existing and post-development conditions. Conservative assumptions were made regarding soil characteristics, such as modelling the native site soil as Silty Clay. Various parameters were sourced from PCSWMM for the soil and overland Manning's n values. Design storms for 2, 5, 10, and 100 years were generated using the Norfolk Design Criteria IDF curves for analysis. An additional 25 mm 4-hour Chicago design storm hyetograph from a comparable area in southern Ontario (City of Pickering) was applied for analysis of the runoff volume. A site-wide assumption was made in the existing and proposed conditions, where runoff coefficients were used from the Norfolk Design Criteria based on the appropriate cover. See Appendix G for modelling parameters and additional information.

2 Existing Conditions

2.1 Existing Drainage Patterns

The study area, including 25 St. Andrew Street, is predominantly urban-residential. Drainage is influenced by topography, land cover, and grade variations around the Site. The primary drainage infrastructure, including catchbasins, storm drains, and municipal storm sewers, is located outside the study area. This includes the 450 mm storm sewer running along St. Andrew Street. Refer to Appendix C Site Servicing Plan for additional existing stormwater infrastructure.

The overland flow from the west portion of the site flows southwest, and the overland flow of the east portion of the site flows southeast, both portions diverging from the middle of the property. The east portion of the site enters an existing catchbasin on St. Andrew Street. The west portion drains southwest toward the 200 Main Street property, ultimately discharging eastward to a catchbasin on Clinton Street. See Appendix A for the pre-development drainage area plan.

As mentioned in the previous report, "DRAFT—200 Main Street in Port Dover Drainage and Stormwater Management Report", the external drainage area (EXT01) will be addressed in this later application. Previously, this area was 0.039 ha and located southwest of the site. Figure 1 shows that this area now overlaps with the A1 subcatchment by 0.01 ha. For the current evaluation, the EXT-01 subcatchment is considered to be 0.028 ha and, in its existing condition, drains overland towards the 200 Main Street site. Additional details can be found in Figure 1 and the calculations in Appendix D.

3 Proposed Conditions

3.1 Proposed Drainage Patterns

The proposed redevelopment primarily includes a 0.10 ha parking lot, with most stormwater captured overland through two proposed catchbasins and one OGS unit with an inlet. As mentioned in the Existing Drainage Patterns section above, the external subcatchment area (EXT01) located southwest of the site has an area of 0.028 ha. The external area flows overland to a catchbasin (DCIB) located on the southwest of the site. This catchbasin connects north to a catchbasin manhole (CBMH1), which collects the flow from the west portion of the site. This catchbasin manhole connects northeast, where the east portion of the site will flow northwest into a catchbasin manhole (CBMH2). These pipes are sized to 450 mm. The cumulative flow is then stored in a cast-in-place concrete storage tank (6.0 m x 5.0 m x 0.9 m). The storage chamber outlets the flow controlled by an orifice plate, which is then treated by an EFO4 OGS unit before entering the 300 mm service connection and discharging into the 450 mm diameter municipal stormwater system on St. Andrew Street. See Appendix B, C and D for the Post-development Drainage Area Plan, Proposed Grading Plan, Site Servicing Plan and General Notes for additional details.

3.2 Water Quantity / Peak Flow Control

The Norfolk County Design Criteria provided IDF curves and runoff coefficient guidelines. Peak flows were calculated for both subcatchments. The table below shows the PCSWMM model results for the subcatchment peak flows for the 2-year to 100-year 4-hour Chicago storm events under existing and proposed conditions. See Appendix F for further information regarding the IDF curve and runoff coefficient data.

Table 1: Analysis of Subcatchments and Runoff Peak Flow Calculations

	Total	Percent			Pe	ak Flow (m	³ /s)	
Catchment ID	Area Impe	Imperv. (%)	nperv. Runoff	2-Year	5-Year	10-Year	25-Year	100-Year
Existing								
A1PRE	0.060	30.77	0.45	0.006	0.009	0.012	0.015	0.020
A2PRE	0.040	30.77	0.45	0.005	0.007	0.009	0.011	0.014
EXT01	0.028	34.62	0.48	0.004	0.007	0.008	0.010	0.012
Proposed Uncontrolled								
A1POST	0.060	100	0.90	0.016	0.022	0.026	0.031	0.039
A2POST	0.040	100	0.90	0.008	0.011	0.013	0.015	0.018
EXT01	0.028	34.62	0.48	0.004	0.007	0.008	0.010	0.012

According to Norfolk County guidelines, the proposed peak flows must not exceed existing conditions. In the current drainage pattern, the A1PRE and EXT01 subcatchments drain into the Clinton Street storm sewer, while the A2PRE peak flows enter the St. Andrew Street storm sewer. For the proposed condition, the A1POST, A2POST, and EXT01 subcatchments will all contribute their cumulative flows into the St. Andrew Street storm sewer. Therefore, the peak flows entering the St. Andrew Street storm sewer from the existing condition will be the maximum values for the post-development scenario.

Table 2 highlights the existing and uncontrolled proposed runoff volumes for design storms from 25 mm to 100-year events.

Table 2: Analysis and Uncontrolled Proposed Runoff Volume Calculations

			Rur	off Volume (
Subcatchment	25 mm	2-Year	5-Year	10-Year	25-Year	100-Year
Existing						
A1PRE	7.22	13.20	21.09	26.34	33.12	43.73
A2PRE	4.84	8.807	14.01	17.47	21.94	28.93
EXT01	3.79	6.68	10.43	12.92	16.14	21.14
Uncontrolled Proposed						
A1POST	14.82	21.11	29.60	35.07	42.10	52.96
A2POST	9.74	13.87	19.45	23.05	27.67	34.81
EXT01	3.79	6.68	10.43	12.92	16.14	21.14

To control the post-development 100-yr peak flow to the existing conditions, the PCSWMM calculated that 32.4 m^3 was required for storage. The proposed $6.0 \text{m} \times 5.0 \text{m} \times 0.9$ concrete storage tank would provide 27 m^3 of internal storage. In addition to a proposed concrete storage tank, the required volume is met by the storage capacity within the pipes and catchbasins. Table 3 highlights the on-site storage totals and shows that the site can contain the required volume ($38.6 \text{ m}^3 > 32.4 \text{ m}^3$) as demonstrated by the 100-year design storm on-site. With the proposed total storage of 38.6 m^3 , no flooding occurred at any node during the 100-year design storm, as demonstrated by the proposed PCSWMM model results. See the calculations in Appendix F and the Site Servicing Plan in Appendix D for additional details.

Table 3: Analysis of Proposed Runoff Volume Retained On-site

Proposed Concrete Storage Chamber (m³)	Pipe and Manhole Storage Volume (m³)	TOTAL on-site (m³)	Total Volume On-Site Volume Required from 100-year (m³)
27.0	11.6	38.6	32.4

An orifice plate in the control manhole was sized at a diameter of 1.5 inches (38 mm) to ensure the controlled peak flow did not exceed the existing peak flow for the 2-year to 100-year storm events while not surcharging the municipal sewer storm connection. Table 4 highlights peak flows for the existing, uncontrolled and controlled post-development conditions.

Table 4: Existing and Controlled Proposed Peak Flows (38 mm & 75 mm Orifice Plate)

	Peak Flow (m³/s)					
Outlet	25 mm	2-Year	5-Year	10-Year	25-Year	100-Year
			Existing			
St. Andrew Street Catchbasin	0.002	0.005	0.007	0.009	0.011	0.014
		U	ncontrolled Pro	posed		
No Orifice - Service Connection to Municipal Storm Sewer	0.012	0.020	0.027	0.032	0.037	0.045
	Controll	ed Propose	ed to Storm Ser	vice Connect	ion (38 mm)	
Service Connection to Municipal Storm Sewer	0.002	0.003	0.003	0.003	0.003	0.004
Controlled Proposed to Storm Service Connection (75 mm)						
Service Connection to Municipal Storm Sewer	0.005	0.007	0.008	0.008	0.009	0.011

However, it should be noted that the recommended size for the orifice as per the MECP Stormwater Management Planning and Design Manual 2003 would be 75 mm due to clogging concerns. For practical design purposes, a 75 mm orifice plate would meet the existing condition criteria to mitigate post-development 10- to 100-year flow conditions as shown in Table 4.

3.3 Water Quality

An Oil and Grit Separator (OGS) was sized for the site to meet the 80% TSS removal requirement as per County Criteria. Appendix H outlines specifications for the proposed Stormceptor Imbrium EFO4 Model.

3.4 Storm Service Connection

Per Norfolk County's guidelines, an orifice plate outlet control has been sized for discharging to the municipal sewer. The controlled peak flow does not exceed the existing peak flow for the 5-year storm event while not surcharging the storm service connection. Table 5 below shows the summary of the storm service connection. For additional details see the Site Servicing Plan and Proposed Cross-Section in Appendix D and E. The capacity calculations are included in Appendix F.

Table 5: Storm Service Connection Summary

From	То	Pipe Diameter (mm)	Slope (%)	Peak Flow (L/s)	Capacity (L/s)	Percent Full (%)
Stormceptor OGS Unit	Municipal Storm Sewer	300	0.31	8	61.2	14.9

3.5 Sediment and Erosion Control

An Erosion and Sediment Control Plan has been developed that implements best practice construction measures to minimize disturbance of sediment/erosion. See Appendix I for the Erosion and Sediment Control Plan.

4 Conclusions

The proposed redevelopment of 25 St. Andrew Street in Port Dover has been thoroughly analyzed in terms of drainage and stormwater management. The following findings and recommendations have been derived based on the detailed hydrological assessments, PCSWMM modelling, and relevant design criteria:

- The existing site consists of a residential property, with native soils identified as clayey silt and silt. The south neighbouring property (200 Main Street) groundwater levels range between elevations of 174.6 to 178 masl.
- The proposed redevelopment will be a 0.10 ha municipal parking lot in addition to a 0.028 ha external area.
- Stormwater will be primarily captured by three proposed catchbasins, with the cumulative captured flow stored in a cast-in place concrete storage tank. This flow is then controlled by an orifice plate and then treated by an OGS unit then directed to the municipal storm sewer on St. Andrew Street.
- PCSWMM modelling indicates a required 32.4 m³ volume which 38.6 m³ can be contained on-site to meet criteria.
- Water quality treatment will be addressed through a Stormceptor EFO4 Oil Grit Separator (OGS), which
 meets Norfolk County's requirement of 80% Total Suspended Solids (TSS) removal. Regular maintenance of
 the OGS is recommended to ensure ongoing compliance with water quality standards.
- A 38 mm orifice plate was sized to ensure the controlled peak flow does not exceed existing peak 2- to 100year flow conditions.
- However, an orifice plate sized to 75 mm would meet minimum provincial guidelines to avoid clogging and would ensure the controlled peak flow does not exceed existing peak 10- to 100-year flow conditions.
- The storm service connection has been sized to ensure the capacity for the required release rate.
- A sediment and erosion control plan for construction has been developed and provided.
- A downstream storm sewer analysis will be reviewed and submitted to the county at a later date.

Respectfully Submitted:

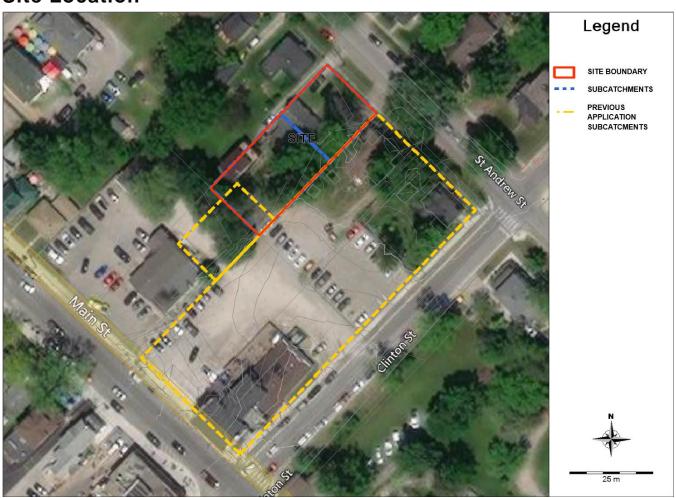


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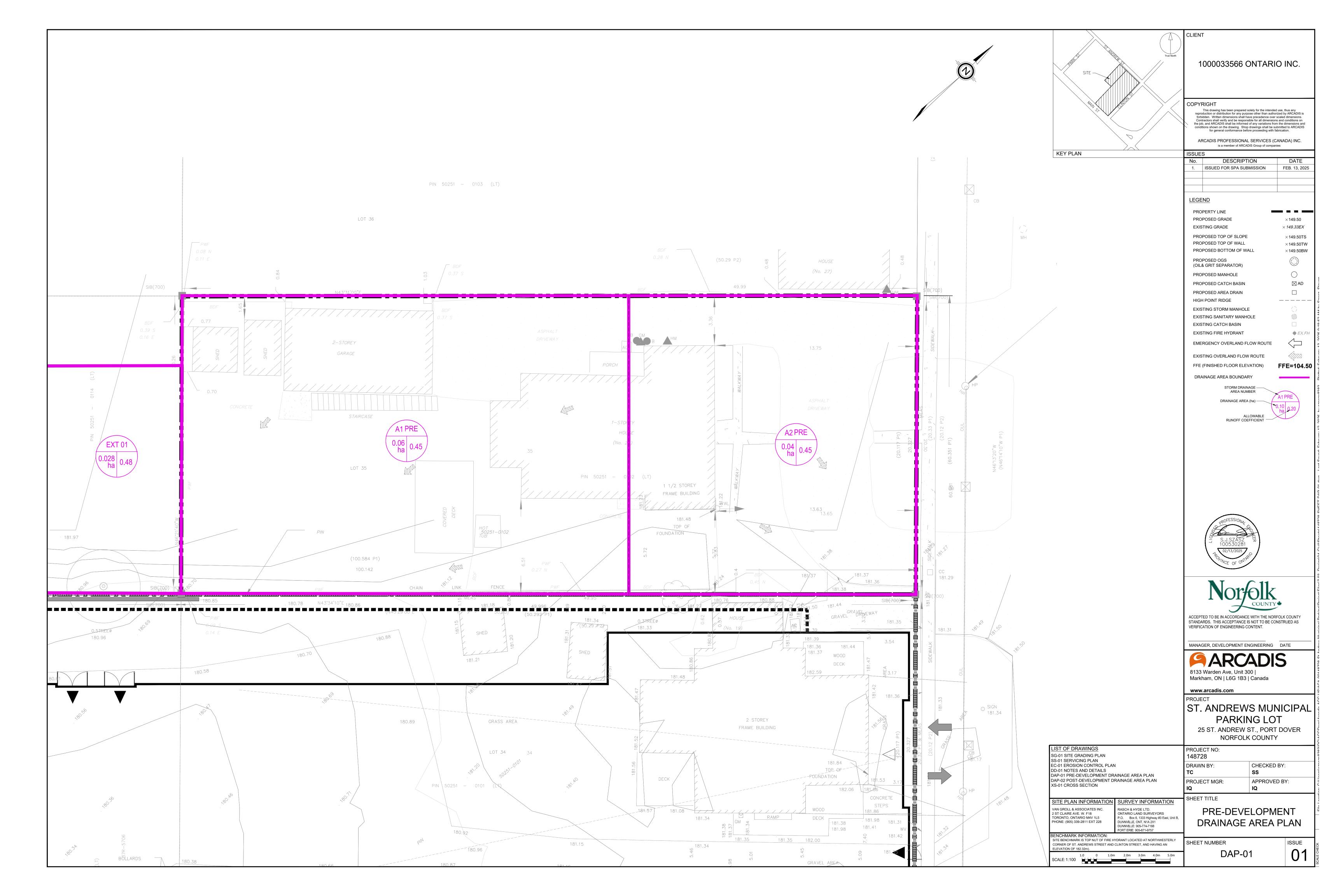
Figure 1

