

523 JAMES STREET, UNIT 3 DELHI, ON. N4B 2C2

PH: (519) 582-8222 F: (519) 582-2098

ATTN: Planning Department

Norfolk County

By Electronic Mail / Online Portal Submission Only

planning@norfolkcounty.ca

RE: Proposed Manufacturing Building,

Luscombe Drive, Simcoe, Norfolk County

Site Plan File: SPPL2025091

DATE: September 30, 2025

To Whom It May Concern,

As per the supplied Agency Comments to this Site Plan Application File, please see the attached materials with this letter as a response and resubmission of materials.

In conjunction with this letter, please see the attached CDN Buildings-supplied Comment Response Matrix and the items provided by J.H. Cohoon Engineering Limited.

The items from J.H. Cohoon Engineering Limited include their Comment Response Matrix/Letter, the Functional Servicing and Stormwater Management Report, the Schedule-H Estimated Cost and Securities Schedule, Site Plan, Grading and Siltation and Erosion Control Plan, Servicing Plan, and the Site Notes and Details.

Please also see the attached Land Appraisal Report.

We trust that the drawings, reports, and letters attached to this cover letter are satisfactory to Norfolk County for the requirements outlined in the Agency Comments. Please do not hesitate to contact the undersigned should any clarification be necessary.

Sincerely,

William Reed

Senior Architectural/Structural Technologist

CDN Buildings

523 James St. Unit 3

W. Reed.

Delhi, ON

N4B 2C2

wr@cdnbuildings.com

(519) 582-8222 - 232

24-1022

2712300 ONTARIO INCHFW HOLDINGS LIMITED

TONY KOUTNY

LUSCOMBE DRIVE [ARN 33104030250382500000] SIMCOE, ON

SPPL2025091

CDN BUILDINGS - COMMENT RESPONSES - 2025-09-30

	PLANNING COMMENTS					
NUMBER	COMMENT	RESPONSE				
1	PLEASE PROVIDE A LAND APPRAISAL AND A SECURITY SCHEDULE FOR THE AGREEMENT MATERIALS	ATTACHED WITH THIS SUBMISSION.				
2	GREAT SITE PLAN DRAWINGS, VERY DETAILED	noted, with thanks.				
BUILDING COMMENTS						
NUMBER	COMMENT	RESPONSE				
1	CONDITIONS:					
1.1	THE OWNER SHALL AGREE TO MAKE APPLICATION FOR A BUILDING PERMIT, AND OBTAIN THE NECESSARY BUILDING PERMITS PRIOR TO COMMENCING CONSTRUCTION	NOTED, APPLICANT IS APPLYING FOR BUILDING PERMIT APPLICATION CONCURRENTLY WITH SITE PLAN APPLICATION, DOCUMENTS TO BE UPDATED AND SUPPLIED AS MADE AVAILABLE				
1.2	AND FURTHER THAT ALL APPLICABLE LAW APPROVALS AS REQUIRED BY THE ONTARIO BUILDING CODE AND SUPPORTING DOCUMENTATION FROM APPROVAL AGENCIES RE SUBMITTED WITH A BUILDING PERMIT APPLICATION [OBC DIVISION A 1.4.1.3.] SPECIFICALLY: SOURCE WATER PROTECTION AND SITE PLAN APPROVAL.	NOTED. SECTION 59 SOURCE WATER PROTECTION AND SITE PLAN APPROVAL DOCUMENTS TO BE SUBMITTED WITH BUILDING PERMIT APPLICATION.				
1.3	AND FURTHER THAT THE OWNER SHALL AGREE TO PROVIDE CONFIRMATION FROM ENVIRONMENTAL INFRASTRUCTURE SERVICES (EIS) OF THE FOLLOWING:	NOTED,				
1.3.A.	SERVICING CAPACITY IS CONFIRMED AS BEING AVAILABLE FOR ALL WATERMAINS AND SANITARY SYSTEMS	REFER TO JH COHOON RESPONSE LETTER.				
1.3.B.	WATERMAINS, STORM SEWERS, SANITARY SYSTEMS AND HYDRO SERVICE ARE CONSTRUCTED AND OPERATIONAL AND ALL REGULATORY APPROVALS HAVE BEEN RECEIVED TO THE SATISFACTION OF THE COUNTY OR APPROPRIATE APPROVAL AUTHORITY	REFER TO JH COHOON RESPONSE LETTER.				
1.3.C.	STORMWATER MANAGEMENT FACILITIES, IF REQUIRED, ARE CONSTRUCTED AND OPERATIONAL	REFER TO JH COHOON RESPONSE LETTER.				
1.3.D.	ADEQUATE WATER SUPPLY IS AVAILABLE FOR FIRE PROTECTION	REFER TO JH COHOON RESPONSE LETTER.				
	CONTINUED ON 1	NEXT PAGE				
NUMBER	COMMENT	RESPONSE				

1	ROADWAYS AND/OR LANES ARE CONSTRUCTED TO THE SATISFACTION OF THE COUNTY AND PROVIDE SAFE AND ADEQUATE ACCESS TO ALL BUILDINGS OR STRUCTURES ZONING COM COMMENT NO COMMENTS FIRE COMM COMMENT	AKNOWLEDGED. AMENTS RESPONSE NOTED AENTS RESPONSE		
1	ENSURE ADEQUATE SPACE AND ACCESS FOR THE FUNCTION FOR THE FIRE DEPARTMENT CONNECTION (IF REQUIRED)	ACROSS FROM SITE, OPPOSITE SIDE OF LUSCOMBE, APPROXIMATELY IN-LINE WITH NORTH PROPERTY LINE. SITE SERVICING SHOWS HYDRANT [17233-3]		
2	ADEQUATE FIRE DEPARTMENT ACCESS THROUGH THE SITE	PROVIDED; FIRE ROUTE SHOWN ON SITE PLAN [A0.02] WITH DISTANCES, WIDTH, CENTRELINE RADII AND ACCESS LOCATIONS FOR BUILDINGS ON-SITE.		
13 1 1		NONE CURRENTLY PROPOSED - NOT APPLICABLE		
	CANADA POST C			
NUMBER	COMMENT	OMMENTS RESPONSE		
NUMBER 1	PLEASE BE ADVISED THAT CANADA POST DOES NOT HAVE ANY COMMENTS ON THIS APPLICATION FOR A NEW BUSINESS ON LUSCOMBE DR. THIS NEW BUSINESS WILL HAVE MAIL DELIVERY SERVICE THROUGH AN EXISTING COMMUNITY MAILBOX ON LUSCOMBE DR. THE CUSTOMER WILL HAVE TO REGISTER FOR MAIL DELIVERY THROUGH THE SIMCOE POST OFFICE OR BY CALLING OUR CUSTOMER SERVICE LINE AT 1-800-267-1177 TO REGISTER.	NOTED, OWNER TO REGISTER.		
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2.B.I.	W	REFER TO JH COHOON RESPONSE LETTER.
	TOTAL WASTEWATER DESIGN FLOWS SHOWN	REFER TO JH COHOON RESPONSE LETTER.
	IN SANITARY DESIGN SHEETS	
2.B.II.	TOTAL DOMESTIC WATER AND FIRE FLOWS AS PER NORFOLK COUNTY DESIGN CRITERIA SECTION 10.1.1.	REFER TO JH COHOON RESPONSE LETTER.
2.1	THE FUNCTIONAL SERVICING REPORT MUST INCLUDE WATER/SANITARY SERVICING AND FIRE FLOW CALCULATIONS. FIRE FLOW CALCULATIONS ARE TO BE COMPLETED IN ACCORDANCE WITH "WATER SUPPLY FOR PUBLIC FIRE PROTECTION 2020" BY FIRE UNDERWRITERS SURVEY.	REFER TO JH COHOON RESPONSE LETTER.
2.2	ONCE THE QUOTE HAS BEEN RECEIVED, APPROVAL FROM THE APPLICANT WILL BE REQUIRED BEFORE PROCESSING.	NOTED, APPLICANT TO PROVIDE APPROVAL ONCE QUOTE IS RECEIVED
3	SECURITIES WILL BE REQUIRED IN THE FORM OF A SCHEDULE H TEMPLATE. 10% FOR SITE WORKS COMPLETED ON PRIVATE PROPERTY AND 100% OF WORKS COMPLETED WITHIN THE MUNICIPAL R.O.W.	REFER TO JH COHOON RESPONSE LETTER.
3.A.	AS-CONSTRUCTED DRAWINGS MUST BE INCLUDED AS PART OF THE SECURITIES. THE MINIMUM AMOUNT OF \$1500.00 @ 100% WILL BE REQUIRED.	REFER TO JH COHOON RESPONSE LETTER.
	FUNCTIONAL SERVICING REPORT	
NUMBER	COMMENT	RESPONSE
4	IN ADDITION TO L/s PLEASE PROVIDE AVERAGE AND PEAK FLOW RATES IN m3/DAY	REFER TO JH COHOON RESPONSE LETTER.
5	PLEASE PROVIDE COMMENTARY ON THE SITE'S PROPOSED DRAINAGE, OUTLET CONTROLS AND OVERALL SWM STRATEGY	REFER TO JH COHOON RESPONSE LETTER.
6	ASSESS AND VERIFY THE DOWNSTREAM HYDRAULIC CAPACITY OF THE EXISTING 750mm STORM SEWER INFRASTRUCTURE WITHIN THE MUNICIPAL RIGHT-OF-WAY, RECOGNIZING IT WAS ORIGINALLY DESIGNED FOR A 5-YEAR STORM EVENT.	REFER TO JH COHOON RESPONSE LETTER.
7	CONFIRM AND EVALUATE THE MAJOR SYSTEM'S HISTORICAL DESIGN, WHICH PROVIDED ON-SITE PONDING IN LOW AREAS TO ACCOMMODATE A 25-YEAR STORM FREQUENCY. ENSURE THAT THE MAJOR SYSTEM CAPACITY AND DESIGN CRITERIA ARE STILL VALID AND ADEQUATE FOR THE	REFER TO JH COHOON RESPONSE LETTER.
	PROPOSED DEVELOPMENT.	
		NEXT PAGE

8	PLEASE IDENTIFY RUNOFF COEFFICIENTS FOR ALL AREAS	REFER TO JH COHOON RESPONSE LETTER.
9	CONFIRM IF PIPE STORAGE IS REQUIRED FOR THE 100-YEAR STORM EVENT. PROVIDE CALCULATIONS SHOWING THE ADEQUATE CAPACITY OF ONSITE PIPE STORAGE	REFER TO JH COHOON RESPONSE LETTER.
10	PROVIDE CALCULATIONS FOR SIZING ALL STORM PIPES, CATCH BASINS AND CULVERTS	REFER TO JH COHOON RESPONSE LETTER.
11	PLEASE REVISE THE CONCLUSION TO DESCRIBE ACCURATE DEVELOPMENT DETAILS, REFLECTING AN INDUSTRIAL DEVELOPMENT INSTEAD OF RESIDENTIAL	REFER TO JH COHOON RESPONSE LETTER.
	TRAFFIC BRIEF CO	
NUMBER	COMMENT	RESPONSE
12	NO COMMENTS	NOTED
	SITE PLAN COM	
NUMBER	COMMENT	RESPONSE
13	PROPERTY BOUNDARIES ARE TO BE EXACT SURVEYS COORDINATES, NOT 'APPROXIMATE BOUNDARIES'	NOTED, CDN WILL PROVIDE SURVEYED BEARINGS AND DISTANCES ON THE SITE PLAN. REFER TO JH COHOON RESPONSE LETTER.
14	INDICATE SNOW STORAGE AREAS	REFER TO JH COHOON RESPONSE LETTER.
	GRADING AND SILTATION AND EROSIC	ON CONTROL PLAN COMMENTS
15	PROVIDE A SECTION THROUGH THE SWM FACILITY IN BOTH DIRECTIONS	REFER TO JH COHOON RESPONSE LETTER.
16	PROVIDE INTERIOR GRAVEL DRIVEWAY AND ASPHALT ENTRANCE SPECIFICATIONS	REFER TO JH COHOON RESPONSE LETTER.
17	PLEASE PROVIDE A DIMENSION BETWEEN THE TWO PROPOSED ENTRANCES, AND THE WIDTH OF THE DRIVEWAYS. PER ENTRANCE BY-LAW 2016-32, A MINIMUM SPACING OF 30 METERS BETWEEN ENTRANCES IS REQUIRED ON COMMERCIAL/INDUSTRIAL LANDS.	CDN BUILDINGS SITE PLAN [A0.02] WILL PROVIDE DIMENSIONS FOR DISTANCING OF DRIVEWAYS. CONFIRMING THAT THE DRIVEWAYS HAVE MORE THAN 30m BETWEEN ENTRANCES
18	PLEASE CONFIRM IF THE POND NO. 1 AND NO. 2 AREA RETAINS THE 100 YR. STORM.	REFER TO JH COHOON RESPONSE LETTER.
19	PLEASE PROVIDE ADDITIONAL GRADES AND OVERLAND FLOW ARROWS NEAR PROPERTY LINES ON ADJACENT SITES	REFER TO JH COHOON RESPONSE LETTER.
20	DRAINAGE AREA 210 IS NOT PERMITTED TO DISCHARGE TO NEIGHBORING PROPERTIES. THE SITE STORMWATER AND OVERLAND FLOW MUST BE SELF-CONTAINED.	REFER TO JH COHOON RESPONSE LETTER.
	2144 - 204 2 04 5 2 4 5 4 5 4 5 4 5 4 5 4 5 4 5 5 5 6 5 6 5	
21	SWM POND OVERFLOW SHALL NOT DISCHARGE TO NEIGHBOURING PROPERTIES	REFER TO JH COHOON RESPONSE LETTER.
21		