Site Location



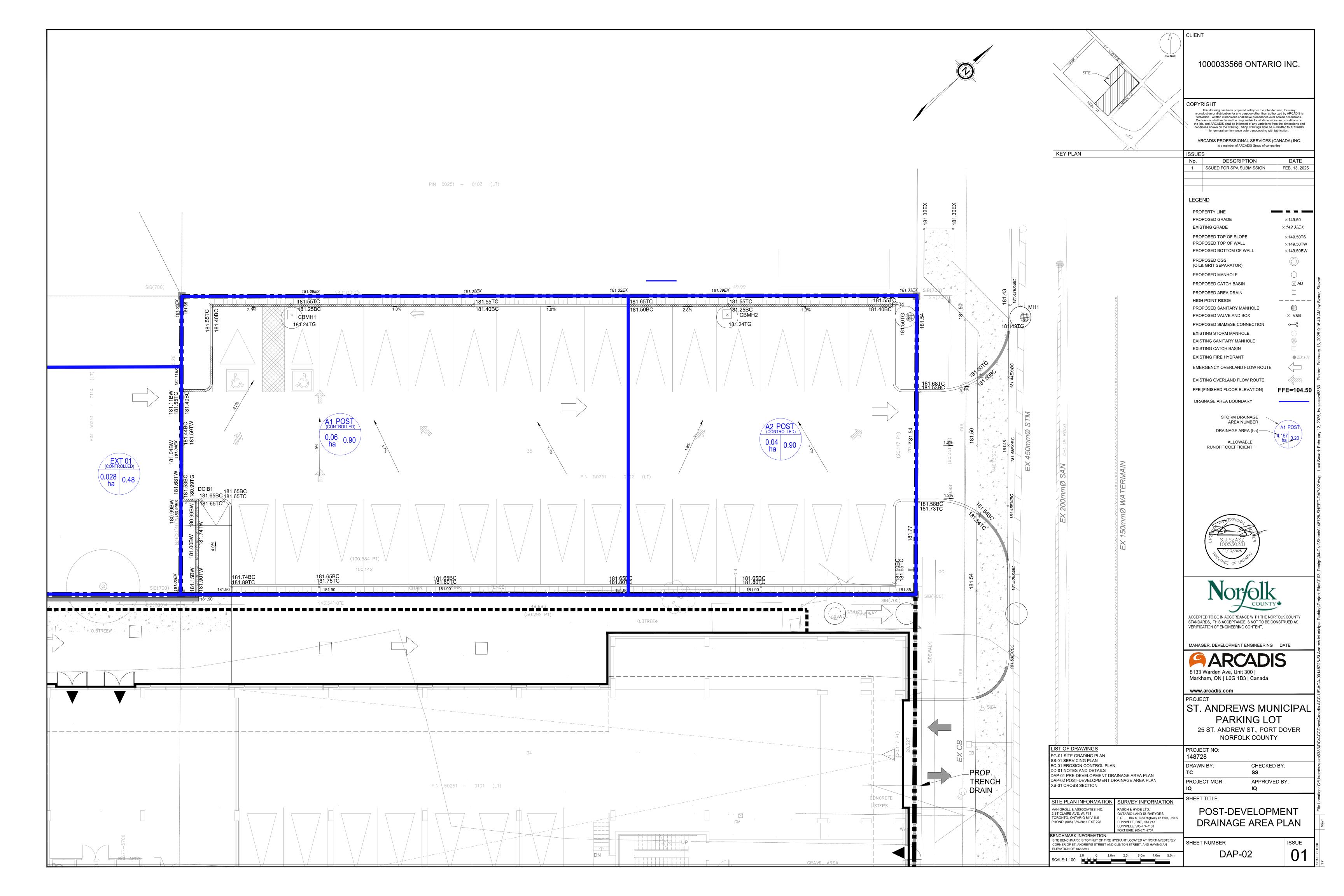
Appendix A

Pre-Development Drainage Area Plan



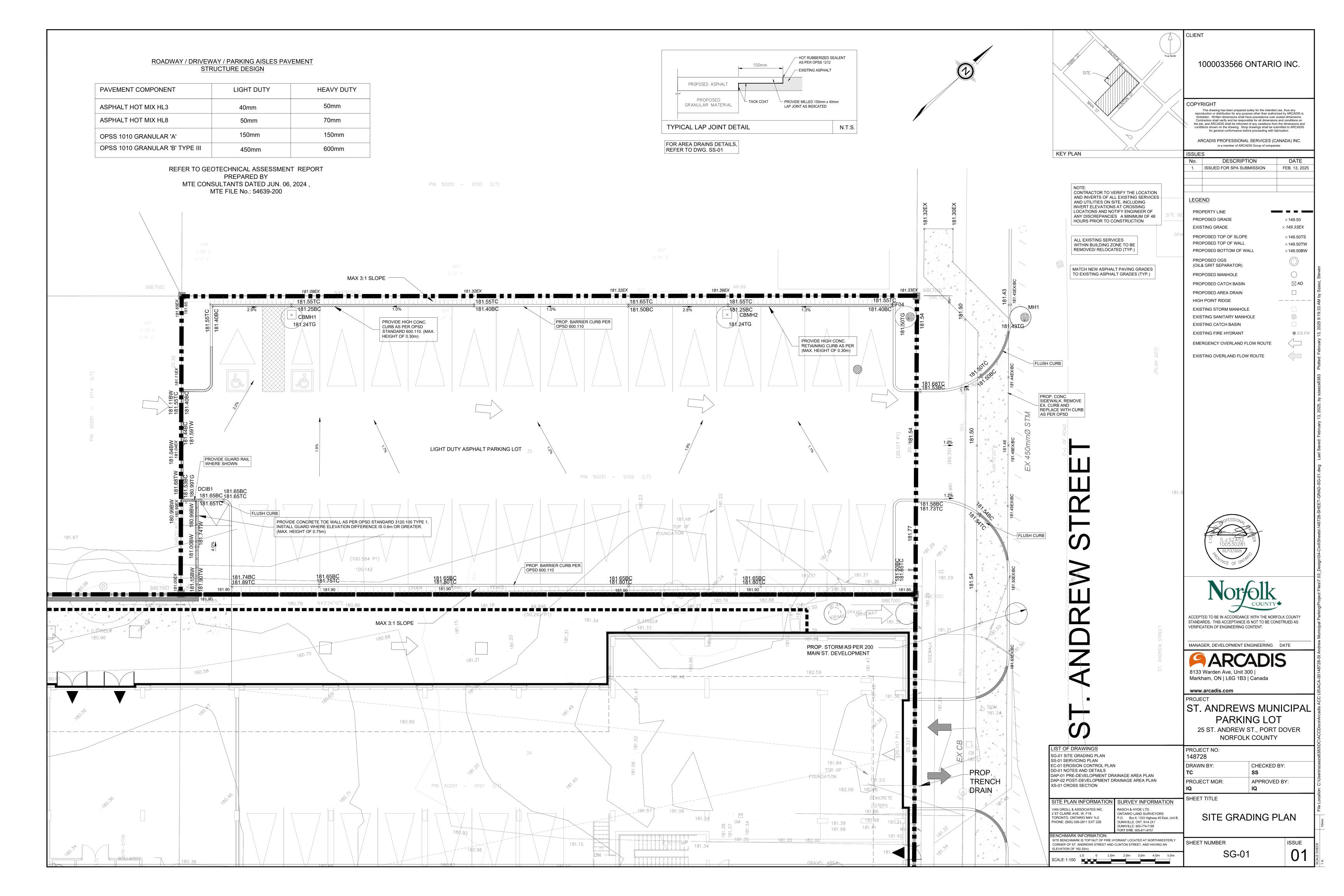
Appendix B

Post-Development Drainage Area Plan



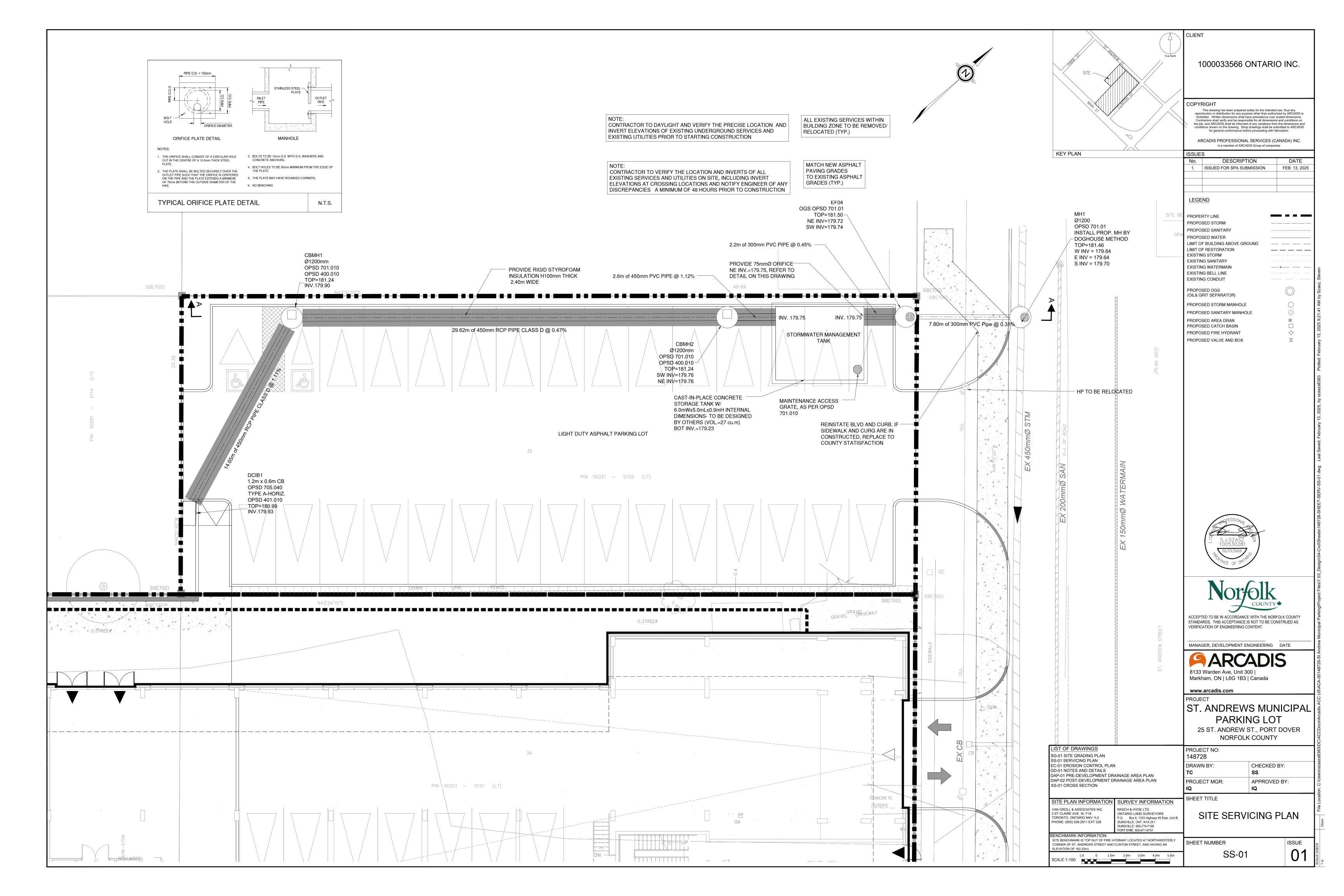
Appendix C

Proposed Grading Plan



Appendix D

Site Servicing Plan & General Notes and Details



GENERAL NOTES:

- PRIOR TO STARTING ANY WORKS, THE CONTRACTOR MUST ENSURE THAT ALL NECESSARY APPROVALS ARE IN PLACE FROM THE CORPORATION OF NORFOLK COUNTY AND OTHER EXTERNAL AGENCIES, AS REQUIRED.
- 2. ALL WORK SHALL BE CARRIED OUT IN COMPLIANCE WITH THE APPLICABLE HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
- 3. ALL WORK AND MATERIALS TO CONFORM WITH THE CURRENT PROVINCIAL BUILDING CODE, MINISTRY OF THE ENVIRONMENT OF ONTARIO, NORFOLK COUNTY, AND ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS. LOCAL UTILITY STANDARDS AND MINISTRY OF TRANSPORTATION STANDARDS WILL APPLY WHERE REQUIRED.
- 4. FOR ALL CONSTRUCTION DETAILS NOT SHOWN ON THE DRAWINGS, REFERENCE SHALL BE MADE TO THE DESIGN STANDARDS OF THE CORPORATION OF NORFOLK COUNTY.
- 5. THE CONTRACTOR IS ADVISED THAT WORKS BY OTHERS MAY BE ONGOING DURING THE PERIOD OF THIS CONTRACT. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH ALL OTHER CONTRACTORS AND PREVENT CONSTRUCTION CONFLICTS.
- 6. THE INFORMATION SHOWN FOR EXISTING UTILITIES WAS PROVIDED BY OTHERS. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES DURING CONSTRUCTION. ALL EXISTING UTILITIES MUST BE LOCATED AND VERIFIED BY EACH PROVIDER PRIOR TO COMMENCEMENT OF WORK. ANY VARIANCE IS TO BE REPORTED TO THE ENGINEER 48 HRS PRIOR TO CONSTRUCTION, LOST TIME AND/OR ANY ADDITIONAL WORKS DUE TO FAILURE OF THE CONTRACTOR TO CONFIRM UTILITY LOCATIONS AND NOTIFY THE ENGINEER OF ANY CONFLICTS 48 HRS PRIOR TO CONSTRUCTION WILL BE AT THE CONTRACTORS EXPENSE.
- THE CONTRACTOR MUST INSTALL ALL SEDIMENT CONTROL DEVICES PRIOR TO THE COMMENCEMENT OF SITE GRADING WORKS, SILT LADEN WATER MUST NOT BE PERMITTED TO ENTER INTO ANY EXISTING CATCH BASINS INLETTING STRUCTURES, OR WATERCOURSES, ADDITIONAL CONTROLS AS DEEMED REQUIRED BY THE AUTHORITIES AND/OR THE ENGINEER DURING CONSTRUCTION ACTIVITIES SHALL BE PROVIDED BY THE CONTRACTOR. THE CONTRACTOR MUST INSPECT SEDIMENT CONTROLS ON A REGULAR BASIS AND AFTER EVERY RAINFALL EVENT. REPAIRS MUST BE DONE IN A TIMELY MANNER TO PREVENT SEDIMENT FROM ENTERING ANY WATER SYSTEMS. ADDITIONAL SILT FENCING MUST BE AVAILABLE IN CASE IMMEDIATE REPAIR IS
- 8. ALL DIMENSIONS, ELEVATIONS AND OTHER INFORMATION SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR 72 HOURS PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES FOUND MUST BE REPORTED
- 9. THE CONTRACTOR IS TO PROVIDE A TOTAL OF TWO CCTV CAMERA INSPECTIONS OF ALL SANITARY AND STORM SEWERS. INCLUDING PICTORIAL REPORT, TWO CD COPIES AND ONE VIDEO TAPE IN A FORMAT SATISFACTORY TO THE ENGINEER. ALL SEWERS ARE TO BE FLUSHED PRIOR TO CAMERA INSPECTION.
- 10. LASER ALIGNMENT CONTROL TO BE UTILIZED ON ALL SEWER INSTALLATIONS.
- 11. ALL PVC SANITARY SEWERS TO BE MANDREL AND AIR TESTED.
- 12. ALL PVC STORM SEWERS TO BE MANDREL TESTED. AIR TEST ONLY ON RECOMMENDATION BY SOIL

CONSTRUCTION NOTES:

- 1. AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION FOR SERVICES ON ANY EXISTING ROAD ALLOWANCE AND EXISTING SERVICES WITHIN AN EXISTING EASEMENT MAINTAINED BY NORFOLK COUNTY IN PORT DOVER ONTARIO, THE SUBDIVIDER / DEVELOPER IS TO OBTAIN A PERMIT OF APPROVED WORK FROM NORFOLK COUNTY AND PORT DOVER ONTARIO.
- 2. THE CONTRACTOR SHALL CONSTRUCT TEMPORARY MEASURES TO CONTROL SILT ENTERING THE STORM DRAINAGE SYSTEM TO THE SPECIFICATIONS OUTLINED IN THE GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES PREPARED BY THE MINISTRY OF NATURAL RESOURCES. THESE MEASURES ARE TO BE INSTALLED PRIOR TO COMMENCING ANY CONSTRUCTION FOR THIS PROJECT, AND ARE TO REMAIN IN PLACE UNTIL CONSTRUCTION HAS BEEN COMPLETED TO THE SPECIFICATIONS OF THE TOWN
- 3. ALL WORK SHALL MEET THE MINIMUM STANDARDS AND SPECIFICATIONS OF NORFOLK COUNTY AND PORT DOVER ONTARIO OR ONTARIO PROVINCIAL STANDARDS.
- 4. NO FOUNDATION DRAIN CONNECTIONS WILL BE PERMITTED. SUMP PUMPS SHALL BE DISCHARGED TO THE OUTSIDE GROUND SURFACE DRAINING AWAY FROM THE BUILDING. REQUIRED TO CONFORM WITH OBC, DIV.B.9.14.
- 5. THE SUBDIVIDER / DEVELOPER IS TO NOTIFY NORFOLK COUNTY AND PORT DOVER ONTARIO AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION. PERMITS ARE REQUIRED PRIOR TO COMMENCEMENT OF WORK AND 48 HOURS IS REQUIRED PRIOR TO INSPECTION REQUESTS.
- 6. THE SUBDIVIDER / DEVELOPER IS TO MEET ALL REQUIREMENTS OF THE OWNERS OF THE UTILITIES ON THIS PLAN. AND MUST MAKE SATISFACTORY ARRANGEMENTS WITH THE UTILITY COMPANIES FOR CROSSING THEIR INSTALLATIONS AND FOR PROVIDING ADEQUATE PROTECTION DURING CONSTRUCTION.
- 7. IF COMMON TRENCH CONSTRUCTION IS TO BE USED FOR THE INSTALLATION OF STORM AND SANITARY SEWERS ON ANY STREET WITHIN THIS SUBDIVISION / DEVELOPMENT / PROJECT, THE PDC'S ARE TO BE CONSTRUCTED AT LEAST 2 METERS BEHIND THE CURB LINE ON THAT STREET, AT THE SAME TIME AS INSTALLATION OF THE SEWERS.
- 8. ALL ORGANIC, UNSTABLE OR UNSUITABLE MATERIALS BENEATH THE ROAD ALLOWANCES OR HOUSE FOUNDATIONS MUST BE REMOVED AND THESE AREAS BACKFILLED WITH AN APPROVED FILL MATERIAL, ALL TO THE SATISFACTION OF A GEOTHECHNICAL ENGINEER.

GRADING:

- PRIOR TO COMMENCEMENT OF GRADING WORKS ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSTALLED AND OPERATIONAL. THE CONTRACTOR SHALL MAINTAIN ALL WORKS UNTIL SERVICING CONSTRUCTION IS COMPLETED TO THE SATISFACTION OF THE ENGINEER AND THE CORPORATION OF NORFOLK
- 2. ALL GRANULAR BASE AND SUB-BASE COURSE MATERIALS SHALL BE COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.
- 3. PAVEMENT STRUCTURE TO BE CONSTRUCTED AS RECOMMENDED BY THE GEOTECHNICAL REPORT.
- 4. CONCRETE CURBS SHALL BE AS PER OPSD 600.060 AND OPSD 600.110.
- 5. INSPECTIONS: ALL WORK ON THE MUNICIPAL RIGHT-OF-WAY AND EASEMENTS TO BE INSPECTED BY THE MUNICIPALITY PRIOR TO BACKFILLING. ALL WORK RELATING TO WATERMAINS AND SEWERS TO BE INSPECTED BY THE MUNICIPALITY WHEN REQUIRED BY THE MUNICIPALITY.
- 6. CONTRACTOR TO OBTAIN A ROAD OCCUPANCY PERMIT 48 HOURS PRIOR TO COMMENCING ANY WORK WITHIN THE MUNICIPAL ROAD ALLOWANCE IF REQUIRED BY THE MUNICIPALITY OR THE REGION.
- 7. EMBANKMENTS TO BE SLOPED AT MAX. 3:1, UNLESS OTHERWISE SPECIFIED.
- 8. SEDIMENT CONTROL TO BE PROVIDED AT CATCH BASINS AND CATCH BASIN MANHOLES UPON INSTALLATION OF STRUCTURES AS PER DETAIL PROVIDED.
- 9. CONTRACTOR WILL BE RESPONSIBLE FOR ALL REMOVALS AS REQUIRED TO FACILITATE NEW CONSTRUCTION. ALL EXISTING STRUCTURES, VALVES, ETC. ARE TO BE ADJUSTED TO PROPOSED ELEVATIONS.
- 10. EXISTING ELEVATIONS AT MATCH POINTS, AS SHOWN ON PLANS, ARE TO BE CONFIRMED BY THE CONTRACTOR 72 HOURS PRIOR TO MOBILIZATION OF FORCES. LOST TIME AND/OR ANY ADDITIONAL WORKS DUE TO FAILURE OF THE CONTRACTOR TO CONFIRM EXISTING ELEVATIONS AND NOTIFY THE ENGINEER OF POSSIBLE CONFLICTS 72 HOURS PRIOR TO MOBILIZATION WILL BE AT THE EXPENSE OF THE CONTRACTOR.

MANHOLES AND CATCHBASINS:

- 1. ALL PRECAST CONCRETE MANHOLES TO MEET M.O.E. SPECIFICATIONS AND CONFORM TO OPSD STANDARDS 701.010 AND 701.011.
- 2. MANHOLE COVERS TO BE AS PER OPSD 401.010, TYPE 'A' FOR SANITARY AND TYPE 'B' FOR STORM.
- 4. MANHOLE STEPS SHALL BE RECTANGULAR STAINLESS STEEL AS PER OPSD 405.010.

3. MANHOLE AND CATCHBASIN ADJUSTERS SHALL BE AS PER OPSD 704.010.

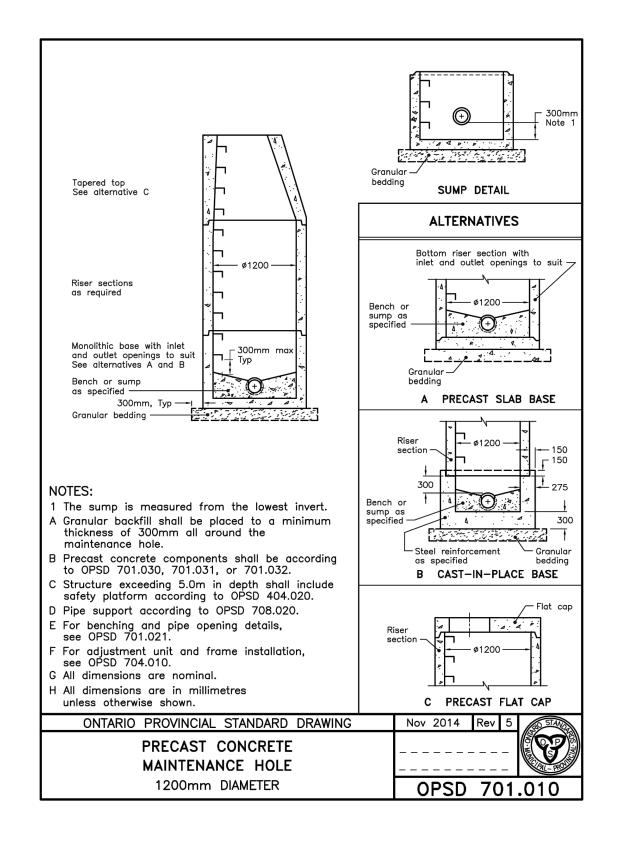
- 5. SAFETY PLATFORMS SHALL BE PROVIDED, AS PER OPSD 404.020, FOR MANHOLES WITH DEPTH EXCEEDING 5.0m.
- 6. BENCHING TO BE PROVIDED AT ALL MANHOLES UNLESS OTHERWISE STATED IN ACCORDANCE WITH OPSD 701.021
- 7. ALL SINGLE AND DOUBLE CATCH BASINS SHALL BE PRECAST AS PER OPSD 705.010 AND 705.020 RESPECTIVELY.
- CATCHBASIN LEADS SHALL BE PVC SDR 35 INSTALLED AT A MINIMUM OF 1.0% SLOPE AND WITH MINIMUM COVER OF 1.2m. SIZES ARE AS FOLLOWS: SINGLE CB- 250mm DIA., DOUBLE CB- 300mm DIA., REAR LOT CB- 300mm DIA.
- 9. CATCHBASIN SUMP TO BE 0.60m DEEP FOR 600x600, AND 0.30m DEEP FOR 1200x1200.
- 10. ALL CATCH BASIN FRAMES AND COVERS SHALL BE AS PER OPSD 400.110.
- 11. DURING CONSTRUCTION ALL CATCH BASINS SHALL BE EQUIPPED WITH A TEMPORARY SEDIMENT CONTROL DEVICE.
- 12. ALL MANHOLE AND CATCH BASIN EXCAVATIONS SHALL BE BACKFILLED WITH GRANULAR 'B' COMPACTED TO 98% SPMDD AND BE PLACED IN ACCORDANCE WITH THE LATEST REVISION OF THE GEOTECHNICAL REPORT.

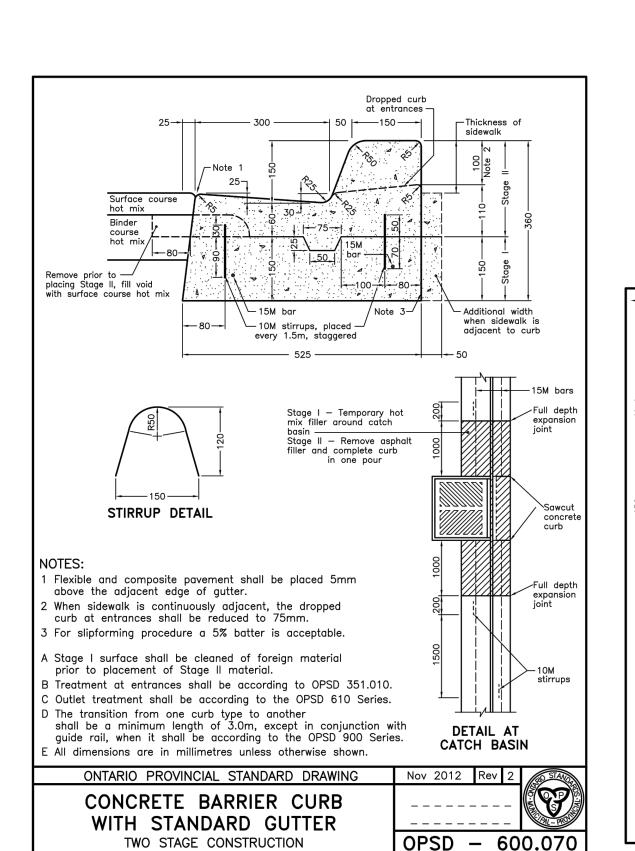
STORM AND SANITARY SEWERS:

- 1. SANITARY SEWERS SHALL BE GREEN PVC SDR 35 (UNLESS OTHERWISE NOTED) INSTALLED IN ACCORD. W/ OPSD 802.01 (NOV. 2010, REV. 2) SANITARY SERVICES SHALL BE GREEN 150mm DIA. PVC SDR 28 INSTALLED IN ACCORD. W/ OPS 1006.020 (NOV. 2005, REV. 1). MIN. 2% SLOPE INSTALLED 2.0m PAST CURB LINE. BEDDING AND COVER MATERIAL FOR SANITARY SEWERS SERVICE CONNECTIONS SHALL BE GRANULAR 'A' COMPACTED TO 98% STANDARD PROCTOR DENSITY.
- 2. ALL SANITARY SEWERS TO BE INSTALLED AS PER COUNTY OF NORFOLK DESIGN GUIDELINES AND SPECIFICATIONS
- 3. STORM SEWERS SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED)
- 300mm DIA. PVC SDR-35 525mm DIA. AND GREATER - 65-D CONCRETE
- 4. CONCRETE STORM SEWERS SHALL BE INSTALLED IN ACCORDANCE WITH OPSD 802.03 (2010) CLASS B. BEDDING MATERIAL SHALL BE GRANULAR 'A' COMPACTED TO 98% S.P.D. COVER MATERIAL SHALL BE SAND COMPACTED TO
- 5. PVC STORM SEWERS SHALL BE INSTALLED IN ACCORDANCE WITH OPSD 802.01 (2010). BEDDING AND COVER MATERIAL SHALL BE GRANULAR 'A' COMPACTED 98% S.P.D.
- 6. SUB-GRADE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF GRANULAR 'B' TO CONFIRM GRANULAR 'B' THICKNESS REQUIRED.

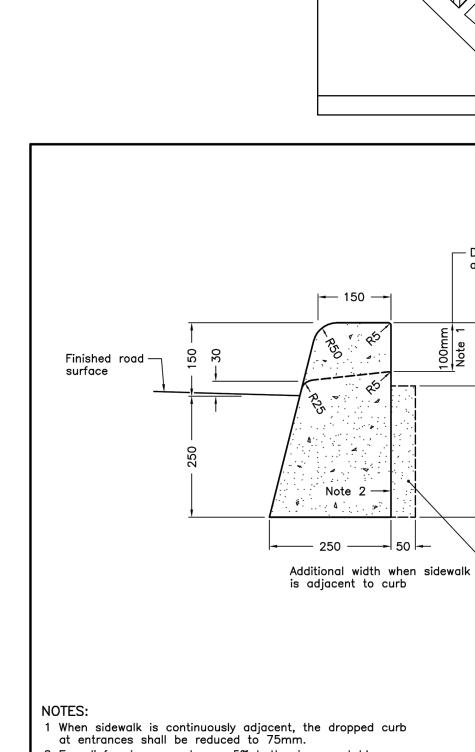
WATERMAINS:

- 1. ALL WATERMAINS AND APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT NORFOLK COUNTY DRAWINGS, M.O.E. GUIDELINES, OPSD STANDARDS AND SPECIFICATIONS AND AWWASTANDARDS AND
- WATERMAIN SHALL BE BLUE AWWA C-9-- PVC DR 18 CLASS 150 UNLESS OTHERWISE NOTED) INSTALLED IN ACCORDANCE WITH OPSD 1103.010 (NOV. 2006, REV. 1) AND 1103.020 (NOV. 2006, REV. 2). WATERMAIN SHALL HAVE TWELVE GAUGE TRACER WIRE FASTENED AT NO GREATER THAN 6.0m INTERVALS AND BROUGHT UP TO FINISHED GRADE AT WATER VALVE BOXES
- 3. WATER SERVICES TO BE 20mm DIA. TYPE 'K' COPPER PIPE, POLYETHYLENE OR MUNICIPEX (OR APPROVED EQUAL), IN ACCORDANCE WITH OPSD 1104.010 (NOV 2006, REV. 2) INSTALLED TO 3.0m PAST CURB LINE.
- 4. ALL WATERMAIN AND SANITARY SEWERS TO BE INSTALLED AS PER COUNTY OF NORFOLK DESIGN GUIDELINES AND SPECIFICATIONS.
- 5. BEDDING AND COVER MATERIAL FOR WATERMAIN AND SERVICE CONNECTIONS SHALL BE GRANULAR "A" COMPACTED TO 100% S.P.D. AS PER OPSD 802.010
- 6. WHERE WATERMAINS OR WATER SERVICES CONFLICT WITH SEWERS, THEY SHALL BE DEFLECTED BELOW THE SEWER TO PROVIDE A MINIMUM SEPARATION DISTANCE OF 0.50m VERTICALLY AND INSULATED.
- 7. VALVES SHALL BE RESILIENT SEAT EPOXY COATED GATE VALVES COMPLYING TO LATEST AWWA CSOO SPECIFICATIONS, VALVE BOXES AS SUPPLIED BY 'MUELER LIMITED' OR 'CANADA VALVE' INSTALLED IN ACCORDANCE WITH OPSD 1101.030 AND OPSD 1100.020.
- 8. HYDRANTS TO COMPLY TO AWWA C502. 'CANADA VALVE' (DARLING), 'CENTURY', 'MACIVITY M67' ARE ACCEPTED. MUST HAVE BRASS TO BRASS SEAT, OPEN COUNTER CLOCKWISE AND BE COATED WITH HIGH QUALITY YELLOW PAINT (OPSD 1105.02) (NOV.2006, REV. 1). ALL EQUIPPED WITH 100mm STORZ CONNECTION (CAP PAINTED BLACK).
- 9. SUB-GRADE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF GRANULAR 'B' TO CONFIRM GRANULAR 'B' THICKNESS REQUIRED.
- 10. MINIMUM 1.7m COVER FOR WATERMAIN AND WATER SERVICES.
- 11. THE OWNER SHALL GRANT THE NECESSARY EASEMENT(S) TO NORFOLK COUNTY AND PORT DOVER ONTARIO TO ENABLE MAINTENANCE OF THE WATER SERVICES ON SITE AND TO INCLUDE A REQUIREMENT FOR A FIRE HYDRANT MAINTENANCE AGREEMENT FOR THE SITE.
- 12. THE FIRE FLOW DEMAND FOR THE SITE AND WATERMAIN DESIGN SHALL BE CALCULATED IN ACCORDANCE WITH THE SECTION 10.1.1 OF NORFOLK COUNTY DESIGN CRITERIA DATED AUGUST, 2017.





TWO STAGE CONSTRUCTION



2 For slipforming procedure a 5% batter is acceptable. A Treatment at entrances shall be according to OPSD 351.010. B Outlet treatment shall be according to the OPSD 610 Series. C The transition from one curb type to another shall be a minimum length of 3.0m, except in conjunction with guide rail where it shall be according to the OPSD 900 Series. D All dimensions are in millimetres unless otherwise shown. ONTARIO PROVINCIAL STANDARD DRAWING CONCRETE BARRIER CURB OPSD 600.110

Note 3 -

TYPE II

4 Cold applied rubber asphalt joint sealing compound.

A Maximum height of slope above top of wall is 4m.

B Concrete for toe walls shall be 30MPa.

6 150mm dia perforated pipe subdrain wrapped in geotextile.

C All dimensions are in millimetres unless otherwise shown

ONTARIO PROVINCIAL STANDARD DRAWING

WALLS

RETAINING

CONCRETE TOE WALL

limit states of 200kPa for Type I and 300kPa for Type II and Type III.

5 Where specified, wall drains shall be installed as per OPSD 3190.100.

Varies

Note 1, Typ

TYPE I

└─ Note 3

JOINT DETAIL

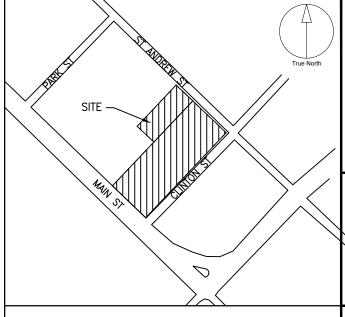
IN CONCRETE TOE WALLS

AT 3.0m SPACING

10mm x 45° → -

Note 6

as specified.



Dropped curb

at entrances

 Thickness of sidewalk

1000033566 ONTARIO INC.

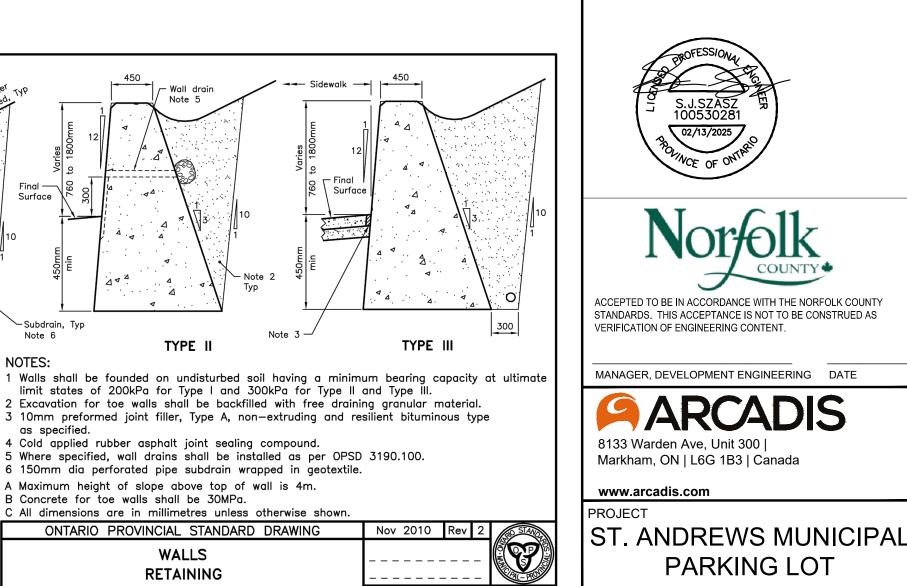
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ARCADIS PROFESSIONAL SERVICES (CANADA) INC. is a member of ARCADIS Group of companies

SUES				
٧o.	DESCRIPTION	DATE		
1.	ISSUED FOR SPA SUBMISSION	FEB. 13, 2025		

<u>LEGEND</u>



PARKING LOT 25 ST. ANDREW ST., PORT DOVER NORFOLK COUNTY PROJECT NO: 148728 DRAWN BY: CHECKED BY:

PROJECT MGR: SHEET TITLE

GENERAL NOTES

AND DETAILS

APPROVED BY:

DD-01

IST OF DRAWINGS SG-01 SITE GRADING PLAN SS-01 SERVICING PLAN C-01 EROSION CONTROL PLAN DD-01 NOTES AND DETAILS DAP-01 PRE-DEVELOPMENT DRAINAGE AREA PLAN DAP-02 POST-DEVELOPMENT DRAINAGE AREA PLAN XS-01 CROSS SECTION

SITE PLAN INFORMATION SURVEY INFORMATION VAN GROLL & ASSOCIATES INC. ASCH & HYDE LTD. ST CLAIRE AVE. W. F18 ITARIO LAND SURVEYORS TORONTO, ONTARIO M4V 1L5 Box 6, 1333 Highway #3 East, Unit B HONE: (905) 339-2811 EXT 228 DUNNVILLE, ONT, N1A 2X1 UNNVILLE: 905-774-7188 RT ERIE: 905-871-975

TYPE III

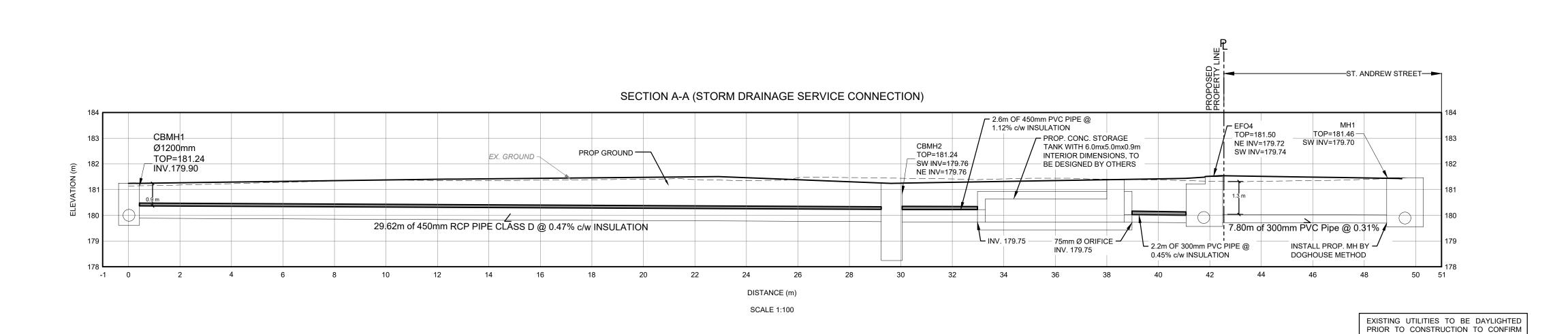
OPSD 3120.100

SITE BENCHMARK IS TOP NUT OF FIRE HYDRANT LOCATED AT NORTHWESTERLY CORNER OF ST. ANDREWS STREET AND CLINTON STREET, AND HAVING AN LEVATION OF 182,32m). SCALE: NOT TO SCALE

SHEET NUMBER

Appendix E

Proposed Cross-Section



KEY PLAN

1000033566 ONTARIO INC.

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ARCADIS PROFESSIONAL SERVICES (CANADA) INC.

is a member of ARCADIS Group of companies					
SUES					
No.	DESCRIPTION	DATE			
1.	ISSUED FOR SPA SUBMISSION	FEB. 13, 2025			

LEGE	<u>ND</u>		

PROPERTY LINE	
-KOPEKTT LINE	
PROPOSED SANITARY MANHOLE	
PROPOSED STORM MANHOLE	\circ
PROPOSED AREA DRAIN	⊠ AD
PROPOSED CATCH BASIN	□СВ
EXISTING CATCH BASIN	
PROPOSED VALVE AND BOX	⋈ V&B
EXISTING FIRE HYDRANT	◆ EX.FH

PROPOSED CATCH BASIN	□CE
EXISTING CATCH BASIN	
PROPOSED VALVE AND BOX	⋈ V&B
EXISTING FIRE HYDRANT	- ∳ - EX.F
PROPOSED SIAMESE CONNECTION	← *
PROPOSED STORM	
PROPOSED SANITARY	
PROPOSED WATER	

PROPOSED WATER	
EXISTING COMBINED	
EXISTING WATER	****
EXISTING STORM	
PROPOSED DOMESTIC WATER METER	M
PROPOSED BACKFLOW PREVENTER	В
PROPOSED DOUBLE CHECK DETECTOR ASSEMBLY	D







www.arcadis.com

ST. ANDREWS MUNICIPAL PARKING LOT

CHECKED BY:

APPROVED BY:

25 ST. ANDREW ST., PORT DOVER NORFOLK COUNTY

LIST OF DRAWINGS				
SG-01 SITE GRADING PLAN				
SS-01 SERVICING PLAN EC-01 EROSION CONTROL PLAN				
DD-01 NOTES AND DETAILS				
DAP-01 PRE-DEVELOPMENT DRAINAGE AREA PLAN DAP-02 POST-DEVELOPMENT DRAINAGE AREA PLAN				
XS-01 CROSS SECTION				
SITE PLAN INFORMATION SURVEY INFORMA				

LOCATION AND DEPTH. NOTIFY ENGINEER IF ANY DISCREPANCIES / CONFLICTS ARE IDENTIFIED. A MINIMUM OF 72 HOURS

BEFORE CONSTRUCTION

DAP-01 PRE-DEVELOPMENT DRAINAGE AREA PLAN DAP-02 POST-DEVELOPMENT DRAINAGE AREA PLAN XS-01 CROSS SECTION				
SITE PLAN INFORMATION	SURVEY INFORMATION			
VAN GROLL & ASSOCIATES INC. 2 ST CLAIRE AVE. W. F18 TORONTO, ONTARIO M4V 1L5 PHONE: (905) 339-2811 EXT 228	RASCH & HYDE LTD. ONTARIO LAND SURVEYORS P.O. Box 6, 1333 Highway #3 East, Unit B, DUNNVILLE, ONT, N1A 2X1			

	IQ
<u>N</u>	SHEET TITI

PROJECT NO: 148728

DRAWN BY:

PROJECT MGR:

 =
CROSS-SECTION

BENCHMARK INFORMATION: SITE BENCHMARK IS TOP NUT OF FIRE HYDRANT LOCATED AT NORTHWESTERLY CORNER OF ST. ANDREWS STREET AND CLINTON STREET, AND HAVING AN ELEVATION OF 182.32m). SCALE: 1:100

DUNNVILLE: 905-774-7188 FORT ERIE: 905-871-9757 SHEET NUMBER

ISSUE XS-01

Stormwater Calculations

IDF Curve Data

Return Period	Α	В	С				
Norfolk County							
2	529.711	4.501	0.745				
5	583.017	3.007	0.703				
10	670.324	3.007	0.698				
25	721.533	2.253	0.679				
50	766.038	1.898	0.668				
100	801.041	1.501	0.657				

RUNOFF CO-EFFICIENTS Table Per Section 7.8.04 from the Norfolk Design Criteria 2019

Based on Area

Single Family Residential 0.45

Based on Surface

Pavement 0.9

City of Pickering Chicago 25 mm 4					
hr Design Storm					
Tir	me	Intensity (mm/hr)			
	-	25mm			
0	0:00	0			
10	0:10	2.17			
20	0:20	2.38			
30	0:30	2.66			
40	0:40	3.03			
50	0:50	3.58			
60	1:00	4.47			
70	1:10	6.2			
80	1:20	12.18			
90	1:30	41.67			
100	1:40	15.28			
110	1:50	9.22			
120	2:00	6.88			
130	2:10	5.62			
140	2:20	4.8			
150	2:30	4.21			
160	2:40	3.78			
170	2:50	3.45			
180	3:00	3.18			
190	3:10	2.95			
200	3:20	2.76			
210	3:30	2.62			
220	3:40	2.47			
230	3:50	2.35			
240	4:00	2.23			



External Area Composite Runoff Coefficient

25 St Andrew Street, Port Dover 148728 January 2025

Prepared by: SN & JT

_ _ _ .

EXT01 Previous Application (200 Main Street, Port Dover)

	(ha)		
Total Area:	0.039		
Gravel:	0.019	Coefficient:	0.70
Landscape:	0.019	Coefficient:	0.25
Composite C:	0.48		

EXT01 Exisiting Minus Overlap from A1 (25 St. Andrew Street)

	(ha)		
Total Area:	0.028		
Gravel:	0.014	Coefficient:	0.70
Landscape:	0.014	Coefficient:	0.25
Composite C:	0.48		

	Storm Service Connection Sewer Characteristics						
Length	Pipe Dia	Pipe Dia	Pipe Type	n	Slope	Cap full Q	Vel. V
(m)	(inch)	(mm)			%	(L/s)	(m/s)
7.80	-	300	PVC	0.013	0.31	53.8	0.76

Orifice Plate		
DIA. (mm)	Peak Flow (L/s)	Percent Full (%)
75	Ω	14.9

Volume of Stru	ıctures					
Pipes	Volume (m ³)					
	Storage-to-OGS	0.16				
	CBMH2-to-Storage	0.41				
	CBMH1-to-OGS	4.71				
	DCIB1-to-CBMH1	2.33				
	Total	7.61				
Structures		Volume (m ³)				
	DCIB1 (1.2x0.6)	0.76				
	CBMH1 (1200mm Dia)	1.51				
	CBMH2 (1200 mm Dia)	1.67				
	Total	3.95	Grand Total	11.6 m³		

Appendix G

PCSWMM Parameters

Appendix G

PCSWMM SOIL PARAMETERS

Tex	oil ture ss ⁽¹⁾	Conductivity	Suction Head (mm)	Porosity	Field Capacity	Wilting Point	Initial Deficit
Silty	Clay	0.51	290.07	0.479	0.371	0.251	0.228

⁽¹⁾ Using the Conservative estimate of Silty Clay, as MTI Consultants Inc. Geotechnical Report native soils were predominantly clayey silt and silt overlying silt, clay and clayey silt made up the composition of the glacial till deposits.

Appendix G

PCSWMM PARAMETERS

EXISTING							
1	2	3	4	5	6	7	8
CA ID#	TOTAL AREA (ha)	R.C. (-)	% IMPERV.	MAX ELEV. (m	MIN ELEV. (m)	FLOW LENGTH (m)	SLOPE (%)
A1PRE	0.060	0.45	30.77	181.40	181.20	34.64	0.58
A2PRE	0.040	0.45	30.77	181.40	181.20	27.21	0.74
EXT01	0.028	0.48	34.62	181.97	180.99	18.86	5.20
PROPOSED							
1	2	3	4	5	6	7	8
CA ID#	TOTAL AREA (ha)	R.C. (-)	% IMPERV.	MAX ELEV. (m	MIN ELEV. (m)	FLOW LENGTH (m)	SLOPE (%)
A1POST	0.060	0.90	100.00	181.70	180.79	29.06	1.10
A2POST	0.040	0.90	100.00	181.70	180.96	24.65	1.70
EXT01	0.028	0.48	34.62	181.97	180.99	18.86	5.20

Appendix H

Stormceptor OGS Model Specifications





Imbrium® Systems ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

01/15/2025

Province:	Ontario
City:	Port Dover, Norfolk County
Nearest Rainfall Station:	BRANTFORD MOE
Climate Station Id:	6140954
Years of Rainfall Data:	41
	•

Site Name:

Drainage Area (ha): 0.10
Runoff Coefficient 'c': 0.90

Particle Size Distribution: Fine

Target TSS Removal (%): 80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	3.27
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	134
Estimated Average Annual Sediment Volume (L/yr):	109

Project Name:	25 St. Andrew Street,
Project Number:	148728
Designer Name:	James Taylor
Designer Company:	Arcadis
Designer Email:	james.nodwelltaylor@arcadis.com
Designer Phone:	519-831-1685
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Net Annual Sediment
(TSS) Load Reduction
Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EFO4	97
EFO5	99
EFO6	100
EFO8	100
EFO10	100
EFO12	100

Recommended Stormceptor EFO Model: EFO4

Estimated Net Annual Sediment (TSS) Load Reduction (%): 97

Water Quality Runoff Volume Capture (%):

> 90





THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators and performance has been third-party verified in accordance with the ISO 14034 Environmental Technology Verification (ETV) protocol.