	T	<u> </u>			
	IT SHOULD BE NOTED THAT 3:1 SLOPE ON A	CDN BUILDINGS TO PROVIDE CHAIN LINK FENCE			
22	SWM POND REQUIRE THE INSTALLATION OF A	around swm pond on its site plan [a0.02].			
	CHAIN LINK FENCE	REFER TO JH COHOON RESPONSE LETTER.			
	SITE SERVICING PLAN	N COMMENTS			
NUMBER	COMMENT	RESPONSE			
	PLEASE REVISE THE NAME OF THIS SHEET IN				
23	THE TITLE BLOCK. IT IS ASSUMED THAT THIS	REFER TO JH COHOON RESPONSE LETTER.			
23	DRAWING IS INTENDED TO BE THE SITE	REFER TO THE COHOON RESPONSE LETTER.			
	SERVICING PLAN				
	INSULATION FOR ALL PIPE REQUIRED				
24	STRUCTURES IS TO BE INDICATED ON THIS	REFER TO JH COHOON RESPONSE LETTER.			
	PLAN.				
	AS PER NORFOLK COUNTY DESIGN CRITERIA				
	SECTION 9.7.00., ALL SANITARY SEWER				
	LATERAL CONNECTIONS SHALL BE INSTALLED				
	USING PREFABRICATED TEES. HOWEVER, IT IS				
	ACCEPTABLE TO TIE INTO THE EXISTING				
25	MANHOLE PROVIDED THERE ARE NO	REFER TO JH COHOON RESPONSE LETTER.			
	CONFLICTS WITH INLET/OUTLETS ON THE				
	STRUCTURES, MINIMUM ADEQUATE				
	SEPARATIONS ARE MAINTAINED AND THE				
	MANHOLE IS RE-BENCHED TO CONFIRM ALL				
	ELEVATIONS ARE SUFFICIENT.				
	THE USE OF ANY SERVICE SADDLE MUST BE				
	APPROVED BY NORFOLK COUNTY ENVIRONMENTAL SERVICES DIVISION. ALL				
	CONNECTIONS SHALL CONFORM TO	REFER TO JH COHOON RESPONSE LETTER.			
25.1	CURRENT OPSD 1006.010 AND OPSS 410.				
	PLEASE INCLUDE THIS INFORMATION AS A				
	NOTE ON THE DRAWING IN YOUR NEXT				
	SUBMISSION.				
	PER NORFOLK COUNTY DESIGN CRITERIA				
26	9.7.00, SANITARY LATERALS SHALL BE	REFER TO JH COHOON RESPONSE LETTER.			
	INSTALLED ON A MINIMUM GRADE OF 2%				
	SANITARY INSPECTION MANHOLE SHALL BE				
27	LOCATED ON THE MUNICIPAL SIDE OF THE	refer to jh cohoon response letter.			
	PROPERTY LINE				
	PLEASE INCLUDE A PIPE CROSSING CHART IN				
	YOUR NEXT SUBMISSION, INCLUDING ALL				
	SEPARATIONS BETWEEEN INTERSECTING				
28	WATER, SANITARY AND STORM	REFER TO JH COHOON RESPONSE LETTER.			
	INFRASTRUCTURE. SEPARATIONS MUST				
	CONFORM TO MINIMUM MECP				
	DIMENSIONS.	70.05.04.04.4.04.05.4.05.7.0			
29	LOCATE ADJACENT HYDRANTS AND THE	TO BE SHOWN ON CDN SITE PLAN [A0.02]; REFER			
	ASSOCIATED FIRE SERVICE RADIUS	TO JH COHOON RESPONSE LETTER.			
20	PLEASE SHOW THE LIMITS OF ALL TRENCHING	DEFED TO THE COHOOSE PERSONS A STREET			
30		REFER TO JH COHOON RESPONSE LETTER.			
	ALL TRENCHES CONTINUED ON N	LIEVI DACE			
NUMBER	COMMENT	RESPONSE			
INUMBER	COMMENT	VESLOIASE			

31	PLEASE ADD A NOTE "ALL APPLICABLE PERMITS ARE TO BE APPLIED FOR PRIOR TO	REFER TO JH COHOON RESPONSE LETTER.		
	THE INSTALLATION OF ANY SERVICES"	THE TO SEE SOLID OF THE PROPERTY.		
32	PLEASE ADD A NOTE TO THE DRAWINGS "NO WORK ON WATER SERVICES CAN TAKE PLACE WITHOUT SUPERVISION OF A LICENSED NORFOLK COUNTY WATER OPERATOR ON-SITE"	REFER TO JH COHOON RESPONSE LETTER.		
33	PER NORFOLK COUNTY DESIGN CRITERIA 7.4.00, AREAS SUBJECT TO THE COLLECTION OF CONTAMINANTS OR SPILLS SHALL BE FITTED WITH ADEQUATE OIL/GRIT SEPARATORS	REFER TO JH COHOON RESPONSE LETTER.		
	CAMBIUM COA	MMENTS		
NUMBER	COMMENT	RESPONSE		
1	THERE ARE NO THREATS ASSOCIATED WITH THE PROPOSAL THAT ARE PROHIBITED OR REQUIRE A RISK MANAGEMENT PLAN	NOTED, RESTRICTED LAND USE NOTICE ISSUED MAY 20, 2025 BY MORGAN MARROTT, RMO.		
2	THE PROPOSED SEWAGE INFRASTRUCTURE WILL BE A THREAT WITHIN THE WELLHEAD PROTECTION AREA AND IS SUBJECT TO THE DESIGN REQUIREMENTS SET OUT IN NORFOLK COUNTY'S ENVIRONMENTAL COMPLIANCE APPROVAL DEVELOPED UNDER THE CONSOLIDATED LINEAR APPROACH.	REFER TO JH COHOON RESPONSE LETTER.		
3	POSSIBLE THREATS ARE LISTED IN APPENDIX A WHICH IS BASED ON THE 2017 TECHNICAL RULES WHICH ARE CURRENTLY IN EFFECT. PLEASE CONTACT THE RISK MANAGEMENT OFFICE IF ANY LISTED ACTIVITY IS PROPOSED AND LET US KNOW IF YOU HAVE ANY QUESTIONS OR REQUIRE FURTHER ASSISTANCE.	NOTED		
	CONTINUED ON N	NEXT PAGE		
	AGREEMENT COORDINA	ATOR COMMENTS		

NUMBER	COMMENT	RESPONSE			
1	COMMENTS PENDING	NOTED			
	REALTY SERVICES (
NUMBER	COMMENT	RESPONSE			
IF PERFORMANCE SECURITIES FOR THE INFRASTRUCTURE WORKS IS AN OBLIGATION OF YOUR SITE PLAN APPROVAL YOU WILL BE REQUIRED TO ENTER INTO A DEVELOPMENT AGREEMENT WITH THE COUNTY. THE AGREEMENT WILL BE REGISTERED ON TITLE AT THE OWNER'S EXPENSE. THE OWNER WILL ALSO BE REQUIRED TO SECURE COMMERCIAL GENERAL LIABILITY INSURANCE COVERAGE. THE COVERAGE MUST BE KEPT IN FORCE DURING THE DURATION OF CONSTRUCTION UNTIL AFTER A SUCCESSFUL SITE INSPECTION AND RELEASE OF THE PERFORMANCE SECURITIES. PLEASE REACH OUT IF AN AGREEMENT IS REQUIRED.		NOTED - DEVELOPMENT AGREEMENT PER OWNER, SECURITIES SCHEDULE SUBMITTED WITH THIS SUBMISSION.			
AUUAADED	GIS COMM				
NUMBER	COMMENT	RESPONSE			
1	PLEASE CONTACT NORFOLK GIS FOR NEW CIVIC ADDRESSES WHEN BUILDING. YOU CAN APPLY FOR A NEW CIVIC ADDRESS HERE. IF A GREEN SIGN IS REQUIRED IN ORDER TO ISSUE AN ADDRESS (GENERALLY ANYWHERE OUTSIDE OF AN URBAN AREA) YOU WILL HAVE TO CALL NORFOLK COUNTY CUSTOMER SERVICE AFTER APPLYING TO MAKE PAYMENT BEFORE THE ADDRESS IS ISSUED (519-426-5870) (226-NORFOLK)	NOTED, VACANT PARCEL, TO BE DEALT WITH AS PART OF THE BUILDING PERMIT PROCESS.			
IF YOU WOULD LIKE TO APPLY FOR A NEW CIVIC ADDRESS BECAUSE YOU ARE PLANNING TO BUILD ON A VACANT PARCEL OF LAND, THIS IS DEALT WITH AS PART OF THE BUILDING PERMIT PROCESS. THE BUILDING INSPECTOR CAN PROVIDE YOU WITH A COPY OF A CIVIC ADDRESS REQUEST FORM OR IT CAN BE DOWNLOADED BELOW. ON THE FORM THERE ARE SEVERAL AREAS THAT NEED TO BE FILLED OUT WITH INFORMATION, AND A SKETCH SHOWING THE LOT LAYOUT OF THE PROPERTY FOR WHICH THE CIVIC ADDRESS IS BEING REQUESTED. A SAMPLE SKETCH WILL BE INCLUDED WITH THE FORM.		NOTED, INSPECTOR TO PROVIDE COPY OF FORMS.			
	CONTINUED ON 1				
	HYDRO ONE CO	DMMENTS COMMENTS			

NUMBER	COMMENT	RESPONSE
1	COMMENTS PENDING	NOTED
		THE CREDIT COMMENTS
NUMBER	COMMENT	RESPONSE
1	COMMENTS PENDING	NOTED
		NS COMMENTS
NUMBER	COMMENT	RESPONSE
1	COMMENTS PENDING	NOTED
	END OF I	DOCUMENT



J.H. COHOON ENGINEERING LIMITED

CONSULTING ENGINEERS

September 18, 2025

Norfolk County 12 Gilbertson Drive Simcoe, Ontario N3Y 5L6

Attention:

Dear:

Re: Luscombe Drive

Simcoe, Ontario

(2712300 Ontario INCHFW Holdings Limited)

SPPL2025091

In response to the comments received with regards to the above noted application, we have provided you with the comments received with the appropriate explanation for your review and approval.

Planning Comments

- 1. Please provide a Land appraisal and a security schedule for the agreement materials. The land appraisal is by others and the construction cost estimate is attached
- 2. Great Site Plan drawings, very detailed. Information only and acknowledged

Building Comments

Conditions:

1. The Owner shall agree to make application for a Building Permit, and obtain the necessary Building Permits prior to commencing construction.

Information only and acknowledged

2. AND FURTHER THAT all applicable law approvals as required by the Ontario Building Code and supporting documentation from approval agencies re submitted with a building permit application. [OBC Division A 1.4.1.3]. Specifically: Source water protection, site plan approval.

Information only and acknowledged



- 3. AND FURTHER THAT the Owner shall agree to provide confirmation from Environmental Infrastructure Services (EIS) of the following:
 - a. Servicing capacity is confirmed as being available for all watermains and sanitary systems;

The modelling is carried out by the Norfolk County staff and is based upon the FSR previously provided.

b. Watermains, storm sewers, sanitary systems and hydro service are constructed and operational and all regulatory approvals have been received to the satisfaction of the County or appropriate approval authority;

Information only and acknowledged

- c. Stormwater management facilities, if required, are constructed and operational; **Information only and acknowledged**
- d. Adequate water supply is available for fire protection; and e) roadways and/or lanes are constructed to the satisfaction of the County and provide safe and adequate access to all buildings or structures.

Information only and acknowledged

Fire Comments

1. Ensure adequate space and access for the function for the fire department connection (if required).

Information only and acknowledged. The fire route as shown meets the requirement of the Ontario Building Code. The buildings are not anticipated to be sprinklered.

2. Adequate fire department access through the site. Information only and acknowledged. The fire route as shown meets the requirement of the Ontario Building Code.

3. If battery storage (i.e., Tesla wall) and/or electric vehicle charging infrastructure is provided please notify NCFD.

Information only and acknowledged. No electric vehicle charging infrastructure is being provided to this site.

Development Engineering Comments General

General

1. Please provide a response matrix in your next submission to address all comments. This letter acts as a comment matrix for this site.

- 2. Sanitary and Water modelling will be required. This is to be completed by Norfolk County's third-party consultant. The cost to complete the modelling and any recommendations from reports are to be implemented into the design at the applicant's expense. The following information will be required to receive a quote and complete the modelling.
 - a. General Plan of Services

Included within this submission

- b. Functional Servicing Report;
 - i. Total Wastewater Design Flows shown in Sanitary Design Sheets;

Included in the re-submission

ii. Total Domestic Water and Fire Flows as per Norfolk County Design Criteria Section 10.1.1

Included with the FSR report

The Functional Servicing Report must include water / sanitary servicing and fire flow calculations. Fire Flow calculations are to be completed in accordance with "Water Supply for Public Fire Protection 2020" by Fire Underwriters Survey.

Once the quote has been received, approval from the applicant will be required before proceeding.

Information only and acknowledged

- 3. Securities will be required in the form of a Schedule 'H' template. 10% for site works completed on private property and 100% of works completed within the municipal R.O.W.
 - a. As-constructed drawings must be included as part of the securities. The minimum amount of \$1,500.00 @ 100% will be required.

Included within the cost estimate.

Functional Servicing Report (April 2025):

- 4. In addition to L/s, please provide average and peak flow rates in m³/day. Updated FSR attached.
- 5. Please provide commentary on the site's proposed drainage, outlet controls and overall SWM strategy.

Updated FSR attached

6. Assess and verify the downstream hydraulic capacity of the existing 750mm storm sewer infrastructure within the municipal right-of-way, recognizing it was originally designed for a 5-year storm event.

The stormwater management report included for this development restricts the rate of runoff up to and including the 100 year storm event. Therefore, no impact to the downstream system is anticipated.

7. Confirm and evaluate the major system's historical design, which provided on-site ponding in low areas to accommodate a 25-year storm frequency. Ensure that the major system capacity and design criteria are still valid and adequate for the proposed development.

The stormwater management report included for this development restricts the rate of runoff up to and including the 100 year storm event. Therefore, no impact to the downstream system is anticipated.

8. Please identify runoff coefficients for all areas.

The stormwater management report includes the imperious surface calculations for the site and for this development restricts the rate of runoff up to and including the 100 year storm event. Therefore, no impact to the downstream system is anticipated.

9. Confirm if pipe storage is required for the 100-year storm event. Provide calculations showing the adequate capacity of onsite pipe storage.

Pipe storage although it will be utilized, is not accounted for the stormwater design.

10. Provide calculations for sizing all storm pipes, catch basins and culverts.

The sizing of the piping is embedded within the stormwater management report (attached).

11. Please revise the conclusion to describe accurate development details, reflecting an industrial development instead of residential.

The functions servicing report has been updated accordingly.

Site Plan (DWG No: 17233-1)

- 13. Property boundaries are to be exact surveys coordinates, not "Approximate Boundaries". The boundaries are included on the survey to be provided under separate cover
- 14. Indicate snow storage areas.

The snow storage areas are illustrated on the plans.

Grading and Siltation & Erosion Control Plan (DWG No: 17233-2):

15. Provide a section through the SWM Facility in both directions.

The site plan has been updated accordingly.

16. Provide interior gravel driveway and asphalt entrance specifications.

The specification is included on the updated drawing.

17. Please provide a dimension between the two proposed entrances, and the width of the driveways. Per Entrance By-Law 2016-32, a minimum spacing of 30 meters between entrances is required on commercial/industrial lands.

The site plan has been updated accordingly.

18. Please confirm if the Pond No. 1 and No. 2 area retains the 100yr Storm.

These ponds are filled during the major storm event with the overland flow heading towards the stormwater management pond where additional control is provided.

19. Please provide additional grades and overland flow arrows near property lines on adjacent sites.

The site plan has been updated accordingly.

20. Drainage area 210 is not permitted to discharge to neighboring properties. The site stormwater and overland flow must be self-contained.

The drainage on the site is contained and directed to the stormwater management facility whereas no discharge to the abutting lands is anticipated with the exception of extremely high events.

21. SWM pond overflow shall not discharge to neighboring properties.

The pre-development drainage direction presently is directed towards the rear of the site and with the grades that are existing, direction of the overland flow to the front / municipal right-of-way is impossible to achieve without extensive amounts of fill.

22. It should be noted that 3:1 slope on a SWM Pond require the installation of a chain-link fence.

The site plan has been updated accordingly.

Site Servicing Plan (DWG No: 17233-3):

23. Please revise the name of this sheet in the title block. It is assumed that this drawing is intended to be the Site Servicing Plan.

The site plan has been updated accordingly.

24. Insulation for all pipe required structures is to be indicated on this plan.

The site plan has been updated accordingly.

25. As per Norfolk County Design Criteria Section 9.7.00, all sanitary sewer lateral connections shall be installed using prefabricated tees. However, it is acceptable to tie into the existing manhole provided there are no conflicts with inlet/outlets on the structures, minimum adequate separations are maintained and the manhole is re-benched to confirm all elevations are sufficient.

The site plan has been updated accordingly.

The use of any service saddle must be approved by Norfolk County Environmental Services Division. All connections shall conform to current OPSD 1006.010 and OPSS 410. Please include this information as a note on the drawing in your next submission.

The site plan has been updated accordingly.

26. Per Norfolk County Design Criteria 9.7.00, sanitary laterals shall be installed on a minimum grade of 2%.

The site plan has been updated accordingly. Noting that the initial length which has sufficient capacity is at 0.5%. We would consider this a sewer and not a lateral.

27. Sanitary inspection manhole shall be located on the municipal side of the property line. The site plan has been updated accordingly.

28. Please include a pipe crossing chart in your next submission, including all separations between intersecting water, sanitary and storm infrastructure. Separations must conform to minimum MECP dimensions.

The site plan has been updated accordingly.

29. Locate adjacent hydrants and the associated fire service radius.

The site plan has been updated accordingly.

- 30. Please show the limits of all trenching and limits of proposed restoration of all trenches. **The site plan has been updated accordingly.**
- 31. Please add a note "All applicable permits are to be applied for prior to the installation of any services".

The site plan has been updated accordingly.

32. Please add a note to the drawings "No work on Water services can take place without supervision of a licensed Norfolk County Water Operator on-site".

The site plan has been updated accordingly.

33. Per Norfolk County Design Criteria 7.4.00, areas subject to the collection of contaminants or spills shall be fitted with adequate oil/grit separators.

Information only and acknowledged.

If you have any further questions, or concerns, please do not hesitate to contact this office.

Yours truly,

J.H. COHOON ENGINEERING LIMITED

R.W. Phillips, P. Eng.

PROPOSED MIXED USE BUILDING

Luscombe Drive SCHEDULE H ESTIMATED COST AND SECURITIES OUR PROJECT NO. 17233

"2025-09-18

EVISION

ITEM	DESCRIPTION	UNIT	QTY.	UNIT PRICE	TOTAL COST	Securi	
	CANITA DV CELLEDO					10%	100% .
	SANITARY SEWERS						
1	Excavate to existing sanitary sewer stub at Property Line	L.S.	1	\$1,000	\$1,000	\$0	\$1,000
2	Sanitary Sewer Size Noted below	matra	10	¢1 <i>E</i> 0	¢1 470	4 0	¢1 470
	a. Existing MH to S2 150 mm Dia b. S2 to S1 150mm Dia	metre metre	10 29	\$150 \$150	\$1,470 \$4,275	\$0 \$428	\$1,470 \$0
	b. S2 to S1 150mm Dia	metre	7	\$150	\$1,050	\$105	\$0
	d. S2 to S1 150mm Dia	metre	24	\$150	\$3,660	\$366	\$0
3	Construct 1200mm Dia concrete maintenance holes complete.	EA	2	\$4,500	\$9,000	\$900	\$0
4	CCTV and Report	L.S.	1	\$2,500	\$2,500	\$250	\$0
	TOTAL SANITARY SEWERS				\$22,955	\$2,049	\$2,470
R \	VATERMAIN						
1	Excavate to existing watermain,	L.S.	1	\$700	\$700	\$0	\$700
2	Watermain				·		
2	of granular base complete.						
	a. Main to Property Line - 50mm Dia	metre	15	\$105	\$1,575	\$0	\$1,575
	a. Property Line to Bldgs - 50mm Dia	metre	42	\$105	\$4,410	\$441	\$0
	TOTAL WATERMAIN				\$5,985	\$441	\$1,575
C . :	STORM SEWERS						
1	Supply and install storm sewer						
	a. 375 mm Dia	metre	207	\$175	\$36,278	\$3,628	\$0
	b. 250 mm dia.	metre	47	\$175	\$8,260	\$826	\$0
	c. 450 mm Dia. d. 525 mm Dia.	metre metre	16 15	\$175 \$175	\$2,730 \$2,573	\$273 \$257	\$0 \$0
	e. 250 mm dia. In ROW	metre	14	\$175	\$2,363	\$0	\$2,363
2	Construct 600x600 precast						
	concrete catchbasin	F.4	0	#0 500	#10.500	#1.050	# 0
	including associated legs in ROW	EA EA	3 1	\$3,500 \$3,500	\$10,500 \$3,500	\$1,050 \$0	\$0 \$3,500
3	Construct 1200 mm P/C CB MH	Γ.	_	¢2 500	¢17.500	¢1.750	\$ 0
	complete	EA	5	\$3,500	\$17,500	\$1,750	\$0
4	Stormceptor	L.S.	1	\$35,000	\$35,000	\$3,500	\$0
5	Outlet Structure	L.S.	1	\$3,500	\$3,500	\$350	\$0
6	Headwall	L.S.	1	\$4,500	\$4,500	\$450	\$0
	TOTAL STORM SEWER			_	\$126,703	\$12,084	\$5,863
D. I	ROAD CONSTRUCTION						
1	Sub excavation to a depth of 590mm below finish grade		0105	6.1 E	#101 07F	#10.100	
		sq.m.	8125	\$15	\$121,875	\$12,188	\$0
2	Supply, place and compact						
	300mm granular 'B'	sq.m.	8125	\$15	\$121,875	\$12,188	\$0
2	Supply place and compact						
3	Supply, place and compact 150mm granular 'A'	sq.m.	8125	\$8	\$60,938	\$6,094	\$0
4	Supply, place and compact 50mm HL4 asphalt pavement						
	including road cut						
		tonne	1025	\$165	\$169,125	\$16,913	\$0

ITEM	DESCRIPTION	UNIT	QTY.	UNIT PRICE	TOTAL COST	Secui	rities
						10%	100% .
5	Supply, place and compact 40mm HL3 asphalt for road.	tonne	725	\$175	\$126,875	\$12,688	\$0
6	Mud Mat and Site Controls	L.S.	1	\$4,000	\$4,000	\$400	\$0
7	Line Painting	L.S.	1	\$1,500	\$1,500	\$150	\$0
8	Proposed Topsoil and Seed	sq. m.	2700	\$8	\$20,250	\$2,025	\$0
	TOTAL ROAD CONSTRUCTION				\$626,438	\$62,644	\$0
E.	HYDRO Intestinally Removed	L.S.	1	\$0	\$0	\$0	\$0_
	TOTAL HYDRO				\$0	\$0	\$0
SUI	MMARY A. SANITARY SEWERS			_	\$22,955	\$0	\$22,955
	B. WATERMAIN				\$5,985	\$441	\$1,575
	C. STORM SEWERS			-	\$126,703	\$12,084	\$5,863
	D. ROAD CONSTRUCTION			-	\$626,438	\$62,644	\$0
	E. HYDRO			-	\$0	\$0	\$0
	TOTAL NORFOLK COUNTY ITEMS				\$782,080	\$75,169	\$30,393
	TOTAL HYDRO ITEMS				\$0	\$0	\$0
	TOTALS				\$782,080	\$75,169	\$30,393
	TOTAL SECURITIES REQUIRED)			Ī	\$1	05,561.25