PERFORMANCE

▶ Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

▶ The Canadian ETV PSD shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle	Percent Less	Particle Size	Percent
Size (µm)	Than	Fraction (µm)	rercent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5





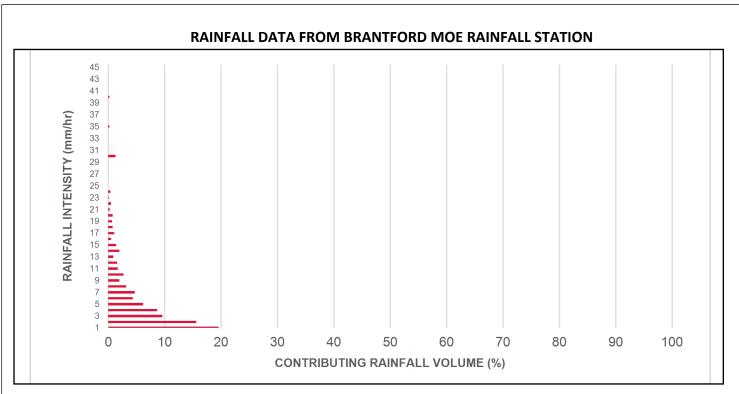
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	9.1	9.1	0.13	8.0	6.0	100	9.1	9.1
1.00	19.6	28.8	0.25	15.0	13.0	100	19.6	28.8
2.00	15.6	44.4	0.50	30.0	25.0	100	15.6	44.4
3.00	9.6	54.0	0.75	45.0	38.0	100	9.6	54.0
4.00	8.7	62.7	1.00	60.0	50.0	100	8.7	62.7
5.00	6.2	68.9	1.25	75.0	63.0	100	6.2	68.9
6.00	4.4	73.3	1.50	90.0	75.0	100	4.4	73.3
7.00	4.7	77.9	1.75	105.0	88.0	98	4.6	77.8
8.00	3.2	81.1	2.00	120.0	100.0	96	3.1	80.9
9.00	2.0	83.1	2.25	135.0	113.0	95	1.9	82.8
10.00	2.7	85.7	2.50	150.0	125.0	93	2.5	85.3
11.00	1.7	87.4	2.75	165.0	138.0	92	1.5	86.8
12.00	1.6	89.0	3.00	180.0	150.0	89	1.4	88.2
13.00	0.9	89.8	3.25	195.0	163.0	88	0.8	88.9
14.00	2.0	91.8	3.50	210.0	175.0	87	1.7	90.7
15.00	1.4	93.2	3.75	225.0	188.0	86	1.2	91.9
16.00	0.5	93.7	4.00	240.0	200.0	83	0.4	92.3
17.00	1.1	94.8	4.25	255.0	213.0	83	0.9	93.1
18.00	0.8	95.5	4.50	270.0	225.0	82	0.6	93.8
19.00	0.7	96.2	4.75	285.0	238.0	82	0.6	94.3
20.00	0.8	97.0	5.00	300.0	250.0	81	0.7	95.0
21.00	0.3	97.4	5.25	315.0	263.0	80	0.3	95.3
22.00	0.5	97.8	5.50	330.0	275.0	80	0.4	95.6
23.00	0.1	97.9	5.75	345.0	288.0	79	0.1	95.7
24.00	0.4	98.3	6.00	360.0	300.0	78	0.3	96.0
25.00	0.0	98.3	6.26	375.0	313.0	78	0.0	96.0
30.00	1.3	99.6	7.51	450.0	375.0	75	1.0	97.0
35.00	0.2	99.8	8.76	525.0	438.0	72	0.1	97.1
40.00	0.2	100.0	10.01	600.0	500.0	69	0.1	97.3
45.00	0.0	100.0	11.26	676.0	563.0	66	0.0	97.3
Estimated Net Annual Sediment (TSS) Load Reduction =								

Climate Station ID: 6140954 Years of Rainfall Data: 41

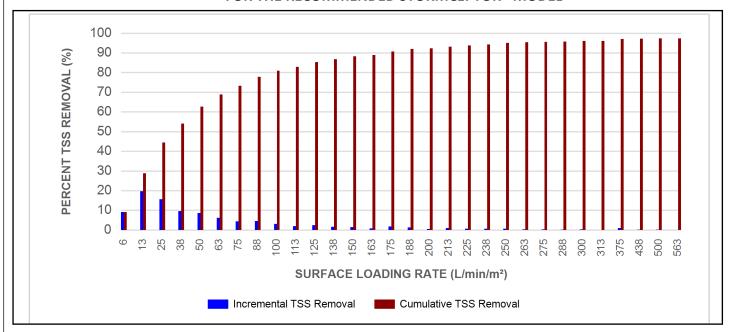








INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL







Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF5 / EFO5	1.5	5	90	762	30	762	30	710	25
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

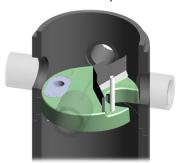
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

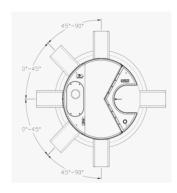
OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid reentrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.









INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45°: The inlet pipe is 1-inch (25mm) higher than the outlet pipe. 45° - 90°: The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maxii Sediment	-	Maxin Sediment	-
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF5 / EFO5	1.5	5	1.62	5.3	420	111	305	10	2124	75	2612	5758
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

^{*}Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef







STANDARD PERFORMANCE SPECIFICATION FOR "OIL GRIT SEPARATOR" (OGS) STORMWATER QUALITY TREAMENT DEVICE

PART 1 - GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

- 1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.
- 1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.
- 1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 - PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1 4 ft (1219 mm) Diameter OGS Units: 1.19 m³ sediment / 265 L oil 5 ft (1524 mm) Diameter OGS Units: 1.95 m³ sediment / 420 L oil 6 ft (1829 mm) Diameter OGS Units: 3.48 m³ sediment / 609 L oil 8 ft (2438 mm) Diameter OGS Units: 8.78 m³ sediment / 1,071 L oil 10 ft (3048 mm) Diameter OGS Units: 17.78 m³ sediment / 1,673 L oil 12 ft (3657 mm) Diameter OGS Units: 31.23 m³ sediment / 2,476 L oil

PART 3 - PERFORMANCE & DESIGN







3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

- 3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.
- 3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.
- 3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².
- 3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.**

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid



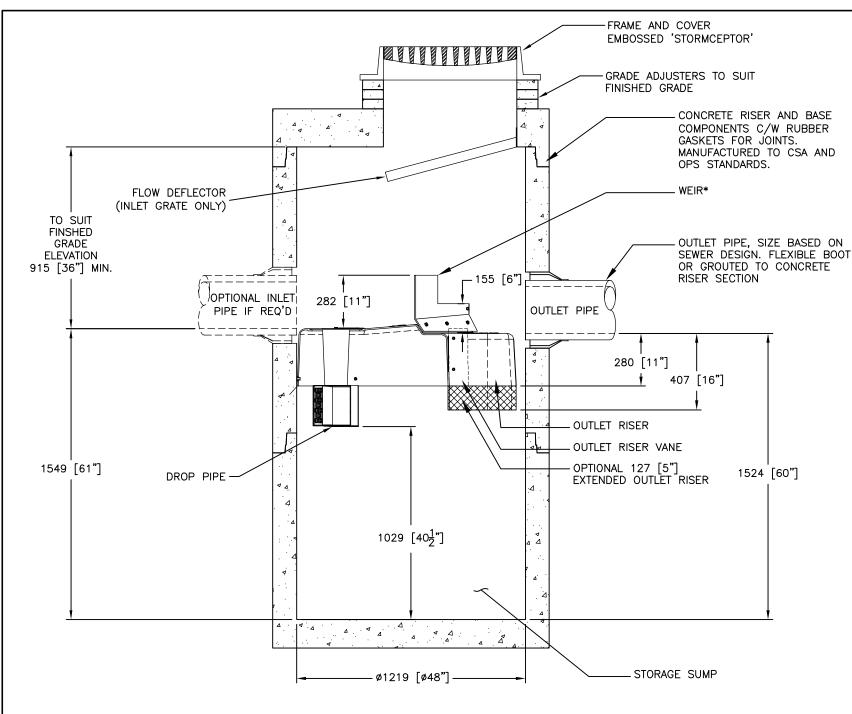




Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators,** with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic
occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance
results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates
(ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing
within the Canadian ETV Program's Procedure for Laboratory Testing of Oil-Grit Separators. However, an
OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with
screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would
not be expected to retain light liquids such as oil and fuel





SECTION VIEW

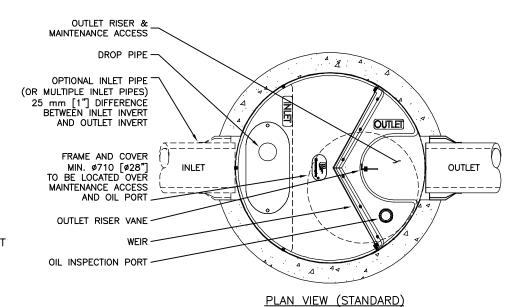
GENERAL NOTES:

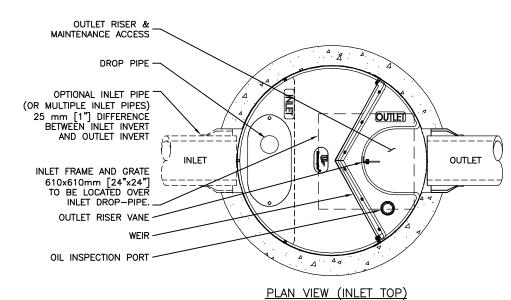
- * MAXIMUM SURFACE LOADING RATE (SLR) INTO LOWER CHAMBER THROUGH DROP PIPE IS 1135 L/min/m² (27.9 gpm/ft²) FOR STORMCEPTOR EF4 AND 535 L/min/m² (13.1 gpm/ft²) FOR STORMCEPTOR EF04 (OIL CAPTURE CONFIGURATION). WEIR HEIGHT IS 150 mm (6 INCH) FOR EF04.
- ALL DIMENSIONS INDICATED ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE SPECIFIED.
- STORMCEPTOR STRUCTURE INLET AND OUTLET PIPE SIZE AND ORIENTATION SHOWN FOR INFORMATIONAL PURPOSES ONLY.
- 3. UNLESS OTHERWISE NOTED, BYPASS INFRASTRUCTURE, SUCH AS ALL UPSTREAM DIVERSION STRUCTURES, CONNECTING STRUCTURES, OR PIPE CONDUITS CONNECTING TO COMPLETE THE STORMCEPTOR SYSTEM SHALL BE PROVIDED AND ADDRESSED SEPARATELY.
- DRAWING FOR INFORMATION PURPOSES ONLY. REFER TO ENGINEER'S SITE/UTILITY PLAN FOR STRUCTURE ORIENTATION.
- NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10
 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF
 RECORD.

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE (LIFTING CLUTCHES PROVIDED)
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT)
- D. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT THE DEVICE FROM CONSTRUCTION-RELATED EROSION RUNOFF.
- E. DEVICE ACTIVATION, BY CONTRACTOR, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE STORMCEPTOR UNIT IS CLEAN AND FREE OF DEBRIS

STANDARD DETAIL NOT FOR CONSTRUCTION





FOR SITE SPECIFIC DRAWINGS PLEASE CONTACT YOUR LOCAL STORMCEPTOR REPRESENTATIVE. SITE SPECIFIC DRAWINGS ARE BASED ON THE BEST AVAILABLE INFORMATION AT THE TIME. SOME FIELD REVISIONS TO THE SYSTEM LOCATION OR CONNECTION PIPING MAY BE NECESSARY BASED ON AVAILABLE SPACE OR SITE CONFIGURATION REVISIONS. ELEVATIONS SHOULD BE MAINTAINED EXCEPT WHERE NOTED ON BYPASS STRUCTURE (IF REQUIRED).

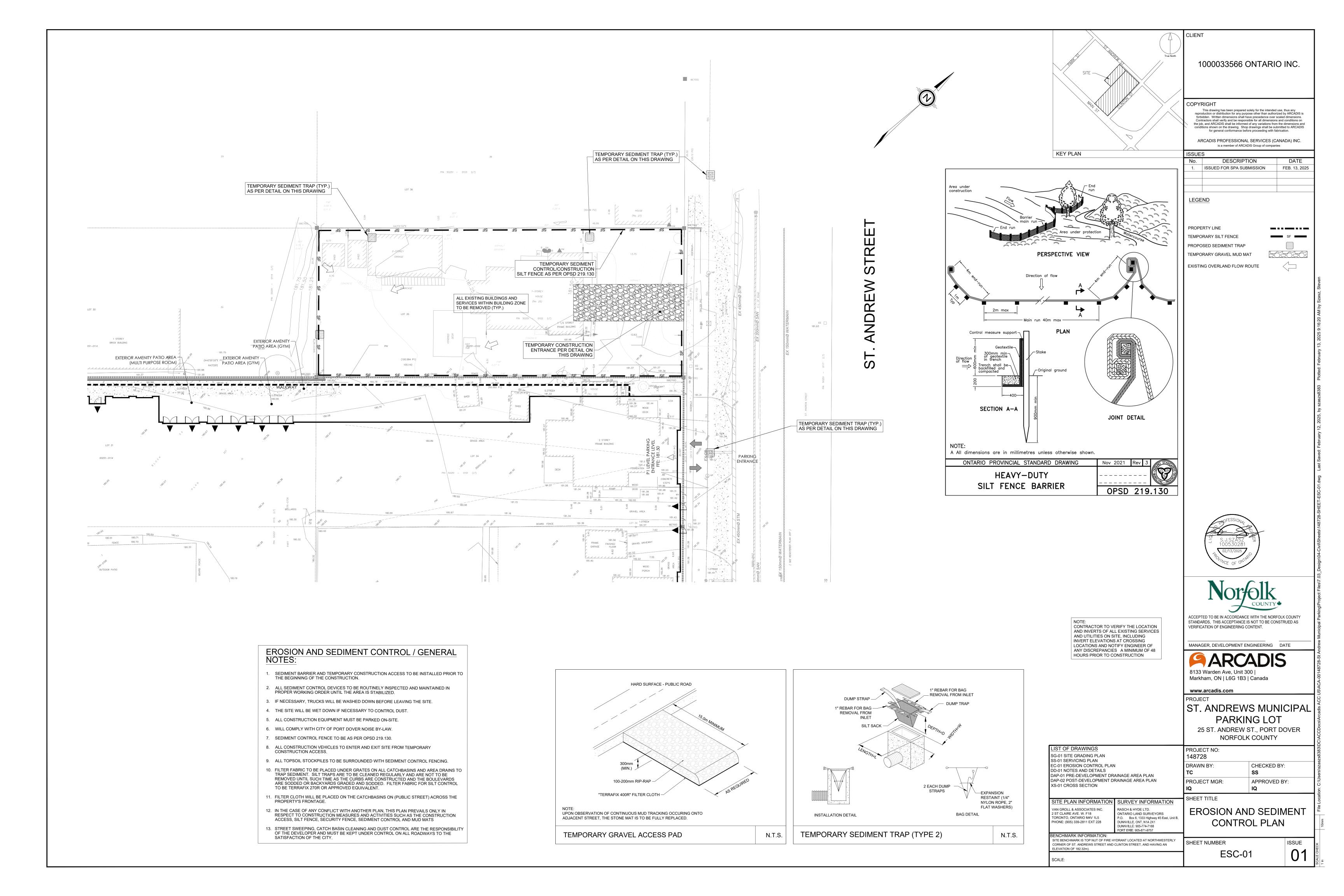
PER ENGINEER OF RECORD

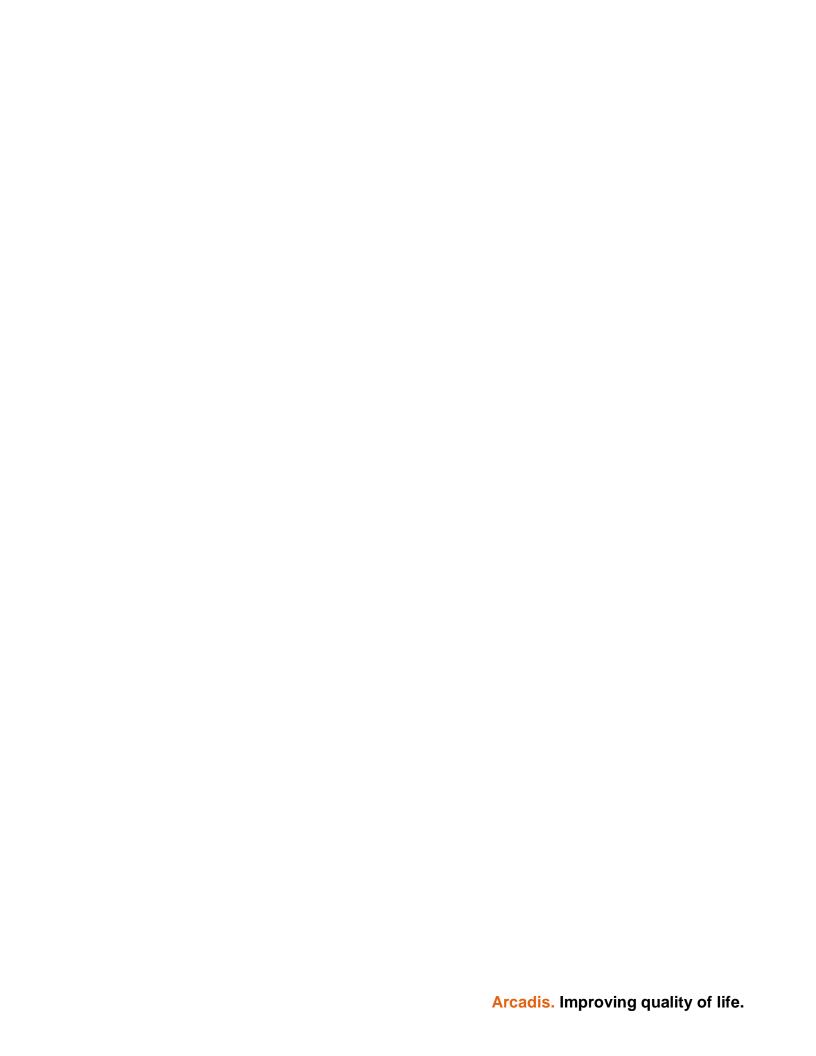
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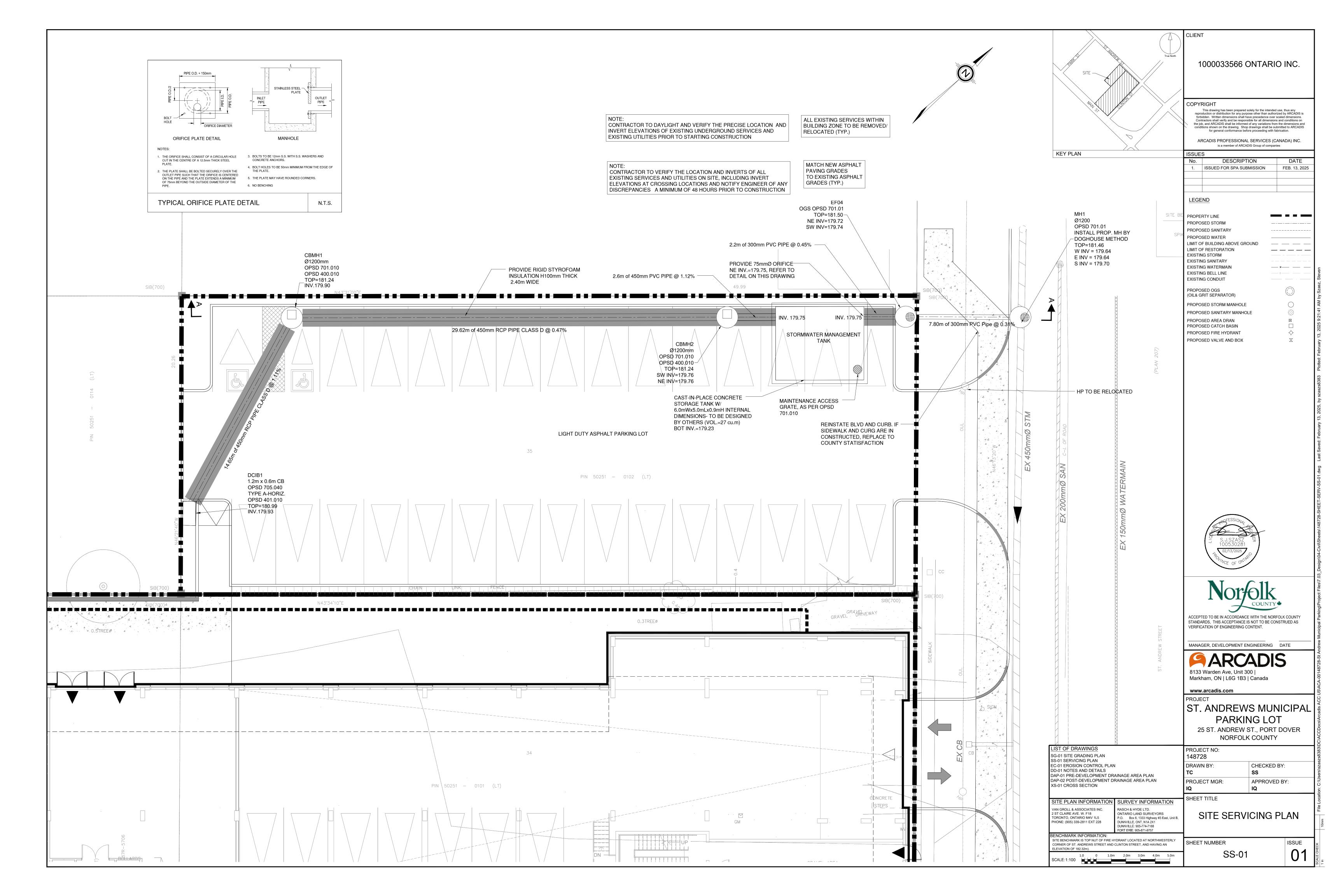
SITE SPECIFIC DAT	A REQUIREMENTS
STORMCEPTOR MODEL	EFO4