KOUTNY TONY

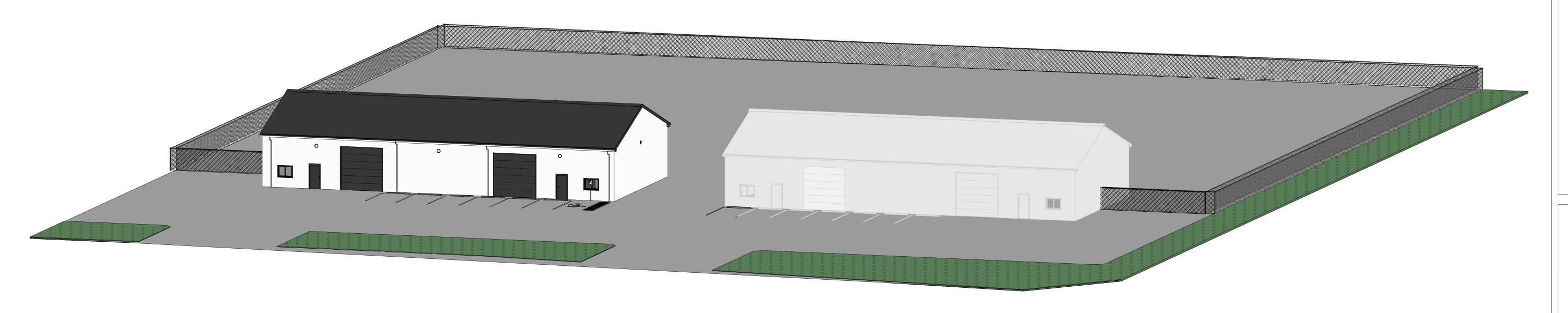
CONTACT: TONY KOUTNY **PHONE:** (905)973-0791

PROPERTY ADDRESS: LUSCOMBE DRIVE,

SIMCOE, ON

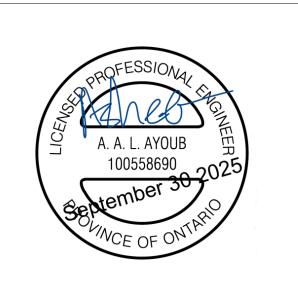
PHONE: tony@koutny.ca

50'-0"W x 100'-0"L x 14'-0"H (4/12 PITCH) CLEAR SPAN STEEL MANUFACTURING BUILDING





Office: 519 582 8222 Fax: 519 582 2098



DEV.	DECODIDATION	F : ==
REV	DESCRIPTION	DATE
1	ISSUED FOR COORDINATION	2024/07/23
2	ISSUED FOR PERMITS	2024/12/19
3	ISSUED FOR PERMITS	2025/03/2
4	AS PER COMMENTS	2025/06/1
5	AS PER COMMENTS	2025/09/30

COLOURS:

WALLS: CHARCOAL
ROOF: BLACK
TRIM: BLACK
DOORS: BLACK (DEFAULT)
WINDOWS: BLACK (DEFAULT)

KOUTNY TONY

LUSCOMBE DRIVE, SIMCOE,ONTARIO

COVER SHEET

PROJECT NUMBER

DATE

DRAWN BY

CHECKED BY

A0.00

SCALE

24-1022

2024/07/02

MATERIAL SPECIFICATIONS		
PART	MATERIAL	FINISH
WIDE FLANGE, ANGLE, CHANNEL	G40.21 350W, ASTM A992 GR50, ASTM A572 GR50	GALVANIZED
HSS	G40.21 350W, ATSM 500 GRADE C OR EQUIVALENT	GALVANIZED/BLACK
PLATE/FLAT BAR	G40.21 380W	GALVANIZED
WEB BAR	G40.21 380W, ASTM A653 SS GR55, ASTM A653 HSLAS GR55 CLASS 1 OR 2	GALVANIZED
COLD FORMED ZEE & CEE	G40.21 380W, ASTM A653 SS GR33, ASTM A653 HSLAS GR55 CLASS 1 OR 2	GALVANIZED
	ASTM A101 SS GR55, ASTM A101 HSLAS GR55 CLASS 1 OR 2	
COLD FORMED NOTCHED ZEE & HAT/CHAIR	G40.21 380W, ASTM A653 SS GR33, ASTM A653 HSLAS GR55 CLASS 1 OR 2	Z275(G90) PRE-GALV
ROD BRACING	G40.21 350W, ASTM A529 GR50, ASTM A572 GR50	GALVANIZED
CABLE BRACING	ASTM A475 EHS 7 WIRE	CLASS A ZINC COATING
HIGH STRENGTH BOLTS < 1-1/2"	ASTM A325, GALV. GRADE 5	GALVANIZED
HIGH STRENGTH BOLTS > 1-1/2"	ASTM A325, GALV. GRADE 5	GALVANIZED
SECONDARY STRUCTURAL MEMBER BOLTS	ASTM A325, GALV. GRADE 5	GALVANIZED
ANCHOR RODS	G40.21 350W, ASTM A529 GR50,ASTM A572 GR50	GALVANIZED
EYE BOLTS	1030 CARBON STEEL	HOT DIP GALVANIZED
HILLSIDE/SLOPED WASHER	A47	GALVANIZED A153
BRACER HILLSIDE WASHER	ASTM A536 GR65	GREY ENAMEL
RTL PANEL	ASTM A792 SS GR50 CLASS 1 OR 4	AZ50 / AZM150 FOR PAINTED GALVALUME
HEX BOLT ASSEMBLIES	GRADE 5 STEEL	GALVANIZED
REBAR	GRADE 400W OR EQUIVALENT	AZ55 / AZM165 FOR BARE GALVALUME
SELF TAPPING SCREWS (TEK 3)	GRADE 2 STEEL	ZINC COAT
DIAMOND RIB 36 SHEET METAL PANEL	26 - 29 GA STRUCTURAL GRADE 80 STEEL ASTM 972	SMP STEEL-PAINT SYSTEM
TUFF RIB SHEET METAL PANEL	26 - 29 GA STRUCTURAL GRADE 80 STEEL ASTM 972	SMP STEEL-PAINT SYSTEM

ALL STRUCTURAL MEMBERS OF THE STEEL BUILDING SYSTEM TO BE HOT DIP GALVANIZED TO ASTM A 123 AND FASTENERS TO ASTM A 153.

COLD	FORMED	ZEE/CEE	MEMBER	SIZES

08Z16; WHERE 08=SECTION DEPTH, Z=ZEE SECTION 16=16GA DESIGN EXAMPLES: 10C12; WHERE 10=SECTION DEPTH, C=CEE SECTION, 12=12GA

PART	DEPTH in[mm]	FLANGE WIDTH in [mm]	LIP LENGTH in[mm]	LIP ANGLE
06Z	6[152]	2.50[64]	0.95[24]	45
08Z	8[203]	2.80[71]	1.08[27]	45
09Z	9[229]	2.88[73]	1.08[27]	45
10Z	10[254]	3.02[77]	1.18[30]	45
12 Z	12[305]	3.14[80]	1.18[30]	45
14Z	14[356]	3.14[80]	1.18[30]	45
06C	6[152]	2.26[57]	0.94[24]	90
08C	8[203]	2.94[75]	0.94[24]	90
09C	9[229]	3.08[78]	0.94[24]	90
10C	10[254]	3.26[83]	0.94[24]	90
12C	12[305]	3.38[86]	0.94[24]	90
14C	14[356]	3.50[89]	0.94[24]	90

DAMAGE TO MATERIAL FINISHES

MINOR ABRASIONS TO GALVANIZED FINISH, CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION; ARE UNAVOIDABLE, AND ARE NOT SUBJECT TO CLAIM, TOUCH UP OF THESE MINOR ABRASIONS IS THE RESPONSIBILITY OF THE ERECTOR AND/OR THE END CUSTOMER.

GENERAL NOTES

- INFORMATION ON THIS DRAWING IS INTENDED FOR CONSTRUCTION ONLY WHEN BEARING A CDN BUILDINGS ENGINEERS SIGNED PROFESSIONAL SEAL AND WHEN FREE OF ANY NOTATIONS STATING OTHERWISE.
- FOUNDATION DESIGN AND CONSTRUCTION IS NOT THE RESPONSIBILITY OF CDN BUILDINGS.
- 3. THE EMBEDMENT OF THE ANCHOR BOLTS IN THE CONCRETE IS THE RESPONSIBILITY OF THE FOUNDATION DESIGNER. THE FRAME REACTIONS LISTED ARE THE MINIMUM LOADS TO BE DEVELOPED
- THE BUILDING REACTION DATA REPORTS THE LOAD WHICH THIS BUILDING PLACES ON THE FOUNDATIONS THE ENDWALL WIND LOAD REACTIONS INCLUDE REACTION FROM ENDWALL BRACING
- COLUMN BASE PLATES ARE DESIGNED ASSUMING A MINIMUM SPECIFIED COMPRESSIVE STRENGTH (fc ') OF CONCRETE
- OD 2,900 P.S.1 (20MPA) AT 28 DAYS.
- ANCHOR BOLT DIAMETER, QUANTITY AND PLACEMENT SHOULD BE AS SHOWN B. ANCHOR BOLTS ARE TO BE ASTM A307 OR EQUAL
- ALL REACTIONS ARE IN KIPS OR KIP-FEET.
- 0. MAXIMUM FRAME REACTIONS INCLUDE WIND AND SEISMIC REACTION FROM SIDEWALL BRACING. SEISMIC REACTION HAVE NOT BEEN AMPLIFIED BY Rd, Ro
- 12. FOUNDATION IS DESIGNED WITH THE ASSUMPTION THAT THE SOIL BEARING CAPACITY IS 3000 psf.

13. GEOTECHNICAL SOIL REPORT TO OTHERWISE STATE SOIL BEARING CAPACITY. BASE LOAD DEFINITIONS FOR RIGID FRAMES

DEAD - SELFWEIGHT OF THE BUILDING SYSTEM. COLLATERAL - MECHANICAL, ELECTRICAL, CEILINGS, SPRINKLER, ETC.

ROOF_LIVE - MINIMUM ROOF LIVE LOAD (NOT COMBINED WITH SNOW LOAD). FLOOR_LIVE - LOADS INDUCED DUE TO INTENDED USE AND OCCUPANCY.

CRANE - LOADS INDUCED FROM CRANES IN THE BUILDING. SNOW - GRAVITY LOADS DUE TO SNOW

WIND_LEFT1 - WIND FROM THE LEFT TO RIGHT COMBINED WITH POSITIVE INTERNAL PRESSURE. WIND_RIGHT1 - WIND FROM THE RIGHT TO LEFT COMBINED WITH POSITIVE INTERNAL PRESSURE.

WIND LEFT2 - WIND FROM THE LEFT TO RIGHT COMBINED WITH NEGATIVE INTERNAL PRESSURE.

WIND_RIGHT2 - WIND FROM THE RIGHT TO LEFT COMBINED WITH NEGATIVE INTERNAL PRESSURE. WIND_LONG1 - WIND FROM NORMAL TO THE RIGID FRAME COMBINED WITH POSITIVE INTERNAL PRESSURE. WIND_LONG2 - WIND DIRECTION REVERSED FROM LNWIND_1 COMBINDED WITH POSITIVE INTERNAL PRESSURE.

SEISMIC LEFT - SEISMIC FORCE IN DIRECTION FROM LEFT TO RIGHT. SEISMIC_RIGHT - SEISMIC FORCE IN FREIGHT FROM RIGHT TO LEFT.

SEISMIC_LONG - SEISMIC FORCE NORMAL TO THE RIGID FRAME.

FIUNB_SL_L - GRAVITY LOADS DUE TO PATTERNED SNOW LOAD FOR RAFTER DESIGN. FIUNB SL R – GRAVITY LOADS DUE TO PATTERNED SNOW LOAD FOR RAFTER DESIGN

FIPAT_SL - GRAVITY LOADS DUE TO UNBALANCED SNOW LOAD.