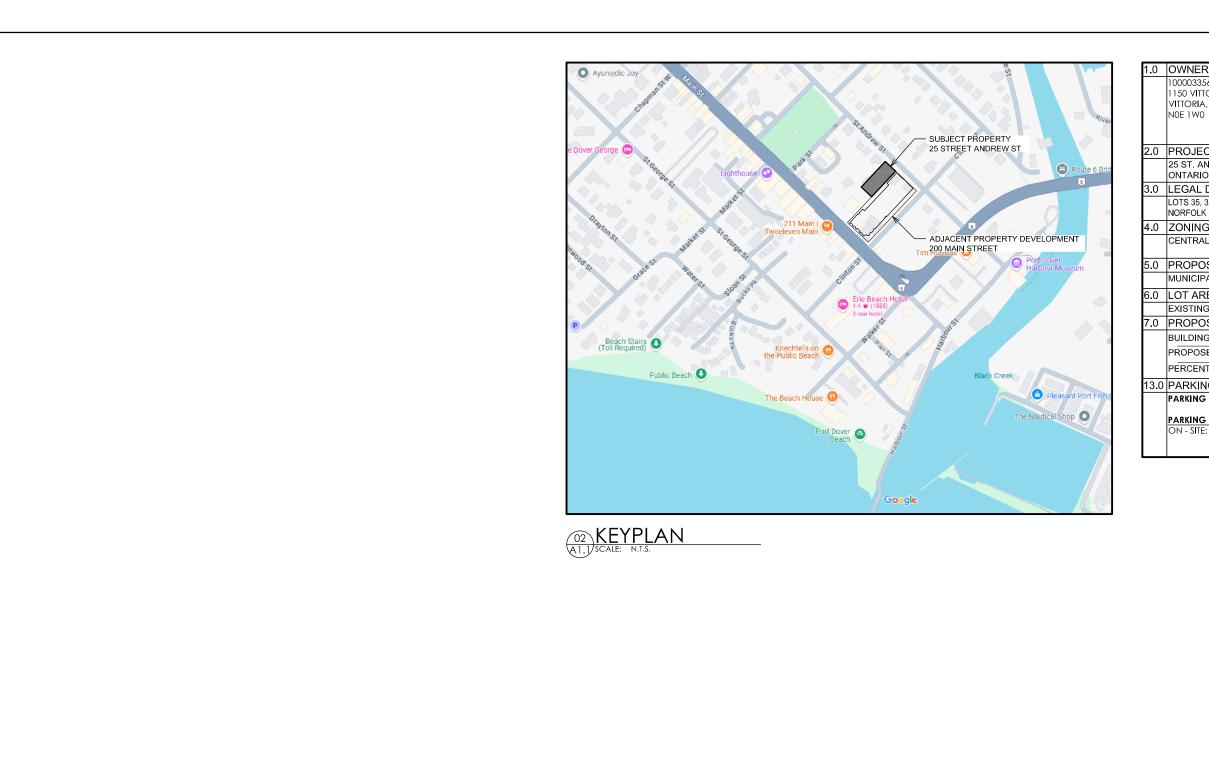
Appendix I

Erosion and Sediment Control Plan

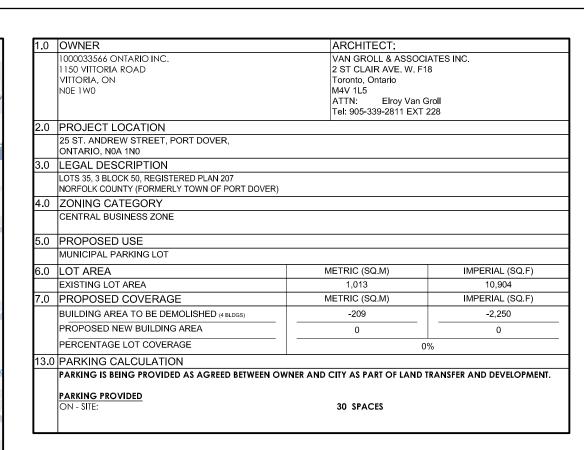








SITE PLAN
SCALE: 1:100



SITEINFORMATION	ZONING	PROPOSED
Lot frontage	N/A	20.30m
Lot depth	N/A	49.99m
Lot width	N/A	20.26m
Lot area	N/A	1,013.22sq.m
Lot coverage	80%	N/A
Front yard	0m	N/A
Rear yard	0m	N/A
Left Interior side yard	0m	N/A
Right Interior side yard	0m	N/A
Exterior side yard (corner lot)	0m	N/A
Landscaped open space	N/A	86.51sq.m
Entrance access width	N/A	7.3m
Exit access width	N/A	7.3m
Size of fencing or screening	N/A	N/A
Type of fencing	N/A	N/A

SITE PLAN NOTES

INSTALL NEW 2.1m WIDE POURED CONCRETE PEDESTRIAN CONNECTION WALKWAY TO NEW PARKING LOT ON ADJACENT SITE, TO MEET NORFOLK COUNTY MUNICIPAL STANDARDS. ENSURE WALKWAY MEETS OBC PATH OF TRAVEL REQUIREMENTS OF 1:20 MAXIMUM SLOPE. SEE ALSO CIVIL DRAWINGS FOR

SLOPE CONCRETE WALKWAY DOWN TO FLUSH CURB AT ASPHALT PARKING FOR BARRIER FREE PATH OF TRAVEL FROM PARKING LOT TO WALKWAY. 1:12 MAXIMUM SLOPE.

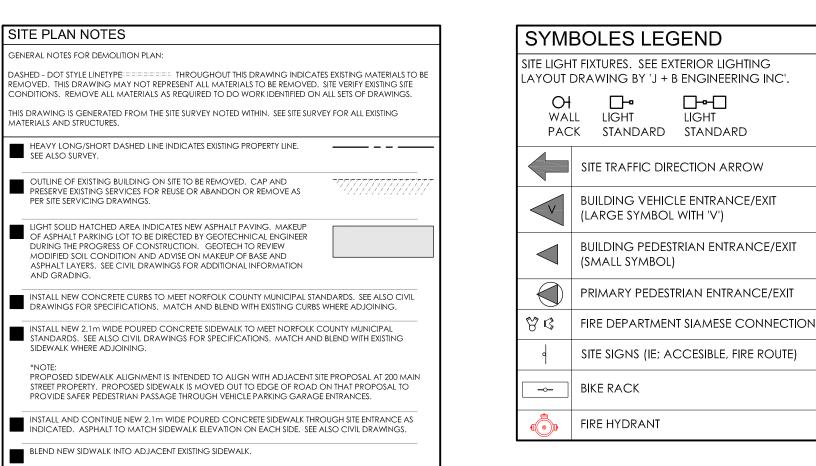
INSTALL PAINTED CROSS HATCH PATTERN AT NEW ACCESSIBLE PARKING SPACE TRANSFER AREA TO MEET REQUIREMENTS OF NORFOLK COUNTY.

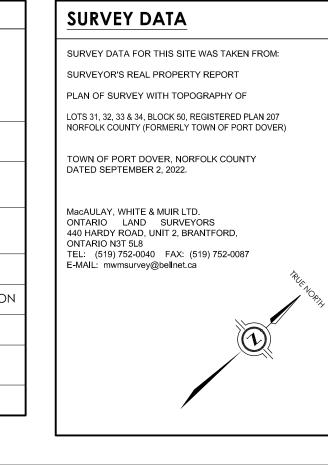
SOLID HATCHED CURBS INDICATE PROPOSE RETAINING. SEE CIVIL DRAWINGS FOR RETAINING DESIGN.

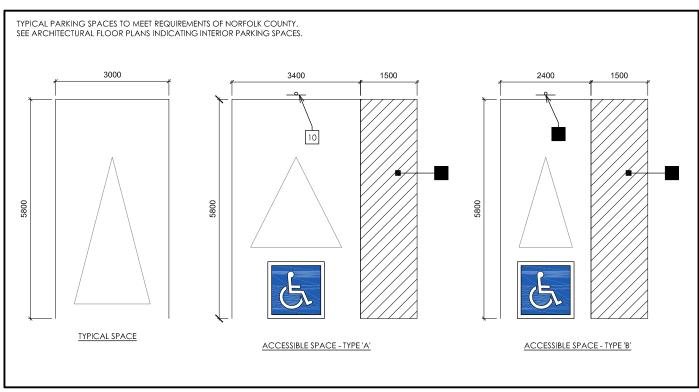
INSTALL NEW ACCESSIBLE PARKING SPACE SIGN AT ALL BARRIER FREE PARKING SPACES TO MEET REQUIREMENTS OF NORFOLK COUNTY.

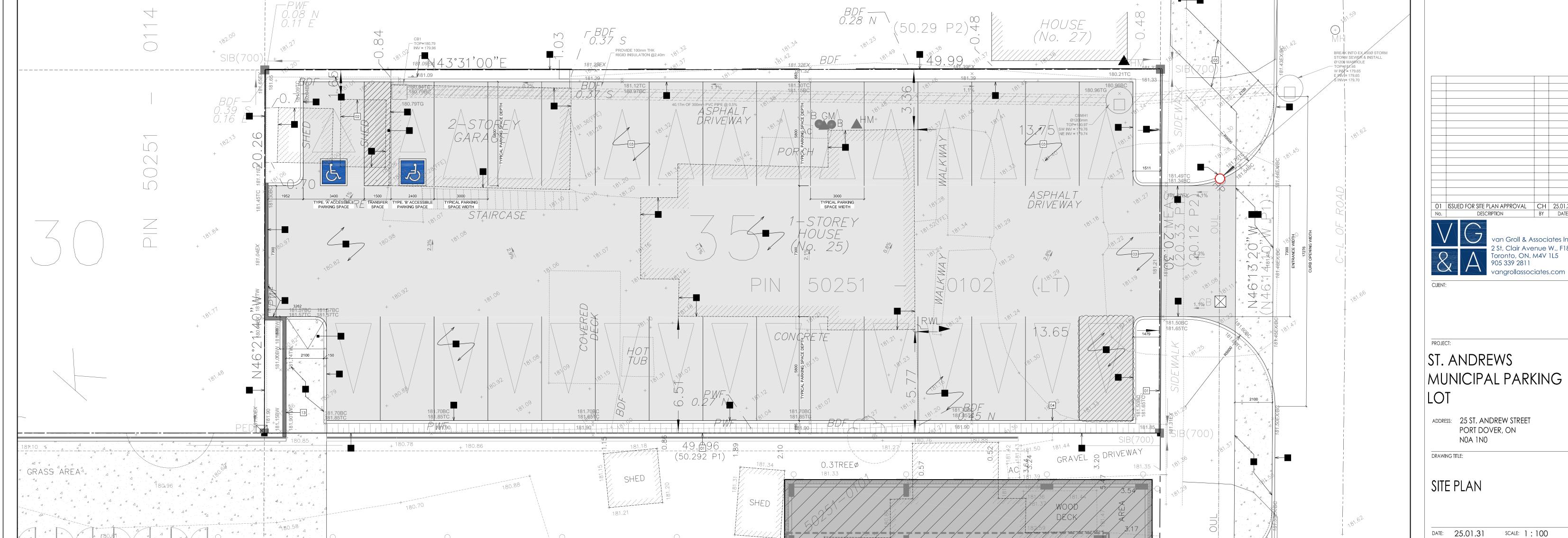
RED DASHED CIRCLE INDICATES EXISTING HYDRO POLE TO BE REMOVED AND REPLACED, EXISTING HYDRO LINES TO BE REVISED, RELOCATED TO NEW POLE LOCATIONS, OR BURIED UNDERGROUND. SEE ELECTRICAL DRAWINGS FOR FULL EXTENT OF REMOVALS AND

BUBBLE HATCHED AREA INDICATES PARKING SPACE TO BE USED AS SNOW STORAGE LOCATION.











DRAWN: CH

171-050

A1.0

DRAWINGS MUST **NOT** BE SCALED.

CONTRACTOR MUST CHECK AND VERIFY

ALL DIMENSIONS, SPECIFICATIONS AND

DISCREPANCIES TO THE ARCHITECT PRIOR

TO PROCEEDING WITH ANY OF THE WORK.

PROJECT NORTH: TRUE NORTH:

DRAWINGS ON SITE AND REPORT ANY

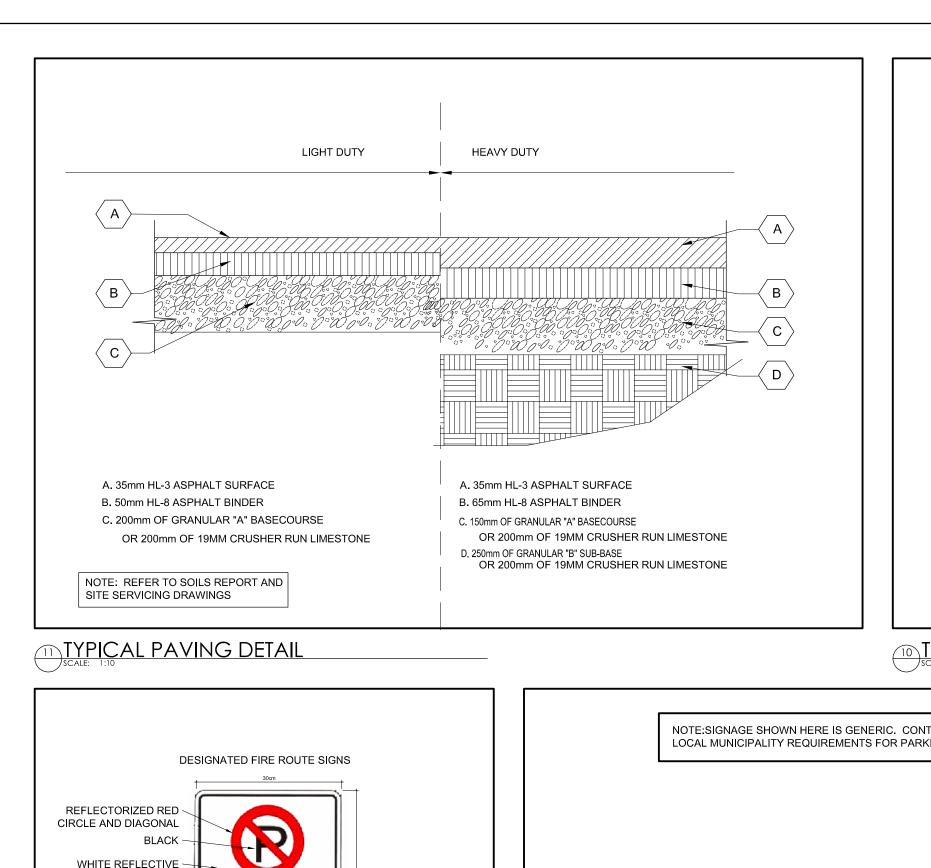
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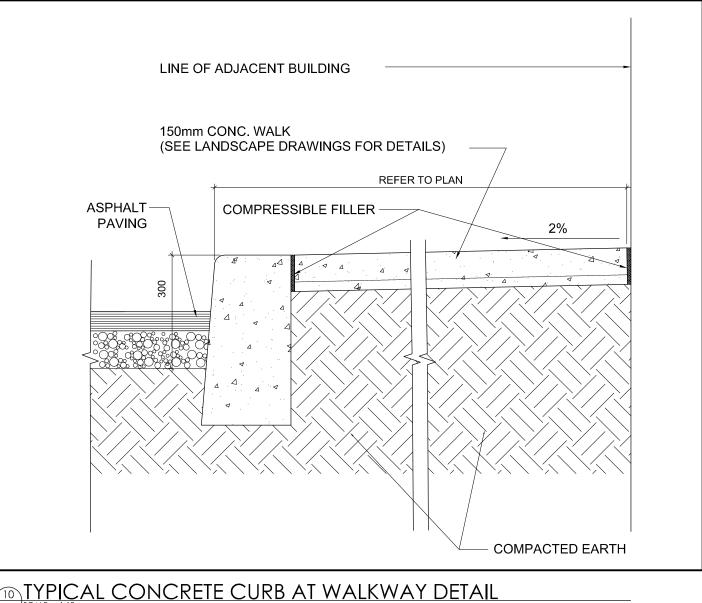
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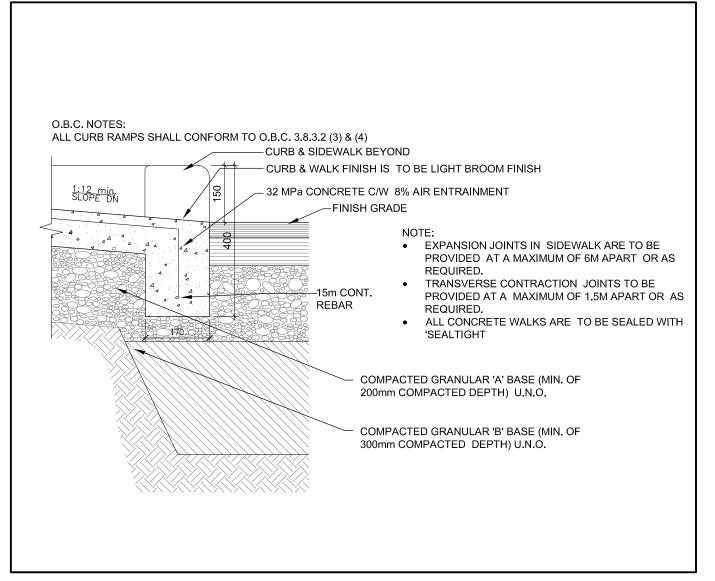
ELROY VAN GROLL

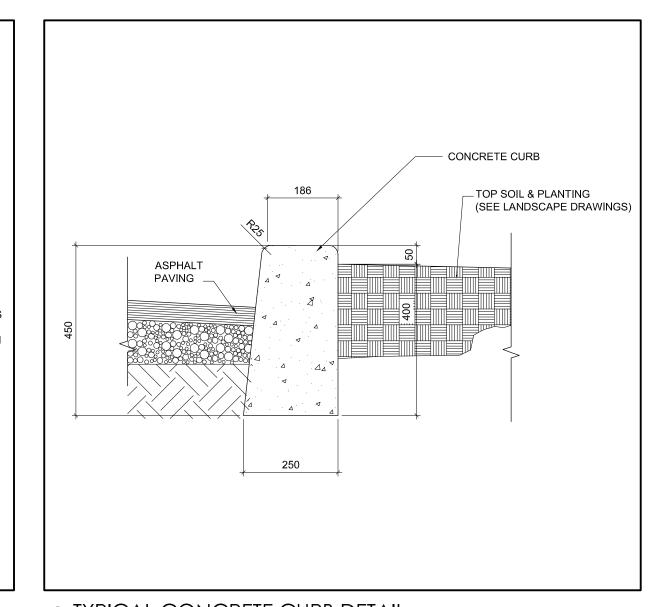
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CONTRACTOR MUST CHECK AND VERIFY ALL DIMENSIONS, SPECIFICATIONS AND DRAWINGS ON SITE AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO PROCEEDING WITH ANY OF THE WORK. PROJECT NORTH: TRUE NORTH: O ARCHITECTS

DRAWINGS MUST **NOT** BE SCALED.

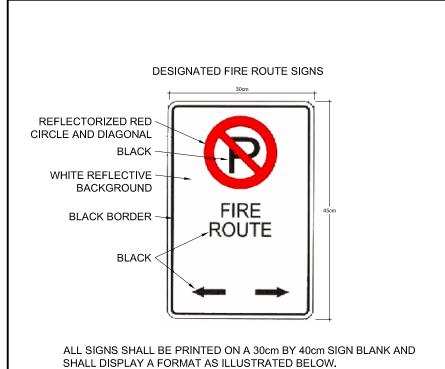
EVO ELROY VAN GROLL LICENCE 4707

TYPICAL CONCRETE CURB AT WALKWAY DETAIL

OP RAMP BASE DETAIL

TYPICAL CONCRETE CURB DETAIL

SCALE: 1:10



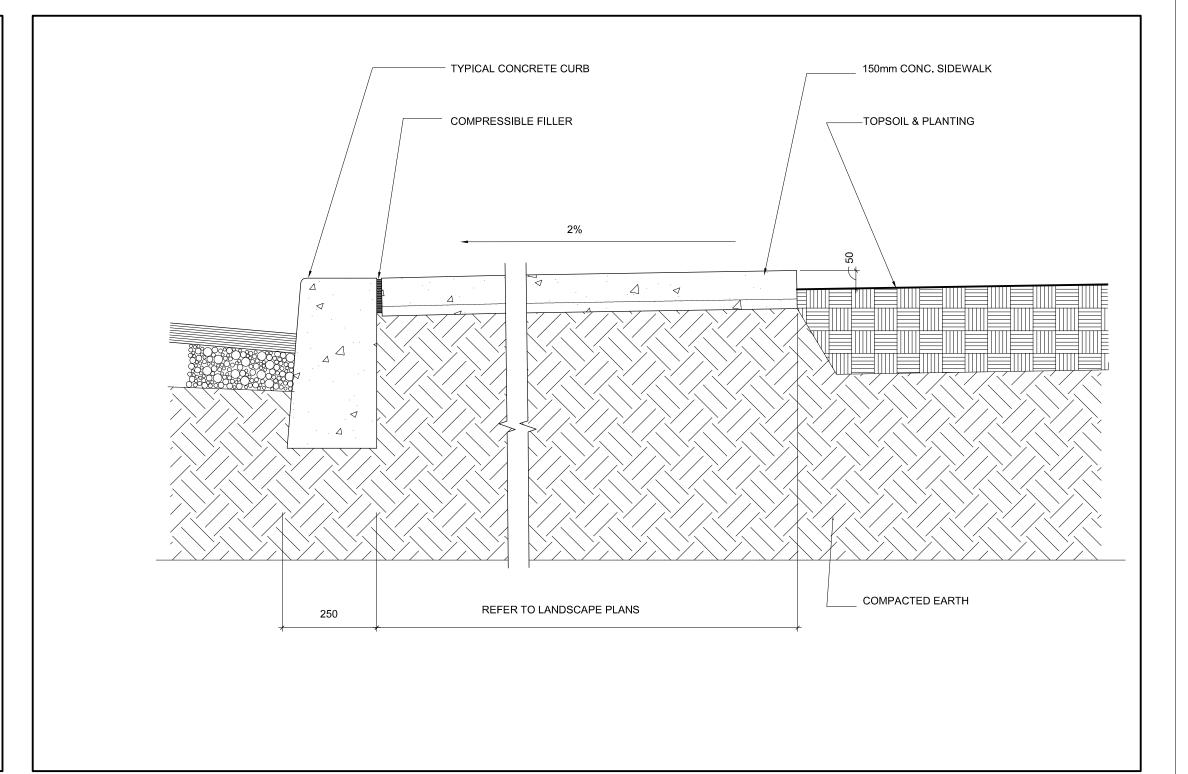
SIGNS SHALL BE PLACED AT EACH LIMIT OF A DESIGNATED FIRE ROUTE AND SHALL DISPLAY SINGLE HEADED ARROWS POINTING IN THE DIRECTION OF SUCH DESIGNATED FIRE ROUTE. SIGNS DISPLAYING DOUBLE HEADED ARROWS SHALL BE PLACED AT INTERMEDIATE POINTS ALONG A DESIGNATED FIRE ROUTE NO FURTHER THAN 30m APART.

SIGNS SHALL BE ERECTED ON A SIGN POST ANCHORED SECURELY TO THE GROUND OR A PLATFORM CAPABLE OF HOLDING THE SIGN ERECT DURING ALL WEATHER CONDITIONS. THE MAXIMUM HEIGHT OF SIGNS SHALL BE NO LESS THAN 2.0m ABOVE THE WEARING SURFACE OF THE DESIGNATED FIRE ROUTE.

SIGNS SHALL BE ERECTED ON THE BOULEVARD OF A DESIGNATED FIRE ROUTE SO THAT NO PART OF THE SIGN IS CLOSER THAN 30cm TO THE ADJACENT CURB NOR FURTHER THAN 3.0m FROM THE CURB. WHERE THERE ARE NO CURBS, NO PART OF THE SIGN SHALL BE CLOSER THAN 1.0m NOR FURTHER THAN 3.0m FROM THE EDGE OF THE ADJACENT PAVEMENT OR TRAVELED ROAD.

NOTE:SIGNAGE SHOWN HERE IS GENERIC. CONTRACTOR TO REFER TO LOCAL MUNICIPALITY REQUIREMENTS FOR PARKING LOT SIGNAGE.

NOTE:SIGNAGE SHOWN HERE IS GENERIC. CONTRACTOR TO REFER TO LOCAL MUNICIPALITY REQUIREMENTS FOR PARKING LOT SIGNAGE. Black Letter "P", Legend and 2 cm Red Reflective Annular Band 1.5 cm Red Reflective Interdictory Stroke Black Letter "P", Blue Reflective Legend and Background and Border White Reflective 2 cm Red Reflective 15 Symbol and Border Annular Band 1.5 cm Red Black Letter "P", Reflective Legend and Border Interdictory Stroke 1.5 White Reflective Blue Reflective Background Background and Van Accessible White Reflective Symbol and Border Maximum Fine BY PERMIT Black Letter "P", \$5000 Legend and Border For Enforcement call: (905) 458-3424 ONLY - White Reflective White Reflective Background 3.5 Background (30x45) cm (30 X 67.5 cm)

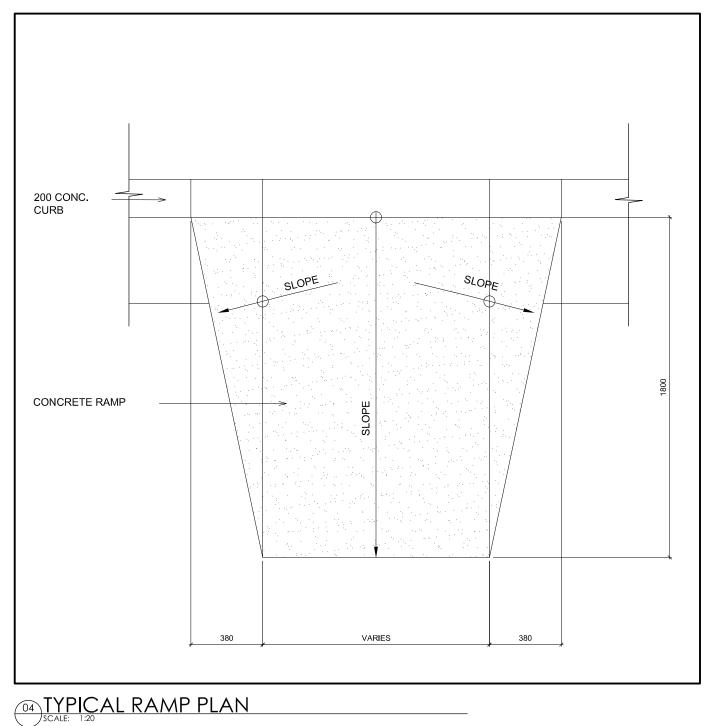


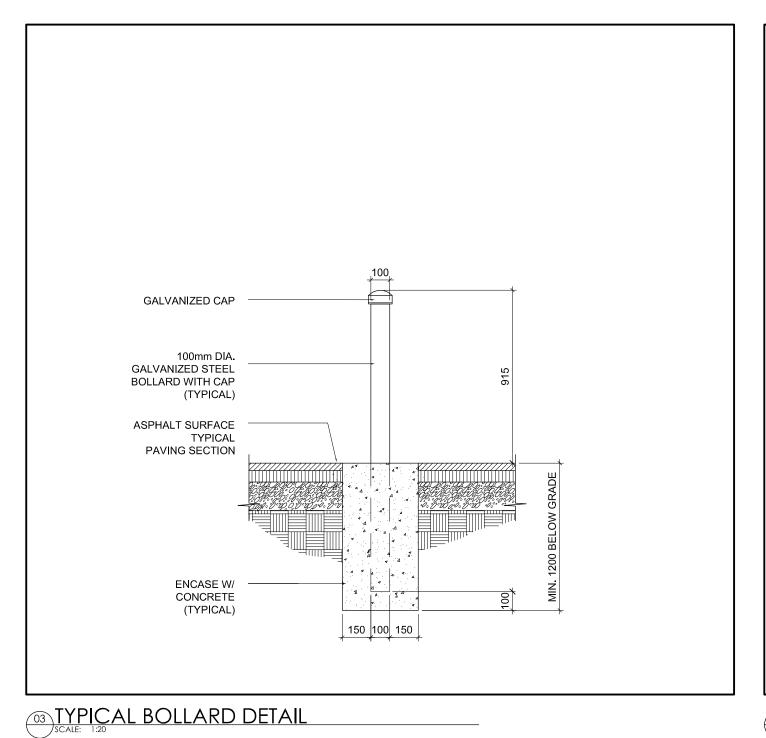
O7 SIGN DETAILS

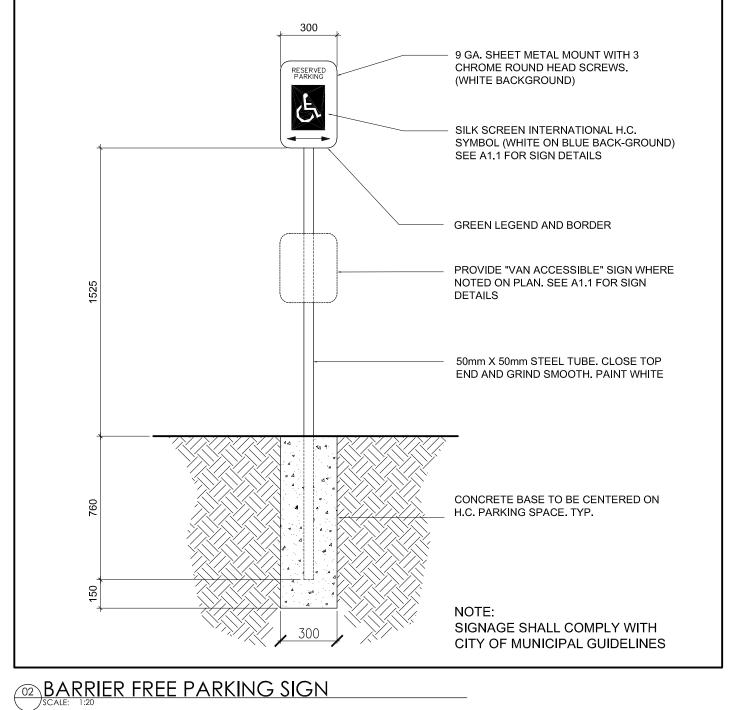
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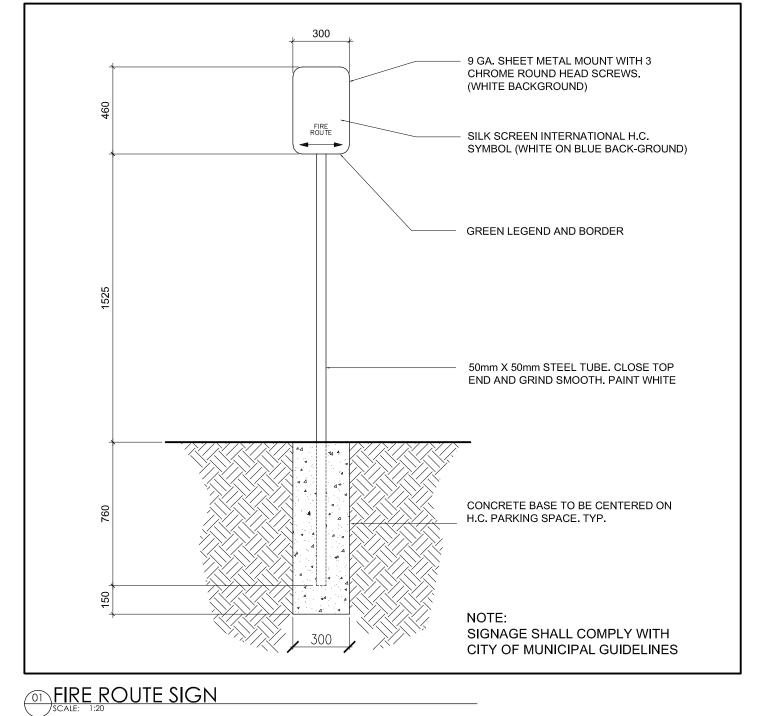
06 BARRER FREE PARKING SIGNAGE

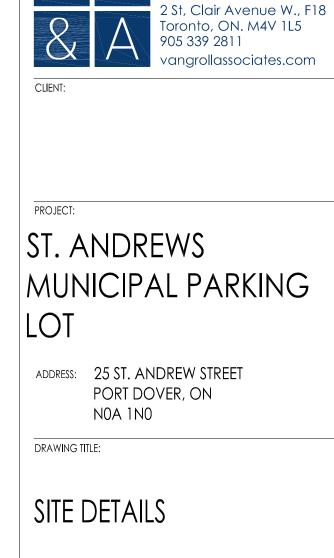
TYPICAL CONCRETE CURB AT WALKWAY AND GUTTER DETAIL











DATE: 25.01.31

drawn: CH

171-050

JOB NUMBER:

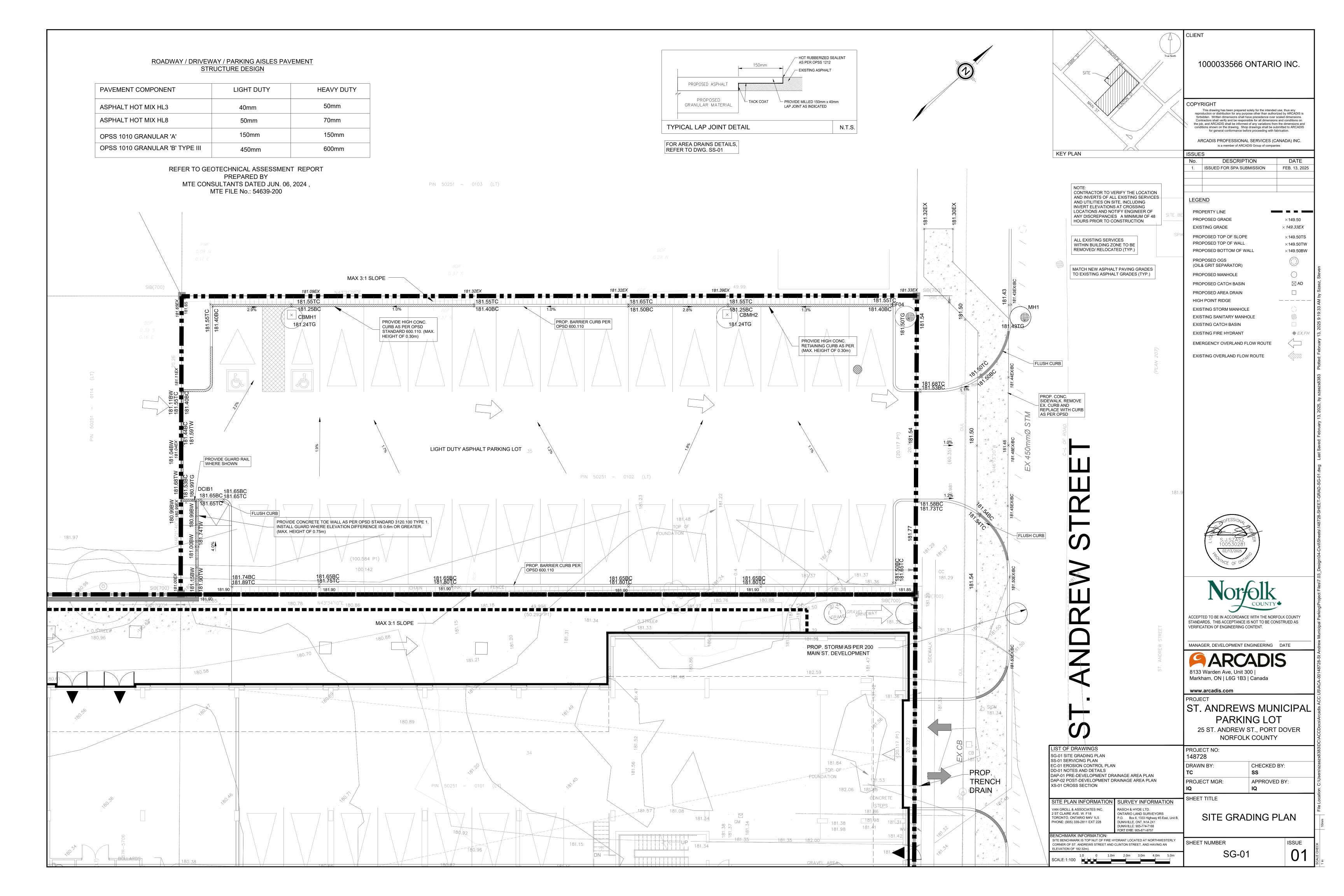
01 ISSUED FOR SITE PLAN APPROVAL CH 25.01.31

an Groll & Associates Inc.

SCALE: AS NOTED

SHEET NUMBER:

DESCRIPTION





Mr. Andrew Wallace Planner Norfolk County 50 Colborne Street South Simcoe, ON N3Y 4H3

Date: February 14, 2025

Our Ref: 148728

Subject: Site Plan Approval - 25 St. Andrew St., Port Dover - 1000033566

Ontario Inc.

Dear Mr. Wallace,

Arcadis Professional Services (Canada) Inc. 360 James Street North Suite 200 Hamilton, Ontario L8L 1H5 Canada

Phone: 905 546 1010 www.arcadis.com

On behalf of our client, 1000033566 Ontario Inc., please accept this letter and the following materials as part of our complete submission of a Site Plan Application ("SPA") for the lands municipally known as 25 St. Andrews St. ("Subject Lands")

The subject lands are legally identified as PDOV PLAN 207 BLK 50 LOT 35. The subject lands are currently occupied by a one-storey residential dwelling. Surrounding land uses include residential to the North, East commercial (Dairy Bar) to the west and a vacant parcel to the South.

The intent of the proposed site plan approval is to redevelop the lands for a surface parking lot which will ultimately owned and operated by the County of Norfolk. The proposed parking lot will replace the existing Clinton Municipal Parking lot located at 33 Clinton Street. The new St. Andrews parking lot proposes twenty-eight (28) standard parking spaces and two (2) accessible parking spaces for a total of thirty (30) parking spaces to replace the 14 spaces within the Clinton Municipal Parking Lot.

The St. Andrews parking lot is planned to provide pedestrian access to Main Street via a proposed pedestrian walkway proposed on the adjacent property to the south which is subject to a separate SPA application process. The relocation of hydro and other utility will be coordinated with the 200 Main Street SPA Application process.

In terms of the current planning framework, the Norfolk County Official Plan ("OP") designates the subject lands as 'Downtown' under Schedule B.17 of the Plan. The proposed municipal parking lot is a permitted land use under the 'Downtown' designation. It is the intent of the new municipal parking lot to improve public parking thereby contributing to the economic viability of the Downtown area. The connection to Main Street via the proposed walkway will provide convenient parking in proximity to the main shopping area. The proposed use is permitted under the OP and no amendment is required.

The subject lands are designated as Central Business District Zone ("CBD") according to Zoning By-Law Norfolk County 1-Z-2014 ("ZBL") which lists 'a parking lot' as one of the permitted uses. As required under the County's by-law, the site plan meets the regulations for Off-Street Parking providing for spaces that are 5.8m x 3.0m with minimum aisle width of 7.3m. Two accessible parking stalls meets the Type A (5.8m x 3.4m) and Type B (5.8m x 2.4m) dimensions with a 1.5m access aisle between the accessible stalls. Therefore, the proposed development does not require an amendment to the ZBL.

Mr. Andrew Wallace Norfolk County February 14, 2025

Please accept the items listed below as part of our complete submission for Site Plan Approval. These have been submitted electronically. If hard copies are required, please advise.

- A Signed Site Plan Application Form
- One (1) copy of the Architectural Package prepared by VanGroll & Associates Inc.
- One (1) copy of the Pre-Development Drainage Area Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Post-Development Drainage Area Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Erosion and Sediment Control Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Grading Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Site Servicing Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the Cross Section Plan prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of the General Notes and Details prepared by Arcadis Professional Services (Canada) Inc.
- One (1) copy of Storm Water Management Design Report prepared by Arcadis Professional Services (Canada) Inc.