TYPICAL MEMBER SIZES				
		ALATERIA!		
NO.	ITEM	MATERIAL		
1	ANCHOR POSTS (MAIN)	4 1/2" HSS TUBE X 0.125" WALL X 54" LONG HDG		
2	ANCHOR POSTS (GABLE)	3 1/2" HSS TUBE X 0.125" WALL X 54" LONG HDG		
3	MAIN POSTS	5" HSS TUBE X 0.125" WALL x'-x" LONG HDG		
4	GABLE/END POSTS	4" HSS TUBE X 0.125" WALL X TO SUIT HDG		
5	X'W TRUSS WELDMENTS	TOP CHORDS		
	ON X'-X" CENTERS	3" SQ HSS TUBE X 0.125" WALL HDG		
	TYP OF #	BOTTOM CHORDS		
		3" SQ HSS TUBE X 0.125" WALL HDG		
		WEBBING MEMBERS		
		2" SQ HSS TUBE X 0.125" WALL HDG		
6	ROOF PURLINS	2" SQ HSS TUBE X 0.125" WALL @ 36" C/C		
7	WALL BRACING	1/2"Ø THREADED STEEL RODS @ 12'-0" LENGTHS HDG		
8	WALL PURLINS	2" SQ HSS TUBE X 0.125" WALL @ 36" C/C		
9	POST BRACING	1/2"Ø THREADED STEEL RODS @ 12'-0" LENGTHS HDG		
10	roof sheeting	29 GA CORRUGATED SHEET METAL TUFF RIB		
11	WALL SHEETING	29 GA CORRUGATED SHEET METAL DIAMOND RIB		
12	INT. WALL SHEETING	N/A		

BASIC LOAD DEFINITIONS FOR ENDWALL FRAMES

COLLATERAL - MECHANICAL, ELECTRICAL, CEILINGS, SPRINKLER, ETC.

ROOF LIVE - MINIMUM ROOF LIVE LOAD (NOT COMBINED WITH SNOW LOAD).

CRANE – LOADS INDUCED FROM CRANES IN THE BUILDING.

WIND_RIGHT1 - WIND FROM THE RIGHT TO LEFT COMBINED WITH POSITIVE INTERNAL PRESSURE (RAFTER).

WIND_P - WIND PRESSURE(NORMAL (INWARD) TO ENDWALL).

WIND_LONG1 - WIND FROM NORMAL TO THE RIGID FRAME COMBINED WITH POSITIVE INTERNAL PRESSURE.

SEISMIC LEFT - SEISMIC FORCE IN DIRECTION FROM LEFT TO RIGHT.

SEISMIC_LONG - SEISMIC FORCE NORMAL TO THE RIGID FRAME.

FIUND SL R - GRAVITY LOADS DUE TO PATTERNED SNOW LOAD FOR RAFTER DESIGN FIPAT_SL - GRAVITY LOADS DUE TO UNBALANCED SNOW LOAD.

DEAD - SELFWEIGHT OF THE BUILDING SYSTEM.

FLOOR LIVE - LOADS INDUCED DUE TO INTENDED USE AND OCCUPANCY.

SNOW – GRAVITY LOADS DUE TO SNOW WIND LEFT! - WIND FROM THE LEFT TO RIGHT COMBINED WITH POSITIVE INTERNAL PRESSURE (RAFTER

WIND_LEFT2 – WIND FROM THE LEFT TO RIGHT COMBINED WITH POSITIVE INTERNAL PRESSURE (WITH BRACING). WIND_RIGHT2 - WIND FROM THE RIGHT TO LEFT COMBINED WITH POSITIVE INTERNAL PRESSURE (WITH BRACING)

WIND_S - WIND SUCTION (NORMAL (OUTWARD) TO ENDWALL). WIND_LONG2 – WIND DIRECTION REVERSED FROM LNWIND_1 COMBINDED WITH POSITIVE INTERNAL PRESSURE.

SEISMIC RIGHT - SEISMIC FORCE IN FREIGHT FROM RIGHT TO LEFT. FIUNB_SL_L - GRAVITY LOADS DUE TO PATTERNED SNOW LOAD FOR RAFTER DESIGN.

BOLT TIGHTENING

AGENCIES AS REQUIRED.

DESIGN RESPONSIBILITY

FOUNDATION DESIGN

SERVICE ABILITY

THIS STANDARD.

SITE INSPECTIONS

EXISTING BUILDINGS

FIRE CODE COMPLIANCE

PRELIMINARY DRAWINGS

CDN BUILDINGS ENGINEER IS THE DESIGN PROFESSIONAL OR ENGINEER OF RECORD FOR THE

COMPONENT OR MATERIAL NOT SOLD BY IT, OR THEIR INTEREACE AND CONNECTION WITH THE

STEEL BUILDINGS SYSTEM, UNLESS SUCH DESIGN RESPONSIBILITY IS SPECIFICALLY REQUIRED BY THE

COMPONENTS SUPPLIED BY IT ARE DESIGNED IN ACCORDANCE WITH THE APPLICABLE BUILDING

(OF THE STRUCTURE) WHETHER DESIGN-BUILDER, ARCHITECT AND/OR PROFESSIONAL ENGINEER OF

GOVERN DESIGN, ALL DESIGN LOADS SUCH AS SNOW LOADS (INCLUDING COEFFICIENTS AND DRIFT CONDITIONS), WIND LOADS, COLLATERAL LOADS, SITE CONDITIONS FOR SEISMIC DESIGN, AND ANY

OTHER SUPERIMPOSED LOADS WHICH THE STRUCTURE IS REQUIRED TO SUSTAIN. IT IS A VIOLATION OF THE LAW FOR ANY PERSON, UNLESS HE/SHE IS ACTING UNDER THE DIRECTION OF A LICENSED

PROFESSIONAL ENGINEER, TO ALTER ANY ITEM ON THESE PLANS IN ANY WAY. IF ANY ITEM ON THESE PLANS IS ALTERED, THE ALERTING ENGINEER MUST AFFIX TO THE ITEM HIS/HER SEAL AND THE

NOTATION "ALTERED BY" FOLLOWED BY HIS/HER SIGNATURE AND THE DATE OF SUCH ALTERNATION,

AND A SPECIFIC DESCRIPTION OF THE ALTERATION. CDN BUILDINGS ASSUMES THAT ALL WINDOWS

AND DOORS WILL BE DESIGNED TO WITHSTAND THE WIND LOADS SHOWN AND WILL REMAIN CLOSED DURING PERIOD OF SEVERE WINDS (THIS DOES NOT APPLY TO BUILDINGS DESIGNED AS

FOR FURTHER CLARIFICATION OF DESIGN RESPONSIBILITY, REFER TO CSSBI B8 - 06 - BUILDINGS

CDN BUILDINGS IS RESPONSIBLE FOR THE DESIGN, MATERIALS, AND WORKMANSHIP OF THE

FOUNDATION. ANCHOR BOLT PLANS ARE PREPARED BY CDN BUILDINGS ARE INTENDED TO SHOW

ONLY LOCATION, DIAMETER, AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE STEEL BUILDING SYSTEM TO THE FOUNDATION. IT IS THE RESPONSIBILITY OF THE END CUSTOMER AND/OR THEIR DESIGN PROFESSIONAL TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR

SPECIFYING BOLT EMBEDMENT, BEARING ANGLES, TIE RODS AND/OR OTHER ASSOCIATED ITEMS

EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS

CDN BUILDINGS DOES NOT SPECIFY GROUT REQUIREMENTS - THE IS THE RESPONSIBILITY OF THE

UNLESS OTHERWISE SPECIFIED IN THE CONTRACT DOCUMENTS, CDN BUILDINGS USES INDUSTRY

STANDARD DEFLECTION LIMITS AS SPECIFIED IN CSSI B15B - 15. IN GENERAL, WE DO NOT USE THE

CDN BUILDINGS DOES PERFORM GENERAL REVIEW OF CONSTRUCTION (SITE INSPECTIONS) FOR

SCOPE OF WORK, UNLESS SPECIFIED IN THE CONTRACT DOCUMENTS FOR AN AGREED FEE.

CDN BUILDINGS DOES NOT INVESTIGATE THE INFLUENCE OF THE STEEL BUILDING SYSTEM ON EXISTING BUILDINGS OR STRUCTURES. THE END CUSTOMER AND/OR THEIR DESIGN PROFESSIONAL

MUST ENSURE CONDITIONS AS A RESULT OF THE PRESENCE OF THE STEEL BUILDING SYSTEM.

MEZZANINES, BLOCK WALLS, OR ANY OTHER COMPONENTS BY OTHERS THAT ARE IDENTIFIED AS

LOADS TO THE CDN BUILDINGS STRUCTURE. CDN BUILDINGS ACCEPTS NO RESPONSIBILITY FOR THE

IT IS THE RESPONSIBILITY OF THE PROJECT DESIGN PROFESSIONAL AND BUILDER TO COMPLY WITH

LOCAL FIRE CODE REGULATIONS INCLUDING CONSIDERATION OF, BUT NOT LIMITED TO, BUILDING

USE AND OCCUPANCY, ALL BUILDING CONSTRUCTION MATERIALS, SEPARATION REQUIREMENTS, EGRESS REQUIREMENTS, FIRE PROTECTION SYSTEMS, ETC. THE BUILDER SHALL ADVISE CDN

CDN BUILDINGS ISSUES PRELIMINARY DRAWINGS MARKED AS 'ISSUED FOR INFORMATION' FOR

DRAWINGS/DOCUMENTS PROVIDED BY OTHER FIRMS, AND ALSO FROM PERVIOUS CDN BUILDINGS

BUILDINGS ENGINEER SHALL TAKE PRECEDENCE OVER ANY PREVIOUS DRAWINGS/DOCUMENTS. THE

STAMP IS PRESENT, PLEASE SIGN AND DATE EACH DRAWING, AND CLEARLY INDICATE ANY CHANGES

CUSTOMER SHALL PERFORM A THOROUGH REVIEW OF ALL ITEMS SHOWN IN EACH DRAWING SET

APPROVAL IS REQUIRED IN ORDER TO PROCEED WITH MANUFACTURING. WHEN THE APPROVAL

REQUIRED. FAILURE TO DO SO IN A TIMELY MANNER MAY RESULT IN PROJECT DELAYS. NOTE THAT

CONSTITUTES ACCEPTANCE OF OUR INTERPRETATION, AND FURTHER CONSTITUTES AGREEMENT THAT

THE BUILDING AS SHOWN REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED. ANY CHANGE

BUILDER/CUSTOMER MUST SECURE ALL REQUIRE APPROVALS AND PERMITS FROM THE APPROPRIATE

ACKNOWLEDGED AND AGREED TO IN WRITING, APPROVAL OF CDN BUILDINGS DRAWINGS

REQUESTS THAT OCCUR AFTER APPROVAL MAY RESULT IN ADDITIONAL COSTS AND DELAYS.

EACH PROJECT. INFORMATION PRESENTED ON PRELIMINARY DRAWINGS MAY DIFFER FROM

MANUFACTURING ABILITY. THE MOST RECENT SET OF DRAWINGS THAT IS SEALED BY A CDN

BUILDINGS OF ANY SPECIAL REQUIREMENTS TO BE FURNISHED BY CDN BUILDINGS.

REQUIREMENTS, OR NECESSARY PROVISIONS FOR STRUCTURAL PERFORMANCE AND

RECEIVED, IN ORDER TO CONFIRM ADHERENCE TO THE CONTRACT REQUIREMENTS.

CHANGES REQUESTED ON THE DRAWINGS ARE NOT BINDING UNLESS SUBSEQUENTLY

INDEPENDENT OR SELE-SUPPORTING, MUST BE DESIGNED BY A PROFESSIONAL ENGINEER. THE ENGINEER MUST ENSURE THAT PROPER ISOLATION FROM THE CDN BUILDING HAS BEEN PROVIDED TO AVOID STRUCTURAL DAMAGE DUE TO DIFFERENTIAL MOVEMENTS, OR INADVERTENTLY APPLYING

COMPONENTS SUPPLIED BY IT. THIS RESPONSIBILITY IS EXPLICITLY EXCLUDED FROM CDN BUILDINGS

RECOMMENDED LIMIT SPECIFIED IN ANNEX D OF CSA \$16, WHICH IS A NON - MANDATORY PART OF

FOUNDATION DESIGNER. THE CHAR PROVIDED WITH THE ANCHOR PLANS/DETAILS IS INTENDED TO

DEMONSTRATE THAT GROUT SHALL BE TAKEN INTO ACCOUNT WHEN DETERMINING ANCHOR BOLT PROJECTION, IT DOES NOT CONSTITUTE THE SPECIFICATION OF GROUT BY THE CDN ENGINEER.

IMPOSED BY THE STEEL BUILDING SYSTEM, OTHER IMPOSED LOADS, AND THE BEARING CAPACITY OF

INCORPORATING **Steel building systems**: responsibilities of parties involved.

THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

INDEPENDENT / SELF - SUPPORTING COMPONENTS

DESIGN OF ANY INDEPENDENT/SELF - SUPPORTING COMPONENTS.