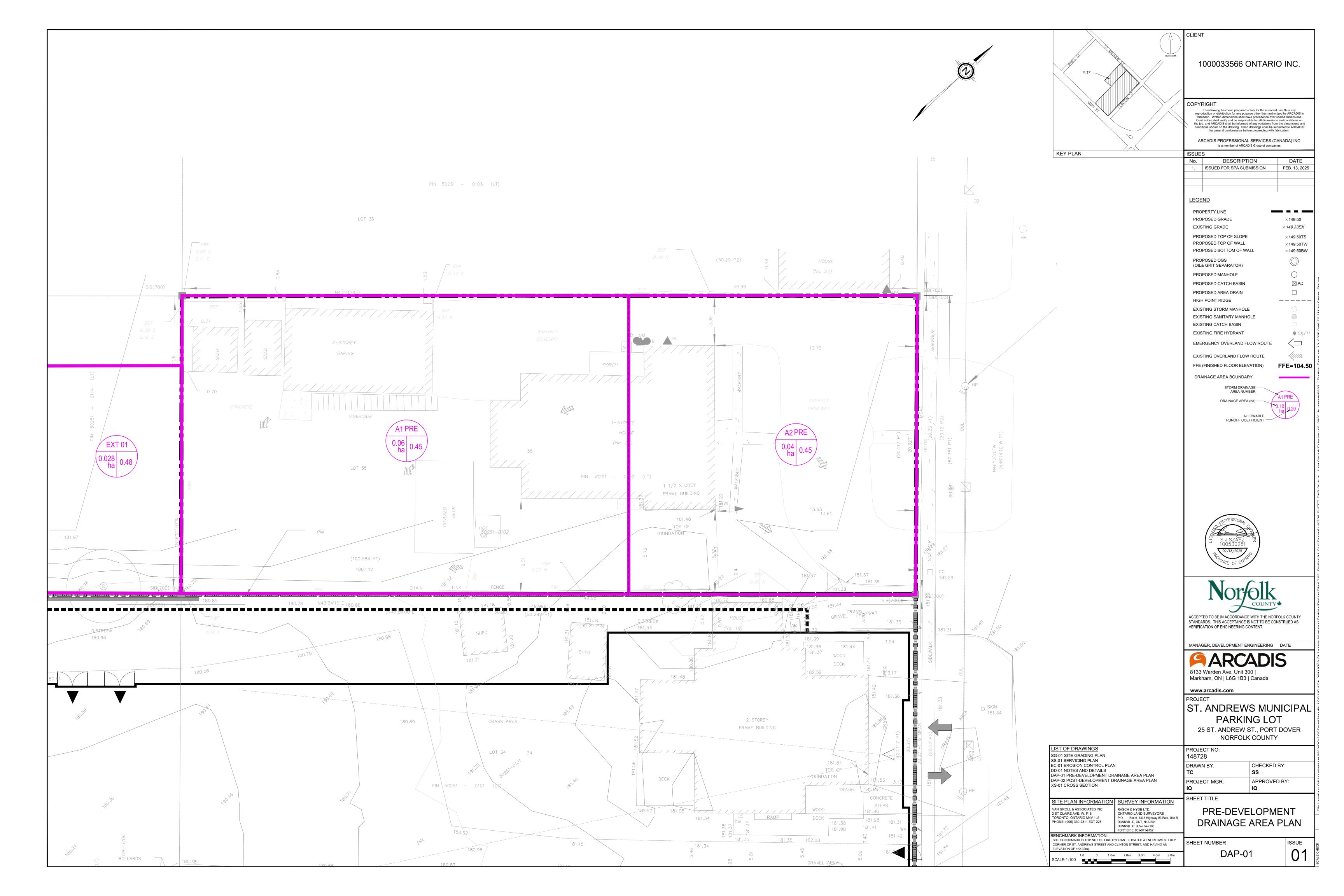
We trust that the attached materials provide you with sufficient information for the processing of this application. Should you have any further questions or require anything more for the processing of this file, please do not hesitate to contact the undersigned.

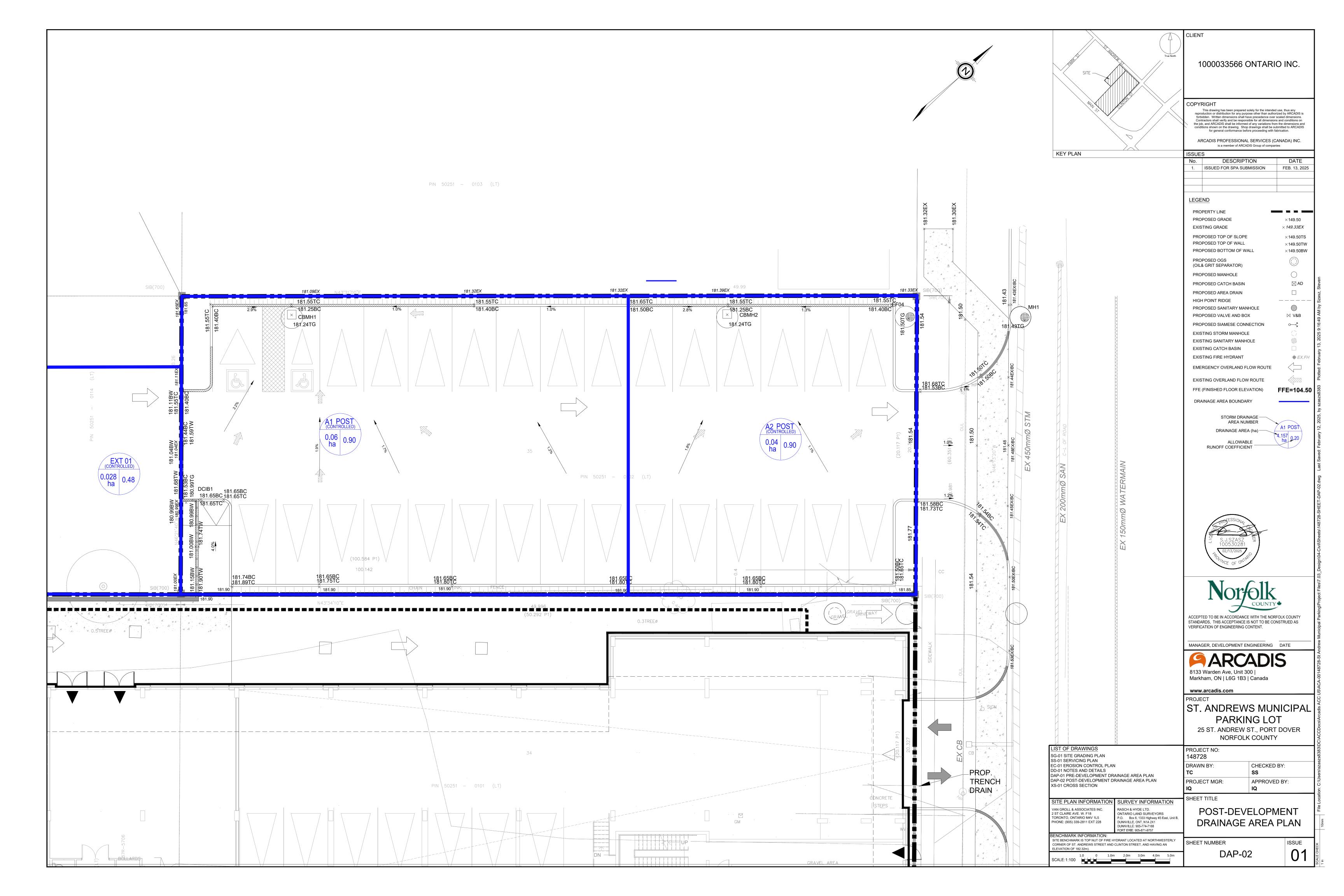
Sincerely,

Arcadis Professional Services (Canada) Inc.

Carmen Jandu, MCIP RPP Planner – Senior Associate Nickee Digman Planning Technician

CC. Ms. Brooke Hayward, via email.





GENERAL NOTES:

- PRIOR TO STARTING ANY WORKS, THE CONTRACTOR MUST ENSURE THAT ALL NECESSARY APPROVALS ARE IN PLACE FROM THE CORPORATION OF NORFOLK COUNTY AND OTHER EXTERNAL AGENCIES, AS REQUIRED.
- 2. ALL WORK SHALL BE CARRIED OUT IN COMPLIANCE WITH THE APPLICABLE HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
- 3. ALL WORK AND MATERIALS TO CONFORM WITH THE CURRENT PROVINCIAL BUILDING CODE, MINISTRY OF THE ENVIRONMENT OF ONTARIO, NORFOLK COUNTY, AND ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS. LOCAL UTILITY STANDARDS AND MINISTRY OF TRANSPORTATION STANDARDS WILL APPLY WHERE REQUIRED.
- 4. FOR ALL CONSTRUCTION DETAILS NOT SHOWN ON THE DRAWINGS, REFERENCE SHALL BE MADE TO THE DESIGN STANDARDS OF THE CORPORATION OF NORFOLK COUNTY.
- 5. THE CONTRACTOR IS ADVISED THAT WORKS BY OTHERS MAY BE ONGOING DURING THE PERIOD OF THIS CONTRACT. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH ALL OTHER CONTRACTORS AND PREVENT CONSTRUCTION CONFLICTS.
- 6. THE INFORMATION SHOWN FOR EXISTING UTILITIES WAS PROVIDED BY OTHERS. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES DURING CONSTRUCTION. ALL EXISTING UTILITIES MUST BE LOCATED AND VERIFIED BY EACH PROVIDER PRIOR TO COMMENCEMENT OF WORK. ANY VARIANCE IS TO BE REPORTED TO THE ENGINEER 48 HRS PRIOR TO CONSTRUCTION, LOST TIME AND/OR ANY ADDITIONAL WORKS DUE TO FAILURE OF THE CONTRACTOR TO CONFIRM UTILITY LOCATIONS AND NOTIFY THE ENGINEER OF ANY CONFLICTS 48 HRS PRIOR TO CONSTRUCTION WILL BE AT THE CONTRACTORS EXPENSE.
- THE CONTRACTOR MUST INSTALL ALL SEDIMENT CONTROL DEVICES PRIOR TO THE COMMENCEMENT OF SITE GRADING WORKS, SILT LADEN WATER MUST NOT BE PERMITTED TO ENTER INTO ANY EXISTING CATCH BASINS INLETTING STRUCTURES, OR WATERCOURSES, ADDITIONAL CONTROLS AS DEEMED REQUIRED BY THE AUTHORITIES AND/OR THE ENGINEER DURING CONSTRUCTION ACTIVITIES SHALL BE PROVIDED BY THE CONTRACTOR. THE CONTRACTOR MUST INSPECT SEDIMENT CONTROLS ON A REGULAR BASIS AND AFTER EVERY RAINFALL EVENT. REPAIRS MUST BE DONE IN A TIMELY MANNER TO PREVENT SEDIMENT FROM ENTERING ANY WATER SYSTEMS. ADDITIONAL SILT FENCING MUST BE AVAILABLE IN CASE IMMEDIATE REPAIR IS
- 8. ALL DIMENSIONS, ELEVATIONS AND OTHER INFORMATION SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR 72 HOURS PRIOR TO ANY CONSTRUCTION. ANY DISCREPANCIES FOUND MUST BE REPORTED
- 9. THE CONTRACTOR IS TO PROVIDE A TOTAL OF TWO CCTV CAMERA INSPECTIONS OF ALL SANITARY AND STORM SEWERS. INCLUDING PICTORIAL REPORT, TWO CD COPIES AND ONE VIDEO TAPE IN A FORMAT SATISFACTORY TO THE ENGINEER. ALL SEWERS ARE TO BE FLUSHED PRIOR TO CAMERA INSPECTION.
- 10. LASER ALIGNMENT CONTROL TO BE UTILIZED ON ALL SEWER INSTALLATIONS.
- 11. ALL PVC SANITARY SEWERS TO BE MANDREL AND AIR TESTED.
- 12. ALL PVC STORM SEWERS TO BE MANDREL TESTED. AIR TEST ONLY ON RECOMMENDATION BY SOIL

CONSTRUCTION NOTES:

- 1. AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION FOR SERVICES ON ANY EXISTING ROAD ALLOWANCE AND EXISTING SERVICES WITHIN AN EXISTING EASEMENT MAINTAINED BY NORFOLK COUNTY IN PORT DOVER ONTARIO, THE SUBDIVIDER / DEVELOPER IS TO OBTAIN A PERMIT OF APPROVED WORK FROM NORFOLK COUNTY AND PORT DOVER ONTARIO.
- 2. THE CONTRACTOR SHALL CONSTRUCT TEMPORARY MEASURES TO CONTROL SILT ENTERING THE STORM DRAINAGE SYSTEM TO THE SPECIFICATIONS OUTLINED IN THE GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES PREPARED BY THE MINISTRY OF NATURAL RESOURCES. THESE MEASURES ARE TO BE INSTALLED PRIOR TO COMMENCING ANY CONSTRUCTION FOR THIS PROJECT, AND ARE TO REMAIN IN PLACE UNTIL CONSTRUCTION HAS BEEN COMPLETED TO THE SPECIFICATIONS OF THE TOWN
- 3. ALL WORK SHALL MEET THE MINIMUM STANDARDS AND SPECIFICATIONS OF NORFOLK COUNTY AND PORT DOVER ONTARIO OR ONTARIO PROVINCIAL STANDARDS.
- 4. NO FOUNDATION DRAIN CONNECTIONS WILL BE PERMITTED. SUMP PUMPS SHALL BE DISCHARGED TO THE OUTSIDE GROUND SURFACE DRAINING AWAY FROM THE BUILDING. REQUIRED TO CONFORM WITH OBC, DIV.B.9.14.
- 5. THE SUBDIVIDER / DEVELOPER IS TO NOTIFY NORFOLK COUNTY AND PORT DOVER ONTARIO AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION. PERMITS ARE REQUIRED PRIOR TO COMMENCEMENT OF WORK AND 48 HOURS IS REQUIRED PRIOR TO INSPECTION REQUESTS.
- 6. THE SUBDIVIDER / DEVELOPER IS TO MEET ALL REQUIREMENTS OF THE OWNERS OF THE UTILITIES ON THIS PLAN. AND MUST MAKE SATISFACTORY ARRANGEMENTS WITH THE UTILITY COMPANIES FOR CROSSING THEIR INSTALLATIONS AND FOR PROVIDING ADEQUATE PROTECTION DURING CONSTRUCTION.
- 7. IF COMMON TRENCH CONSTRUCTION IS TO BE USED FOR THE INSTALLATION OF STORM AND SANITARY SEWERS ON ANY STREET WITHIN THIS SUBDIVISION / DEVELOPMENT / PROJECT, THE PDC'S ARE TO BE CONSTRUCTED AT LEAST 2 METERS BEHIND THE CURB LINE ON THAT STREET, AT THE SAME TIME AS INSTALLATION OF THE SEWERS.
- 8. ALL ORGANIC, UNSTABLE OR UNSUITABLE MATERIALS BENEATH THE ROAD ALLOWANCES OR HOUSE FOUNDATIONS MUST BE REMOVED AND THESE AREAS BACKFILLED WITH AN APPROVED FILL MATERIAL, ALL TO THE SATISFACTION OF A GEOTHECHNICAL ENGINEER.

GRADING:

- PRIOR TO COMMENCEMENT OF GRADING WORKS ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSTALLED AND OPERATIONAL. THE CONTRACTOR SHALL MAINTAIN ALL WORKS UNTIL SERVICING CONSTRUCTION IS COMPLETED TO THE SATISFACTION OF THE ENGINEER AND THE CORPORATION OF NORFOLK
- 2. ALL GRANULAR BASE AND SUB-BASE COURSE MATERIALS SHALL BE COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.
- 3. PAVEMENT STRUCTURE TO BE CONSTRUCTED AS RECOMMENDED BY THE GEOTECHNICAL REPORT.
- 4. CONCRETE CURBS SHALL BE AS PER OPSD 600.060 AND OPSD 600.110.
- 5. INSPECTIONS: ALL WORK ON THE MUNICIPAL RIGHT-OF-WAY AND EASEMENTS TO BE INSPECTED BY THE MUNICIPALITY PRIOR TO BACKFILLING. ALL WORK RELATING TO WATERMAINS AND SEWERS TO BE INSPECTED BY THE MUNICIPALITY WHEN REQUIRED BY THE MUNICIPALITY.
- 6. CONTRACTOR TO OBTAIN A ROAD OCCUPANCY PERMIT 48 HOURS PRIOR TO COMMENCING ANY WORK WITHIN THE MUNICIPAL ROAD ALLOWANCE IF REQUIRED BY THE MUNICIPALITY OR THE REGION.
- 7. EMBANKMENTS TO BE SLOPED AT MAX. 3:1, UNLESS OTHERWISE SPECIFIED.
- 8. SEDIMENT CONTROL TO BE PROVIDED AT CATCH BASINS AND CATCH BASIN MANHOLES UPON INSTALLATION OF STRUCTURES AS PER DETAIL PROVIDED.
- 9. CONTRACTOR WILL BE RESPONSIBLE FOR ALL REMOVALS AS REQUIRED TO FACILITATE NEW CONSTRUCTION. ALL EXISTING STRUCTURES, VALVES, ETC. ARE TO BE ADJUSTED TO PROPOSED ELEVATIONS.
- 10. EXISTING ELEVATIONS AT MATCH POINTS, AS SHOWN ON PLANS, ARE TO BE CONFIRMED BY THE CONTRACTOR 72 HOURS PRIOR TO MOBILIZATION OF FORCES. LOST TIME AND/OR ANY ADDITIONAL WORKS DUE TO FAILURE OF THE CONTRACTOR TO CONFIRM EXISTING ELEVATIONS AND NOTIFY THE ENGINEER OF POSSIBLE CONFLICTS 72 HOURS PRIOR TO MOBILIZATION WILL BE AT THE EXPENSE OF THE CONTRACTOR.

MANHOLES AND CATCHBASINS:

- 1. ALL PRECAST CONCRETE MANHOLES TO MEET M.O.E. SPECIFICATIONS AND CONFORM TO OPSD STANDARDS 701.010 AND 701.011.
- 2. MANHOLE COVERS TO BE AS PER OPSD 401.010, TYPE 'A' FOR SANITARY AND TYPE 'B' FOR STORM.
- 4. MANHOLE STEPS SHALL BE RECTANGULAR STAINLESS STEEL AS PER OPSD 405.010.

3. MANHOLE AND CATCHBASIN ADJUSTERS SHALL BE AS PER OPSD 704.010.

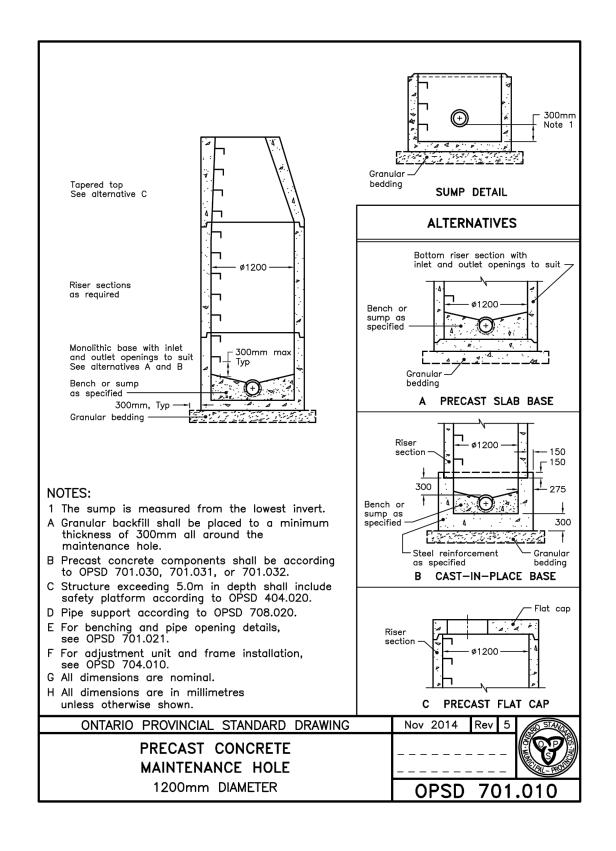
- 5. SAFETY PLATFORMS SHALL BE PROVIDED, AS PER OPSD 404.020, FOR MANHOLES WITH DEPTH EXCEEDING 5.0m.
- 6. BENCHING TO BE PROVIDED AT ALL MANHOLES UNLESS OTHERWISE STATED IN ACCORDANCE WITH OPSD 701.021
- 7. ALL SINGLE AND DOUBLE CATCH BASINS SHALL BE PRECAST AS PER OPSD 705.010 AND 705.020 RESPECTIVELY.
- CATCHBASIN LEADS SHALL BE PVC SDR 35 INSTALLED AT A MINIMUM OF 1.0% SLOPE AND WITH MINIMUM COVER OF 1.2m. SIZES ARE AS FOLLOWS: SINGLE CB- 250mm DIA., DOUBLE CB- 300mm DIA., REAR LOT CB- 300mm DIA.
- 9. CATCHBASIN SUMP TO BE 0.60m DEEP FOR 600x600, AND 0.30m DEEP FOR 1200x1200.
- 10. ALL CATCH BASIN FRAMES AND COVERS SHALL BE AS PER OPSD 400.110.
- 11. DURING CONSTRUCTION ALL CATCH BASINS SHALL BE EQUIPPED WITH A TEMPORARY SEDIMENT CONTROL DEVICE.
- 12. ALL MANHOLE AND CATCH BASIN EXCAVATIONS SHALL BE BACKFILLED WITH GRANULAR 'B' COMPACTED TO 98% SPMDD AND BE PLACED IN ACCORDANCE WITH THE LATEST REVISION OF THE GEOTECHNICAL REPORT.

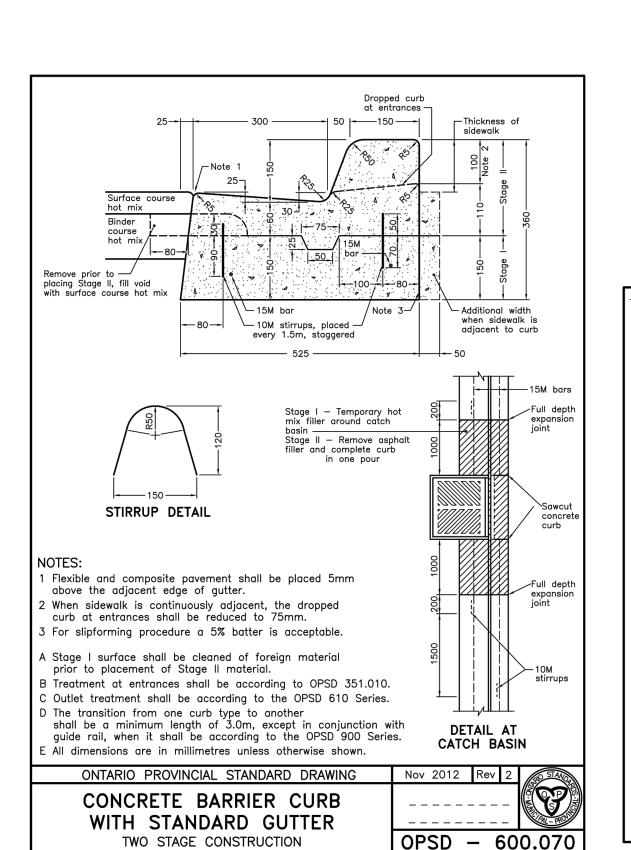
STORM AND SANITARY SEWERS:

- 1. SANITARY SEWERS SHALL BE GREEN PVC SDR 35 (UNLESS OTHERWISE NOTED) INSTALLED IN ACCORD. W/ OPSD 802.01 (NOV. 2010, REV. 2) SANITARY SERVICES SHALL BE GREEN 150mm DIA. PVC SDR 28 INSTALLED IN ACCORD. W/ OPS 1006.020 (NOV. 2005, REV. 1). MIN. 2% SLOPE INSTALLED 2.0m PAST CURB LINE. BEDDING AND COVER MATERIAL FOR SANITARY SEWERS SERVICE CONNECTIONS SHALL BE GRANULAR 'A' COMPACTED TO 98% STANDARD PROCTOR DENSITY
- 2. ALL SANITARY SEWERS TO BE INSTALLED AS PER COUNTY OF NORFOLK DESIGN GUIDELINES AND SPECIFICATIONS
- 3. STORM SEWERS SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED)
- 300mm DIA. PVC SDR-35 525mm DIA. AND GREATER - 65-D CONCRETE
- 4. CONCRETE STORM SEWERS SHALL BE INSTALLED IN ACCORDANCE WITH OPSD 802.03 (2010) CLASS B. BEDDING MATERIAL SHALL BE GRANULAR 'A' COMPACTED TO 98% S.P.D. COVER MATERIAL SHALL BE SAND COMPACTED TO
- 5. PVC STORM SEWERS SHALL BE INSTALLED IN ACCORDANCE WITH OPSD 802.01 (2010). BEDDING AND COVER MATERIAL SHALL BE GRANULAR 'A' COMPACTED 98% S.P.D.
- 6. SUB-GRADE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF GRANULAR 'B' TO CONFIRM GRANULAR 'B' THICKNESS REQUIRED.

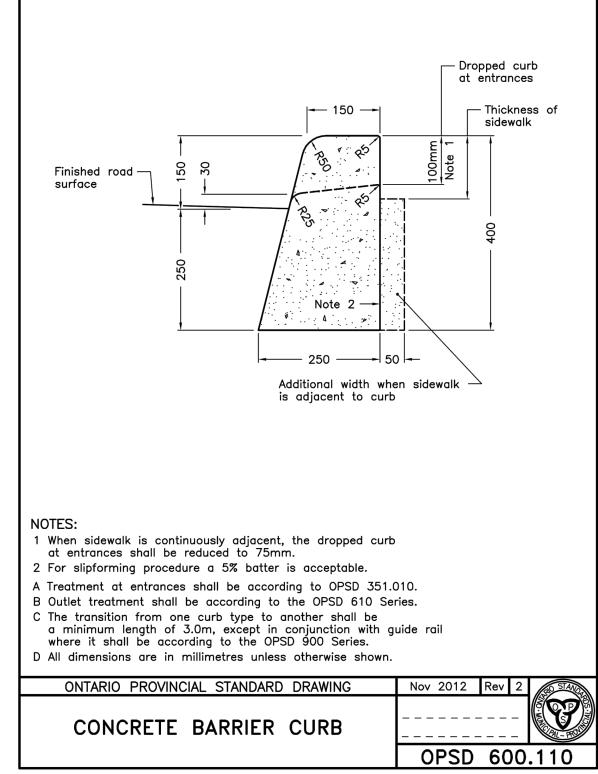
WATERMAINS:

- 1. ALL WATERMAINS AND APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT NORFOLK COUNTY DRAWINGS, M.O.E. GUIDELINES, OPSD STANDARDS AND SPECIFICATIONS AND AWWASTANDARDS AND
- WATERMAIN SHALL BE BLUE AWWA C-9-- PVC DR 18 CLASS 150 UNLESS OTHERWISE NOTED) INSTALLED IN ACCORDANCE WITH OPSD 1103.010 (NOV. 2006, REV. 1) AND 1103.020 (NOV. 2006, REV. 2). WATERMAIN SHALL HAVE TWELVE GAUGE TRACER WIRE FASTENED AT NO GREATER THAN 6.0m INTERVALS AND BROUGHT UP TO FINISHED GRADE AT WATER VALVE BOXES
- 3. WATER SERVICES TO BE 20mm DIA. TYPE 'K' COPPER PIPE, POLYETHYLENE OR MUNICIPEX (OR APPROVED EQUAL), IN ACCORDANCE WITH OPSD 1104.010 (NOV 2006, REV. 2) INSTALLED TO 3.0m PAST CURB LINE.
- 4. ALL WATERMAIN AND SANITARY SEWERS TO BE INSTALLED AS PER COUNTY OF NORFOLK DESIGN GUIDELINES AND SPECIFICATIONS.
- 5. BEDDING AND COVER MATERIAL FOR WATERMAIN AND SERVICE CONNECTIONS SHALL BE GRANULAR "A" COMPACTED TO 100% S.P.D. AS PER OPSD 802.010
- 6. WHERE WATERMAINS OR WATER SERVICES CONFLICT WITH SEWERS, THEY SHALL BE DEFLECTED BELOW THE SEWER TO PROVIDE A MINIMUM SEPARATION DISTANCE OF 0.50m VERTICALLY AND INSULATED.
- 7. VALVES SHALL BE RESILIENT SEAT EPOXY COATED GATE VALVES COMPLYING TO LATEST AWWA CSOO SPECIFICATIONS, VALVE BOXES AS SUPPLIED BY 'MUELER LIMITED' OR 'CANADA VALVE' INSTALLED IN ACCORDANCE WITH OPSD 1101.030 AND OPSD 1100.020.
- 8. HYDRANTS TO COMPLY TO AWWA C502. 'CANADA VALVE' (DARLING), 'CENTURY', 'MACIVITY M67' ARE ACCEPTED. MUST HAVE BRASS TO BRASS SEAT, OPEN COUNTER CLOCKWISE AND BE COATED WITH HIGH QUALITY YELLOW PAINT (OPSD 1105.02) (NOV.2006, REV. 1). ALL EQUIPPED WITH 100mm STORZ CONNECTION (CAP PAINTED BLACK).
- 9. SUB-GRADE TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF GRANULAR 'B' TO CONFIRM GRANULAR 'B' THICKNESS REQUIRED.
- 10. MINIMUM 1.7m COVER FOR WATERMAIN AND WATER SERVICES.
- 11. THE OWNER SHALL GRANT THE NECESSARY EASEMENT(S) TO NORFOLK COUNTY AND PORT DOVER ONTARIO TO ENABLE MAINTENANCE OF THE WATER SERVICES ON SITE AND TO INCLUDE A REQUIREMENT FOR A FIRE HYDRANT MAINTENANCE AGREEMENT FOR THE SITE.
- 12. THE FIRE FLOW DEMAND FOR THE SITE AND WATERMAIN DESIGN SHALL BE CALCULATED IN ACCORDANCE WITH THE SECTION 10.1.1 OF NORFOLK COUNTY DESIGN CRITERIA DATED AUGUST, 2017.





TWO STAGE CONSTRUCTION



Note 3 -

IST OF DRAWINGS

SS-01 SERVICING PLAN

SCALE: NOT TO SCALE

SG-01 SITE GRADING PLAN

C-01 EROSION CONTROL PLAN

TYPE II

4 Cold applied rubber asphalt joint sealing compound.

A Maximum height of slope above top of wall is 4m.

C All dimensions are in millimetres unless otherwise shown

ONTARIO PROVINCIAL STANDARD DRAWING

WALLS

RETAINING

CONCRETE TOE WALL

B Concrete for toe walls shall be 30MPa.

Varies

Note 1, Typ

TYPE I

└─ Note 3

JOINT DETAIL

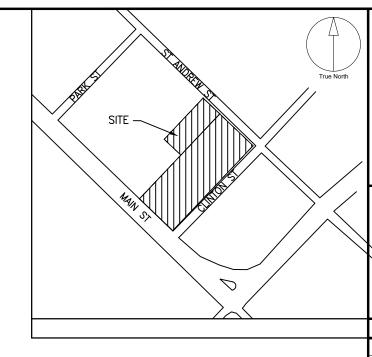
IN CONCRETE TOE WALLS

AT 3.0m SPACING

10mm x 45° → -

Note 6

as specified.



1000033566 ONTARIO INC.

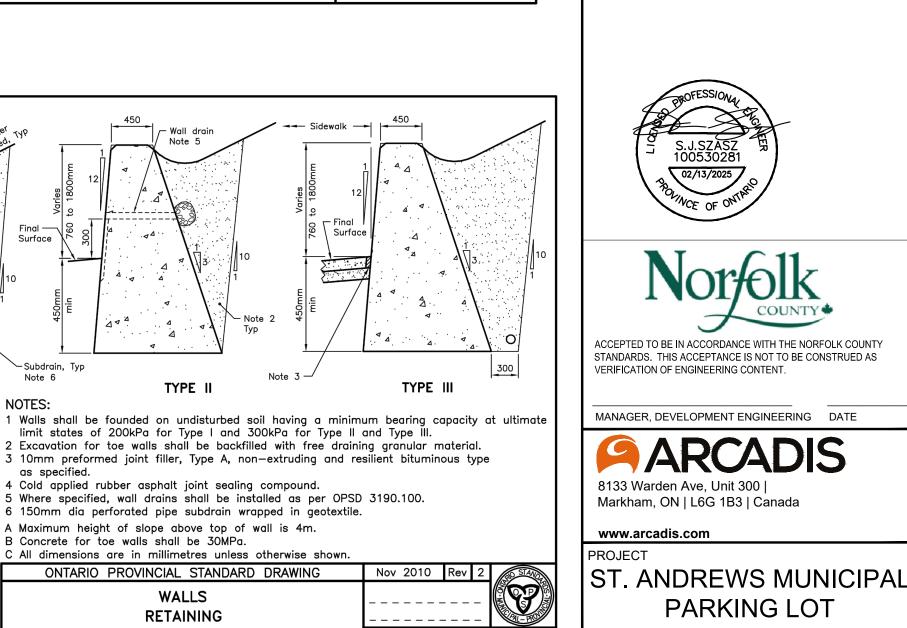
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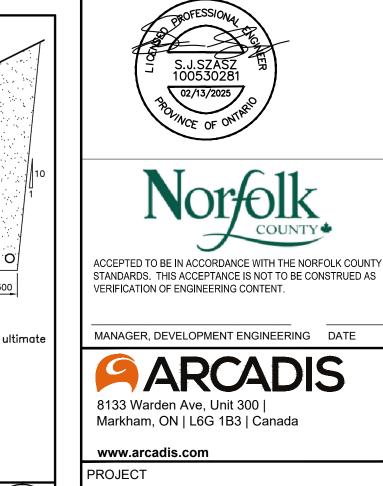
ntractors shall verify and be responsible for all dimensions and conditions on the job, and ARCADIS shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to ARCADIS for general conformance before proceeding with fabrication.

ARCADIS PROFESSIONAL SERVICES (CANADA) INC. is a member of ARCADIS Group of companies

DESCRIPTION DATE ISSUED FOR SPA SUBMISSION FEB. 13, 2025

<u>LEGEND</u>





148728

DRAWN BY:

PARKING LOT 25 ST. ANDREW ST., PORT DOVER NORFOLK COUNTY PROJECT NO:

DD-01 NOTES AND DETAILS DAP-01 PRE-DEVELOPMENT DRAINAGE AREA PLAN DAP-02 POST-DEVELOPMENT DRAINAGE AREA PLAN XS-01 CROSS SECTION SITE PLAN INFORMATION SURVEY INFORMATION VAN GROLL & ASSOCIATES INC. ASCH & HYDE LTD. ST CLAIRE AVE. W. F18 ITARIO LAND SURVEYORS

OPSD 3120.100

TORONTO, ONTARIO M4V 1L5 Box 6, 1333 Highway #3 East, Unit B HONE: (905) 339-2811 EXT 228 DUNNVILLE, ONT, N1A 2X1 JNNVILLE: 905-774-7188 RT ERIE: 905-871-975 SITE BENCHMARK IS TOP NUT OF FIRE HYDRANT LOCATED AT NORTHWESTERLY CORNER OF ST. ANDREWS STREET AND CLINTON STREET, AND HAVING AN LEVATION OF 182,32m).

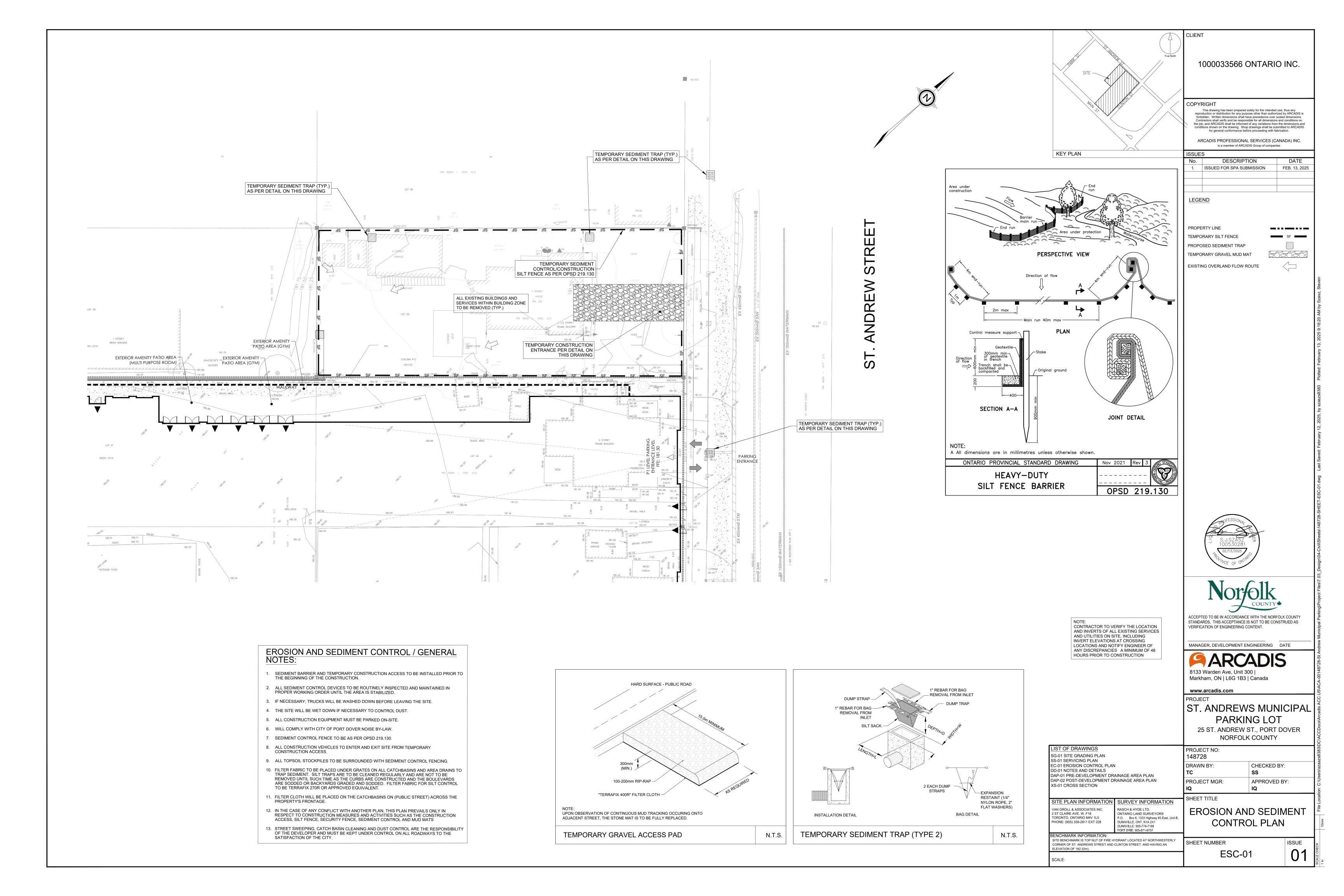
PROJECT MGR: APPROVED BY: SHEET TITLE **GENERAL NOTES**

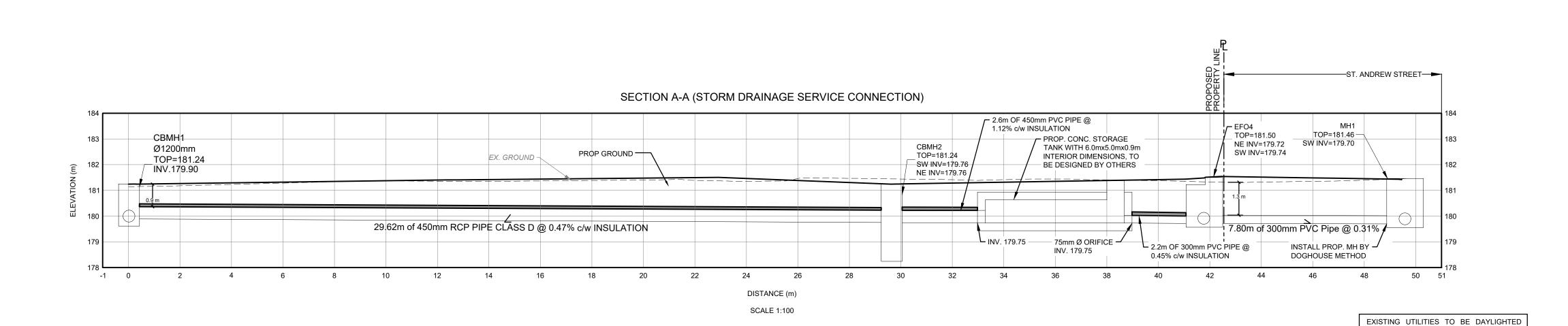
CHECKED BY:

AND DETAILS

DD-01

SHEET NUMBER





SITE KEY PLAN

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ISSUED FOR SPA SUBMISSION

ARCADIS PROFESSIONAL SERVICES (CANADA) INC.

1000033566 ONTARIO INC.

is a member of ARCADIS Group of companies

ISSUES

No. DESCRIPTION DATE

FEB. 13, 2025

→ EX.FH

LEGEND

PROPERTY LINE
PROPOSED SANITARY MANHOLE

PROPOSED SANITARY MANHOLE

PROPOSED STORM MANHOLE

PROPOSED AREA DRAIN

PROPOSED CATCH BASIN

EXISTING CATCH BASIN

□

PROPOSED VALVE AND BOX

□

V&B

EXISTING FIRE HYDRANT
PROPOSED SIAMESE CONNECTION
PROPOSED STORM

PROPOSED SANITARY
PROPOSED WATER
EXISTING COMBINED
EXISTING WATER

EXISTING WATER

EXISTING STORM

PROPOSED DOMESTIC WATER METER

M

PROPOSED DOMESTIC WATER METER
PROPOSED BACKFLOW PREVENTER
PROPOSED DOUBLE CHECK
DETECTOR ASSEMBLY

OROFESSIONAL S.J.SZASZ 100530281 02/13/2025 70/1/VCE OF ONTRE

ACCEPTED TO BE IN ACCORDANCE WITH THE NORFOLK COUNTY STANDARDS. THIS ACCEPTANCE IS NOT TO BE CONSTRUED AS

MANAGER, DEVELOPMENT ENGINEERING DATE

ARCADI: 8133 Warden Ave, Unit 300 | Markham, ON | L6G 1B3 | Canada

VERIFICATION OF ENGINEERING CONTENT.

www.arcadis.com

ST. ANDREWS MUNICIPAL PARKING LOT

25 ST. ANDREW ST., PORT DOVER NORFOLK COUNTY

LIST OF DRAWINGS

SG-01 SITE GRADING PLAN
SS-01 SERVICING PLAN
EC-01 EROSION CONTROL PLAN
DD-01 NOTES AND DETAILS
DAP-01 PRE-DEVELOPMENT DRAINAGE AREA PLAN
DAP-02 POST-DEVELOPMENT DRAINAGE AREA PLAN
XS-01 CROSS SECTION

PRIOR TO CONSTRUCTION TO CONFIRM LOCATION AND DEPTH. NOTIFY ENGINEER IF ANY DISCREPANCIES / CONFLICTS ARE IDENTIFIED. A MINIMUM OF 72 HOURS

BEFORE CONSTRUCTION

SITE PLAN INFORMATION

VAN GROLL & ASSOCIATES INC.
2 ST CLAIRE AVE. W. F18
TORONTO, ONTARIO M4V 1L5
PHONE: (905) 339-2811 EXT 228

SURVEY INFORMATION

RASCH & HYDE LTD.
ONTARIO LAND SURVEYORS
P.O. Box 6, 1333 Highway #3 East, Unit B,
DUNNVILLE, ONT, N1A 2X1

DUNNVILLE: 905-774-7188
FORT ERIE: 905-871-9757

BENCHMARK INFORMATION:
SITE BENCHMARK IS TOP NUT OF FIRE HYDRANT LOCATED AT NORTHWESTERLY
CORNER OF ST. ANDREWS STREET AND CLINTON STREET, AND HAVING AN
ELEVATION OF 182.32m).

SCALE: 1:100

PROJECT NO:
148728

DRAWN BY:
TC

PROJECT MGR:
IQ

APPROVED BY:
IQ

SHEET TITLE

CROSS-SECTION

SHEET NUMBER ISSUE

XS-01