RECORD, IS RESPONSIBLE FOR SPECIFYING TO CDN BUILDINGS THE CODE AND STANDARDS TO

CODES OR OTHER CRITERIA, ALL AS SPECIFIED BY THE OWNER, THE PROFESSIONAL ENGINEER AND/OR ARCHITECT OF RECORD RETAINED BY THE OWNER, OR THE DESIGN- BUILDER. THE DESIGNER

CONSTRUCTION PROJECT. CDN BUILDINGS IS NOT RESPONSIBLE FOR THE DESIGN OF ANY

CONTRACT DOCUMENTS. CDN BUILDINGS IS ONLY RESPONSIBLE FOR ENSURING THAT THE

PROPER INSTALLATION AND INSPECTION OF ALL FASTENERS IS THE RESPONSIBILITY OF THE ERECTOR. ALL HIGH STRENGTH (ASTM A123, GALV. GRADE 5) BOLTS AND NUTS MUST BE TIGHTENED BY THE 'TURN – OF – NUT' METHOD AS SPECIFIED IN THE 'INSTALLATION AND INSPECTION OF BOLTED JOINTS' CLAUSE OF CSA \$16. INSPECTION OF HIGH STRENGTH BOLTED JOINTS BY OTHER THAN THE ERECTOR MUST BE SPECIFIED IN THE CONTRACT DOCUMENTS, AND THE ERECTOR IS RESPONSIBLE FOR ENSURING THAT INSTALLATION AND INSPECTION PROCEDURES ARE COMPATIBLE PRIOR TO THE START OF THE ERECTION.

TABLE 8: NUT ROTATION FROM SNUG-TIGHT CONDITION	
BOLT LENGTH** UP TO AND INCLUDING 4 BOLT DIAMETERS	TURN 1/3
OVER 4 DIAMETERS AND NOT EXCEEDING 8 DIAMETERS OR 8 INCHES	1/2
EXCEEDING 8 DIAMETERS OR 8 INCHES ** BOLT LENGTH IS MEASURED FROM THE UNDERSIDE OF THE HEAD TO THE EXTREME END OF POINT	2/3

<u>ERECTION – GENERAL</u>

CDN BUILDINGS IS RESPONSIBLE FOR THE ERECTION OF THE STEEL BUILDING SYSTEM, THE SUPPLY OF ANY TOOLS OR EQUIPMENT, SUPERVISION FOR THE ERECTION OF THE STRUCTURE, OR ANY OTHER FIELD WORK. FIELD ERECTION OF A STEEL BUILDING, AS IN ALL CONSTRUCTION PROJECTS, INVOLVES HAZARDS TO PERSONS WITHIN THE AREA OF THE CONSTRUCTION AND RISK OF DAMAGE TO THE PROPERTY ITSELF. CDN BUILDINGS DOES NOT FURNISH A GENERAL ERECTION MANUAL, HOWEVER FIELD ERECTION PROCEDURES CAN VARY BECAUSE OF MANY ITEMS INCLUDING LOCAL CONDITIONS, EQUIPMENT AVAILABILITY, THE TYPE OF BUILDING BEING ERECTED AND THE EXPERTISE OF THE PARTICULAR ERECTOR. THE ERECTOR, BY ENTERING INTO A CONTRACT TO ERECT THE BUILDING, HOLD ITSELF OUT AS SKILLED IN THE ERECTION OF STEEL BUILDING SYSTEMS, AND IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE MUNICIPAL, PROVINCIAL, AND FEDERAL CONSTRUCTION AND SAFETY REGULATIONS AS WELL AS ANY APPLICABLE REQUIREMENTS MUNICIPAL, PROVINCIAL, FEDERAL, AND INTERNATIONAL UNION RULES OR PRACTICES. THE ERECTION DRAWINGS FURNISHED BY CDN BUILDINGS ARE NOT INTENDED TO SPECIFY ANY PARTICULAR METHOD OF ERECTION TO BE FOLLOWED BY THE ERECTOR. THE ERECTOR IS ALSO RESPONSIBLE FOR SUPPLYING ANY SAFETY DEVICES SUCH AS FALL ARREST SYSTEMS, MAN-LIFTS, AND ANCHOR POINTS ETC. WHICH MAY BE REQUIRED TO SAFELY ERECT THE STEEL BUILDING SYSTEM. CDN BUILDINGS EXPRESSLY DISCLAIMS ANY RESPONSIBILITY FOR ANY INJURY TO PERSONS IN THE COURSE OF ERECTION OR DAMAGE TO THE PRODUCT ITSELF. ONLY EXPERIENCED PERSONS WHO ARE SKILLED AND QUALIFIED IN THE ERECTION OF STEEL BUILDINGS SHOULD BE PERMITTED TO FIELD -ERECT A BUILDING DUE TO THE HAZARDS OF THIS CONSTRUCTION ACTIVITY. ALL ERECTION EQUIPMENT AND DETAILED ERECTING PROCEDURES WILL BE DETERMINED BY AND INDEPENDENT QUALIFIED PROFESSIONAL ENGINEER RETAINED BY THE BUILDER AS REQUIRED.

ERECTION TOLERANCES

ERECTION TOLERANCES ARE THOSE SET FORTH IN THE "DESIGN OF STEEL STRUCTURES" (CSA S16 LATEST EDITION)

TEMPORARY BRACING DURING CONSTRUCTION

THE ERECTOR SHALL FURNISH TEMPORARY GUYS AND BRACING WHERE NEEDED FOR SQUARING, PLUMBING, AND SECURING THE STRUCTURAL FRAMING AGAINST LOADS, SUCH AS WIND LOADS ACTING ON THE EXPOSED FRAMING, AS WELL AS LOADS DUE TO ERECTION EQUIPMENT AND OPERATION. THESE CONSTRUCTION LOADS CAN BE SIGNIFICANTLY HIGHER THAN LOADS WHICH WILL BE APPLIED ONCE THE BUILDING IS COMPLETED ERECTED, AND ACCORDINGLY, BRACING FURNISHED BY CDN BUILDINGS FOR THE STEEL BUILDING SYSTEM CANNOT BE ASSUMED TO BE ADEQUATE DURING ERECTION, COLUMN BASE PLATES ARE TYPICALLY 'PIN' CONNECTIONS, AND IT IS THEREFORE EXTREMELY DANGEROUS TO LEAVE ANY COLUMN AS 'FREE STANDING' (NO LATERAL SUPPORT AT THE TOP) FOR ANY LENGTH OF TIME, SPECIAL CARE MUST BE TAKEN WHEN COLUMNS ARE GROUTED, AS THEY TEND TO BE UNSTABLE UNTIL THE GROUT IS IN PLACE. TEMPORARY SUPPORTS SUCH AS TEMPORARY GUYS, BRACING, FALSEWORK, CRIBBLING OR OTHER ELEMENTS REQUIRE FOR THE ERECTIONS OPERATION SHALL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR.

FABRICATION/DRAWING ERRORS

THE BUILDER/CUSTOMER IS RESPONSIBLE FOR CONTACTING CDN BUILDINGS MANAGEMENT TEAM TO ADVISE CDN BUILDINGS OF FABRICATION/DRAWING PROBLEMS AND CORRESPONDING FIELD CORRECTION COST ESTIMATES. CDN BUILDINGS WILL THEN BE RESPONSIBLE FOR PROVIDING THE BUILDER WITH WRITTEN APPROVAL TO PROCEED WITH APPROPRIATE FIELD CORRECTIONS. THIS WILL BE DONE IN A TIMELY MANNER. NOTE: IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT CDN BUILDINGS APPROVAL, THEY ARE DOING SO AT THEIR OWN RISK AND COST. CDN BUILDINGS WILL ONLY BE RESPONSIBLE FOR CLAIMS WHERE THE BUILDER/CUSTOMER DOCUMENTS THE PROBLEM, IT'S CORRECTION, AND REASONABLE COSTS FOR THE REPAIR AND SUBMITS SAME FOR PAYMENT WITHIN 15 DAYS OF THE OCCURRENCE.

DRAWING DISCREPANCIES

IN THE CASE OF DISCREPANCIES BETWEEN CDN BUILDINGS DRAWINGS AND DETAILS VERSUS THE PLANS FOR OTHER TRADES, THE CDN BUILDINGS PLANS GOVERN (CISC CODE OF STANDARD

CORRECTION OF ERRORS AND REPAIRS

THE CORRECTION OF MINOR MISALIGNMENTS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, SHIMMING, MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. (CISC CODE OF STANDARD PRACTICE)

ALL FIELD WELDING SHALL BE DONE AT THE DIRECTION OF A DESIGN PROFESSIONAL AND DONE IN ACCORDANCE WITH CWB REQUIREMENTS BY WELDERS QUALIFIED TO PERFORM THE APPLICABLE WELDING PROCEDURE. USE MINIMUM 70ks1 ELECTRODES. FIELD INSPECTION IS NOT BY CDN

DELIVERIES AND SHORTAGES

- A. UNLESS OTHERWISE SPECIFIED, TERMS SHALL BE F.O.B. CDN BUILDINGS POINT OF MANUFACTURE. IF TRANSPORTATION CHARGES ARE INCORPORATED IN THE PRICE QUOTED, SUCH CHARGES ARE FREIGHT PREPAID UNLESS OTHERWISE SPECIFIED. DELIVERY SHALL BE DEEMED TO OCCUR WHEN THE GOODS ARE SHIPPED FROM THE POINT OF
- MANUFACTURE C. CDN BUILDINGS IS AUTHORIZED TO SHIP INSTALLMENTS. IF FOR ANY REASON THE DELIVERY IS
- DELAYED BY THE PURCHASER, STORAGE OR DEMURRAGE COSTS WILL BE BORNE BY THE PURCHASER.
- D. DELIVERY DATES ARE APPROXIMATE ONLY, AND ANY ESTIMATED DELIVERY SCHEDULE, IF STATED, SHALL BE DETERMINED FROM THE DATE CDN BUILDINGS RECEIVED COMPLETE AND SATISFACTORY INFORMATION NECESSARY TO MANUFACTURE THE GOODS. CDN BUILDINGS SHALL NOT, HOWEVER, BE LIABLE FOR ANY DELAY IN DELIVERY CAUSED BY CONDITIONS BEYOND CDN BUILDINGS REASONABLE CONTROL. IN ADDITION, THERE MAY BE DELAYS IN DELIVERY OF GOODS NOT NORMALLY CARRIED BY CDN BUILDINGS. ANY SUCH CAUSES OF DELAY EVEN THOUGH EXISTING ON THE DATE OF ORDER OR ON THE DATE OF STARTING OF MANUFACTURE SHALL EXTEND THE TIME OF CDN BUILDINGS PERFORMANCE BY THE LENGTH OF DELAYS OCCASIONED THEREBY, INCLUDING DELAYS REASONABLY INCIDENT TO THE RESUMPTION OF NORMAL PROCEDURES.
- UNDER NO CIRCUMSTANCES SHALL CDN BUILDINGS BE LIABLE FOR ANY INDIRECT OF CONSEQUENTIAL DAMAGES WHATSOEVER
- F. CDN BUILDINGS WILL ENDEAVOR TO DELIVER THE GOODS ON THE REQUIRED DELIVERY DATE. THE CDN BUILDINGS TRUCK IS NOT CONSIDERED LATE IF DELIVERIES ARE WITHIN ONE HOUR OF THE SCHEDULED DELIVERY DATE, IF THE SHIPMENT IS BY CONTRACT CARRIER (FOB DESTINATION) IT IS THE RESPONSIBILITY OF THE CUSTOMER TO FILE CLAIMS WITH THE CARRIER. CDN BUILDINGS CANNOT ASSUME AND LIABILITY FOR THE CLAIM.
- G. IN THE EVENT THAT ANY SERVICES ARE TO BE PERFORMED BY CDN BUILDINGS SHALL BE DELAYED BY MORE THAN THIRTY (30) DAYS ON ACCOUNT FACTORS BEYOND CDN BUILDINGS CONTROL, THEN UPON NOTICE FROM CDN BUILDINGS, PURCHASER SHALL PAY CDN BUILDINGS THE COST OF ALL WORK, SERVICES AND MATERIALS TO THE DATE OF SUCH NOTICE. H. THE PURCHASER SHALL BE RESPONSIBLE TO INSPECT ALL GOODS RECEIVED PURSUANT HERETO.
- IF THE PURCHASER IS OF THE VIEW THAT PART OF THE GOODS ARE DAMAGED PREVENTING USE OF SAME, SUCH GOODS SHALL BE NOTED AS DAMAGED AND THE CDN BUILDINGS PROJECT MANAGEMENT TEAM SHALL BE NOTIFIED IMMEDIATELY TO DETERMINE WHAT RECTIFICATION IN CDN BUILDINGS SOLE ABSOLUTE DISCRETION IS NECESSARY. CDN BUILDINGS SHALL NOT ACCEPT CLAIMS FOR DAMAGE AFTER LOADING AT THE CDN BUILDINGS PLANT OR CAUSED DURING TRANSIT, UNLOADING OR HANDLING AT THE JOBSITE, FURTHER, CDN BUILDINGS SHALL NOT ACCEPT CLAIMS FOR VIABLE SHORTAGES UNLESS CDN BUILDINGS IS NOTIFIED WITH IN 48 HOURS OF TAKING DELIVERY AND ONE COPY OF THE PACKING LIST. WITH SHORTAGES PROPERLY NOTED, IS RETURNED TO CDN BUILDINGS WITHIN THE SAID 48 HOURS. CDN BUILDINGS SHALL NOT ACCEPT CLAIMS FOR SHORTAGES THAT ARE NOT VISIBLE UNLESS CDN BUILDINGS WITHIN FIFTEEN (15) DAYS OF TAKING THE DELIVERY AND ONE COPY OF THE PACKING LIST, WITH STORAGES PROPERLY NOTED, IS RETURNED TO CDN BUILDINGS WITHIN THE SAID FIFTEEN (15) DAYS. CDN BUILDINGS SHALL NOT BE RESPONSIBLE FOR ANY COST, EXPENSE, OR DAMAGE OF ANY KIND ARISING FROM THE USE OD DAMAGED OR OTHERWISE DEFECTIVE GOODS. CDN BUILDINGS WILL NOT ACCEPT THE COST OD EQUIPMENT (RENTAL OR DEPRECIATION), SMALL TOOLS, SUPERVISION, OVERHEAD OR PROFIT SUBJECT TO ANY CLAIM. ANY CLAIM MUST BE APPROVED BY THE CDN BUILDINGS PROJECT MANAGEMENT TEAM.

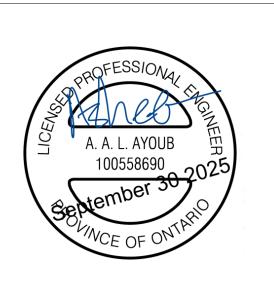
INVOICE PAYMENT

BY ACCEPTANCE OF THE MATERIALS OR SERVICES, THE BUILDER AND/OR CUSTOMER AGREES TO CDN BUILDINGS TERMS & CONDITIONS. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE. EACH CLAIM WILL BE ADJUDICATED ON ITS ON MERIT AND SETTLED ACCORDINGLY



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	REVISION TABLE	i
NO.	DESCRIPTION	DAT
1	ISSUED FOR COORDINATION	2024/07/
2	ISSUED FOR PERMITS	2024/12/
3	ISSUED FOR PERMITS	2025/03/
4	AS PER COMMENTS	2025/06/
5	AS PER COMMENTS	2025/09/
	1	
	1	
	_	

KOUTNY TONY

LUSCOMBE DRIVE, SIMCOE, ONTARIO

GENERAL NOTES

PROJECT NUMBER 24-1022 DATE 2024/07/02 **DRAWN BY** CHECKED BY

3/8" = 1'-0" SCALE

<u>LOT INFO</u>

LEGAL DESCRIPTION: WDM CON. 14, PT. LOT 4 RP37R11237, PART 1

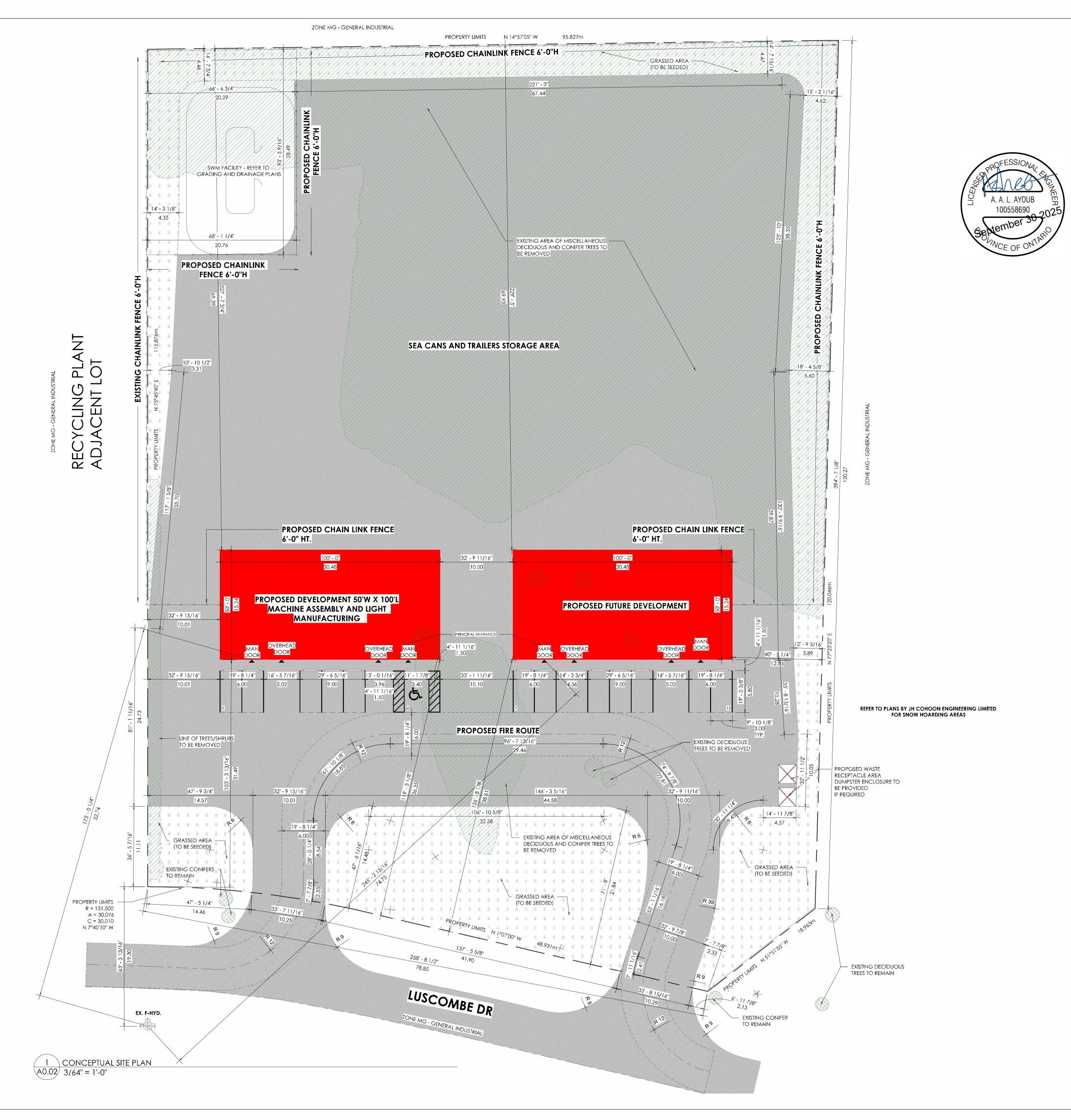
LUSCOMBE DRIVE TOWNSHIP/CITY: SIMCOE / NORFOLK COUNTY GENERAL INDUSTRIAL ZONE(MG)

ZONING: ROLL #: 40302503825 MACHINE ASSEMBLY AND LIGHT MANUFACTURING

ZONING BY-LAW NO. 0225-2007 - PART 8

	PROVISIONS (m)	PROPOSED (m)
front yard setback	30M	31.49M
rear yard setback	9M	69.39M
minimum interior side yard setback	3M	10.01
MINIMUM EXTERIOR SIDE YARD SETBACK	6M	N/A
MAXIMUM HEIGHT	9.5M	7.06M
MAXIMUM COVERAGE - PHASE-1	T.B.C	3.92%
MAXIMUM COVERAGE - ALL PHASES	T.B.C	7.84%
MIN. LANDSCAPE COVERAGE	T.B.C	19.7 %
PARKING SPACES	12 PARKING SPACES + 1 TYPE A ACCESIBLE PARKING SPACE	13 PARKING SPACES + 1 TYPE A ACCESIBLE PARKING SPACI

LOT AREA $= 127,484.99FT^2/11,843.743 M^2$ EXISTING STRUCTURE = $O FT^2 / M^2$ PROPOSED BUILDING = (PHASE 1) 5000 FT 2 / 464.5 M 2 PROPOSED BUILDING = (ALL PHASES) 10,000 FT²/ 929.03 M²





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PROJECT NORTH



REVISION TABLE			
DESCRIPTION	DATE		
ISSUED FOR COORDINATION	2024/07/23		
ISSUED FOR PERMITS	2024/12/19		
ISSUED FOR PERMITS	2025/03/21		
AS PER COMMENTS	2025/06/16		
AS PER COMMENTS	2025/09/30		

KOUTNY TONY

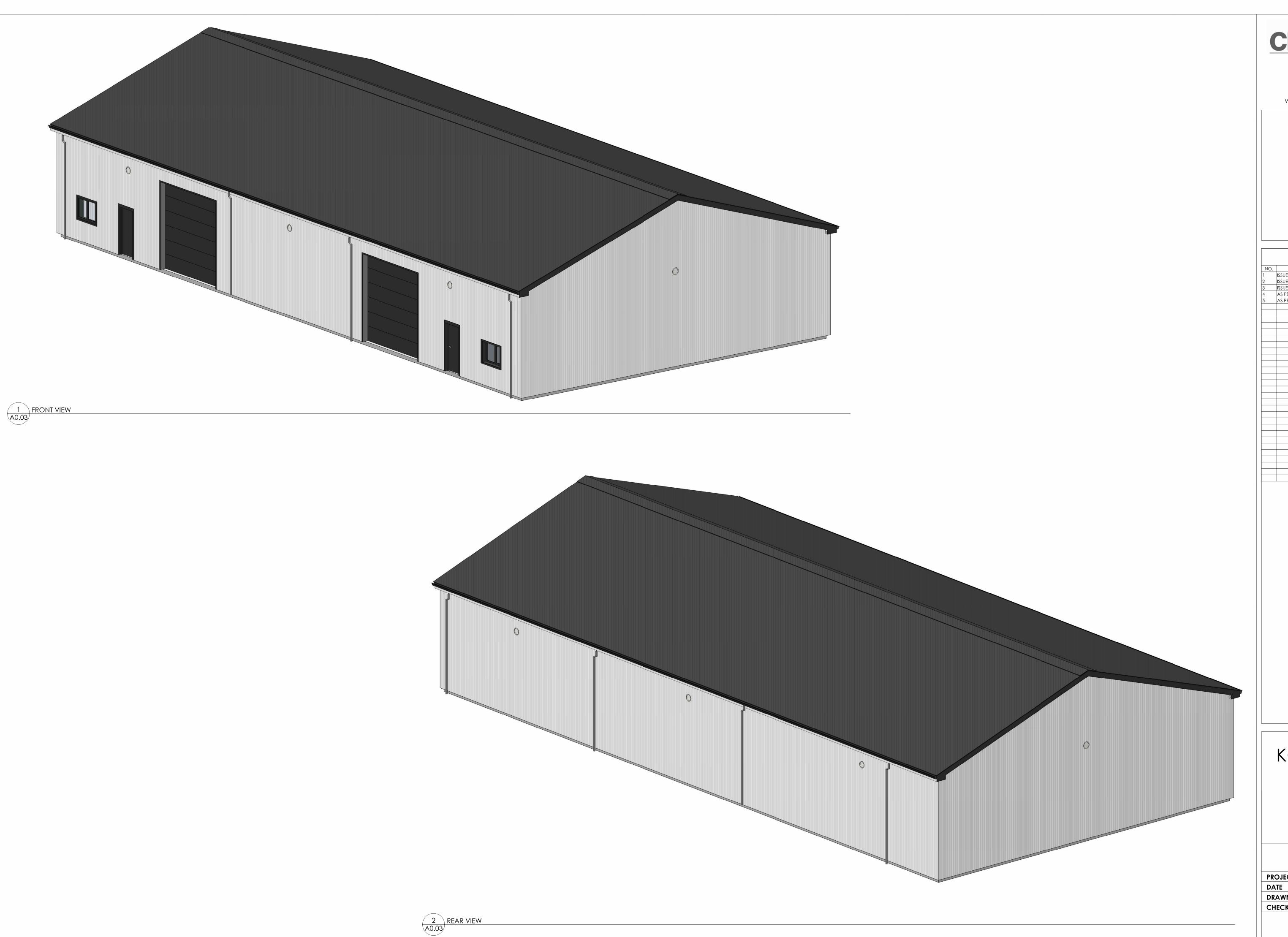
LUSCOMBE DRIVE, SIMCOE, ONTARIO

SITE PLAN

A O C	\mathbf{O}
CHECKED BY	A
DRAWN BY	WI
DATE	2024/07/0
PROJECT NUMBER	24-102

AU.02

SCALE As indicated





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NO.	DESCRIPTION	DATE
1	ISSUED FOR COORDINATION	2024/07/23
2	ISSUED FOR PERMITS	2024/12/19
3	ISSUED FOR PERMITS	2025/03/21
4	AS PER COMMENTS	2025/06/16
5	AS PER COMMENTS	2025/09/30

KOUTNY TONY

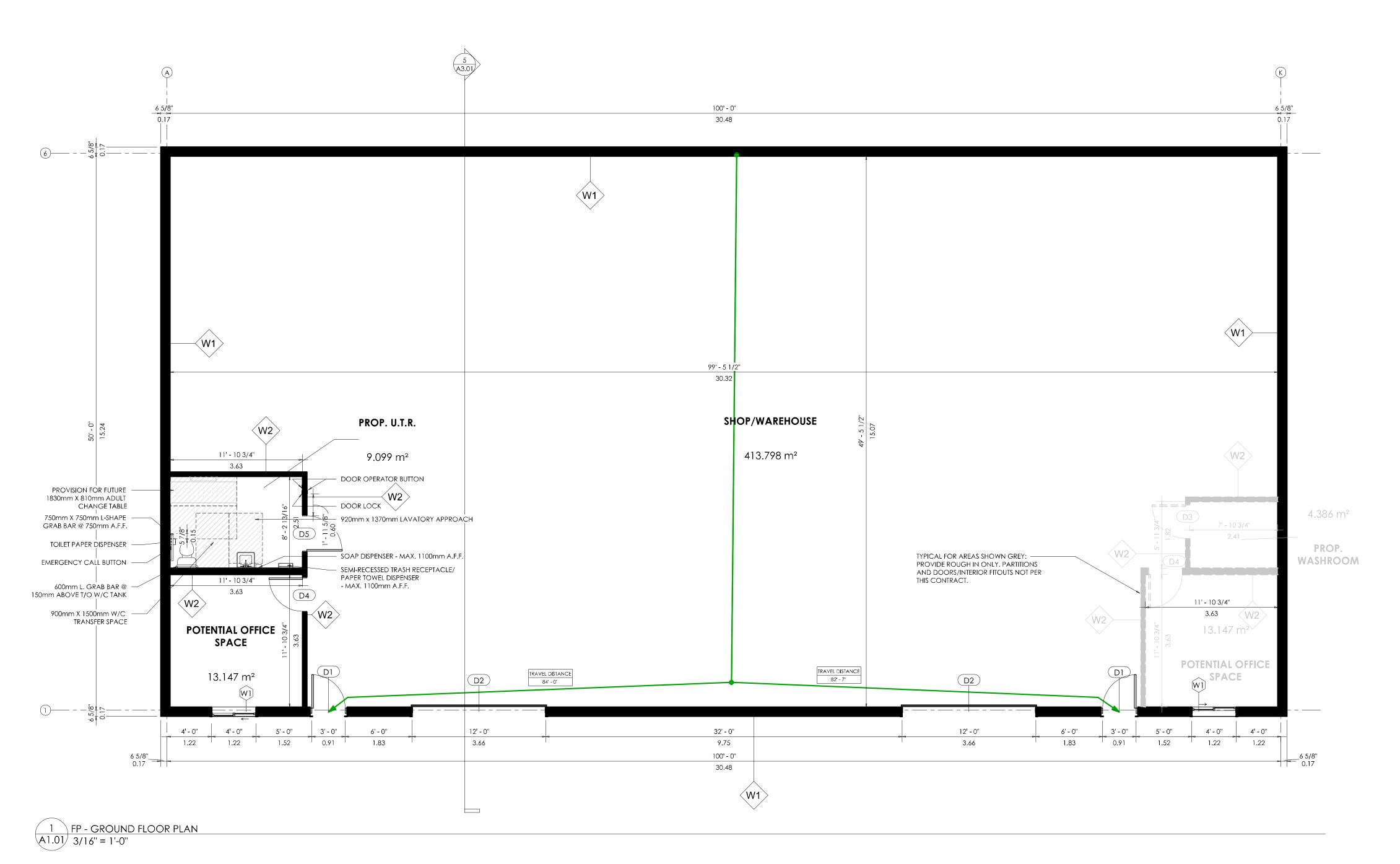
LUSCOMBE DRIVE, SIMCOE,ONTARIO

ISOMETRIC MODELS

PROJECT NUMBER	24-1022
DATE	2024/07/02
DRAWN BY	BK
CHECKED BY	AA

A0.03

SCALE



	WALL TYPE SCHEDULE	
9 3/4" 3/4" 2" 5" 5/8" 29 Ga. DIAMOND RIB STEEL SHEETING (VERTICAL) EXTERIOR STEEL SHEETING 5" C-GIRTS @ 36" C/C 1"x4" STRAPPING @ 36" C/C 2" RIGID INSULATION (R-6.3 MINIMUM) AIR BARRIER 5/8" T&G PLYWOOD SHT'G 5" SQ. HSS POSTS 4" FIBREGLASS BATT INSULATION (R-13) VAPOUR BARRIER 29 Ga. DIAMOND RIB STEEL SHEETING 5" C-GIRTS @ 36" C/C 2" RIGID INSULATION (R-6.3 MINIMUM) AIR BARRIER 5/8" T&G PLYWOOD SHT'G 5" SQ. HSS POSTS 4" FIBREGLASS BATT INSULATION (R-13) VAPOUR BARRIER 29 Ga. INTERIOR STEEL SHEETING	1/2" GYPSUM WALL BOARD 1/2" GYPSUM WALL BOARD 3 5/8" METAL STUDS @ 16" C/C 1/2" GYPSUM WALL BOARD	1/2" GYPSUM WALL BOARD 6" METAL STUDS @ 16" C/C 1/2" GYPSUM WALL BOARD
W1	W2	W3

Door Schedu		
Description	Туре	Type Mark
COMMERCIAL STEEL MAN DOOR W/ PANIC BAR	\$ 36" x 80"	D1
G-5000 INSULATED OVERHEAD DOOR W/ CHAIN FALL OPENER	12'W x 12'H	D2
INTERIOR WASHROOM DOOR W/ PRIVACY LOCK	36" x 80" (1)	D3
INTERIOR OFFICE DOOR W/ LOCKSET	36" x 80" (2)	D4
	38" x 80" (1)	D5

Window Schedule				
Description	Type	Type Mark		
INSULATED VINYL WINDOW	48" x 36"	W1		

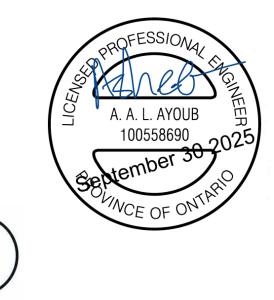
BUILDING CLASSIFICATION :- **3.2.2.71. GROUP F, DIVISION, UP TO 2 STOREYS**ROOF R VALUE REQUIRED :- R19 + 11 ci
WALL R VALUE REQUIRED :- R13 + 6.5 ci

	ONTARIO BUII	LDING	CC	DE	DAT	A M	ATR	IX P	ART-3	3	BUILDING CODE REFERENCI
3.00	BUILDING CODE VERSION: (D. REG 332	/12			LAST	AME	NDME	NT: O. RE	G. 89/23	
3.01	PROJECT TYPE	CATEG	ORY:			NEW (CONS	TRUCTI	ION		[A]1.1.2.2.
		DESCRI	OITS	<u>۷:</u>		NEW S	STEEL-	FRAME	ED SHOP/	'WAREHOUSE	<u> </u>
3.02	MAJOR OCCUPANCY CLASSIFICATION		OCCUPANCY: GROUP-F, DIVISION-2 SHOP/WAREHOUSE W/ ACCESSORY OFFICE				3.1.2.				
3.03	SUPERIMPOSED MAJOR OCCUPANCY	YES/NC	<u>):</u>			NO					3.2.2.7.
	MAJOR OCCUPANCI	DESCRI	1OIT9	<u> </u>		N/A					
3.04	BUILDING AREA (m²)	DESCRI ENVELO		<u>V:</u>	EXIS N/A	TING:		<u>IEW:</u> 80.026	_	<u>TOTAL:</u> 480.026	[A]1.4.1.2.
3.05	GROSS AREA (m²)	DESCRI	PTIOI	<u>۷:</u>	EXIS	TING:	N	IEW:	-	TOTAL:	[A]1.4.1.2.
		GROUNI	O FLO	OR	N/A		4:	53.577	4	453.577	-
3.06	MEZZANINE AREA (m²)	DESCRI	PTIOI	<u>V:</u>		TING: OT APPL		I <u>EW:</u> LE	-	TOTAL:	3.2.1.1.
3.07	BUILDING HEIGHT	STOREY	S AB	OVE (GRAD	 E:	1				[A]1.4.1.2. &
		HEIGHT				_	5.	.71m			3.2.1.1.
		STOREY					0				
3.08	HIGH BUILDING	NO NO	JULL			<u></u>					3.2.6.
3.09	NUMBER OF STREETS/										3.2.2.10 &
	FIREFIGHTER ACCESS	STREET(<u>S):</u>		1						3.2.5.
3.10	BUILDING CLASSIFICATION	3.2.2.71			GRC	OUP - F /	DIVIS	ION - 2			3.2.2.20 - 83.
3.11	SPRINKLER SYSTEM	NOT REC					3.2.1.5 & 3.2.2.1718., 3.2.4.8. TO 3.2.4.10., & 3.2.5.13				
3.12	STANDPIPE SYSTEM	NOT REC	NOT REQUIRED						3.2.9.		
3.13	FIRE ALARM SYSTEM	REQUIR	REQUIRED?: NO						3.2.4.		
		TYPE:									
3.14	WATER SUPPLY/ SERVICE IS ADEQUATE	YES	YES					3.2.5.7.			
3.15	CONSTRUCTION TYPE	RESTRIC	CTION	<u>l:</u>	CON	ABUSTIBL	E PER	MITTEE	>		3.2.2.2083. & 3.2.1.4.
		ACTUA	<u>L:</u>		ИОИ	I-COMB	USTIBI	LE			
		HEAVY	<u>TIMB</u>	ER:	NO						
3.16	IMPORTANCE CATEGORY	NORMA	L 								4.1.2.1.(3) & T4.1.2.1.B
3.17	SEISMIC HAZARD INDEX	<u>IE Fa Sc</u>	(0.2) =	<=0.	35			_		4.1.2.1.(3) & 4.1.8.18.(1).
3.18	OCCUPANT LOAD	AREA:	<u>OC</u> (C. TYF	PE B.	ASED C	<u> </u>	<u>occ</u>	. LOAD	POST	4.1.2.1.(3) & 4.1.8.18.(1).
		ALL	F-2		EI	XTURES		18		<u>LIMIT:</u> NO	
3.19	BARRIER FREE DESIGN	BARRIE		Es.	YI			10		110	3.8. &
	5, MARIEN TREE DESIGNA	ENTRAI			1						3.1.8.2.
3.20	HAZARDOUS SUBSTANCES	NO	_								3.3.1.2. & 3.3.1.19.
3.21	REQUIRED FIRE RESISTANCE RATINGS	HORIZ. ASSEMI		RAT	ING:	SUPPO ASSE			N/C II FRR	N LIEU OF	3.2.2.20-83., 3.2.1.2., 3.2.1.4., 3.2.2.15.
		FLOORS		0.75	H	0.75H			NO		0.2.2.10.
3.22	SPATIAL SEPARATION	EBF / A	REA	<u>L.[</u>	⊃.(m)	<u>L/H C</u> <u>H/L:</u>	<u>DR</u>	%OI (PER	P'G MITTED)	FRR	3.2.3.
			06m²	-	.64	7:1		1009		0	
			06m ²		.60	7:1 3.5:	1	100%	6	0	
			07m² 07m²	5.0 9.9		3.5:		25% 69%		0	
3.23	PLUMBING FIXTURE REQUIREMENTS	RATIO:	*						I NOTED	OTHERWISE	3.7.4., 3.8.2.3.
		AREA:	<u>OC</u>	C AD:	OBC	REF:	WC REG	<u>''s</u> QUIRED	WC AC	<u>C's</u> TUAL	
		ALL	18		3.7.4.	0	2	. UIIXEL	2		-



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	OR COORDINATION	2024/07/2
	OR PERMITS	2024/07/2
	OR PERMITS	2025/03/2
	COMMENTS	2025/06/1
	COMMENTS	2025/09/3
ASTERV	SOMIMENTS	2020/07/0

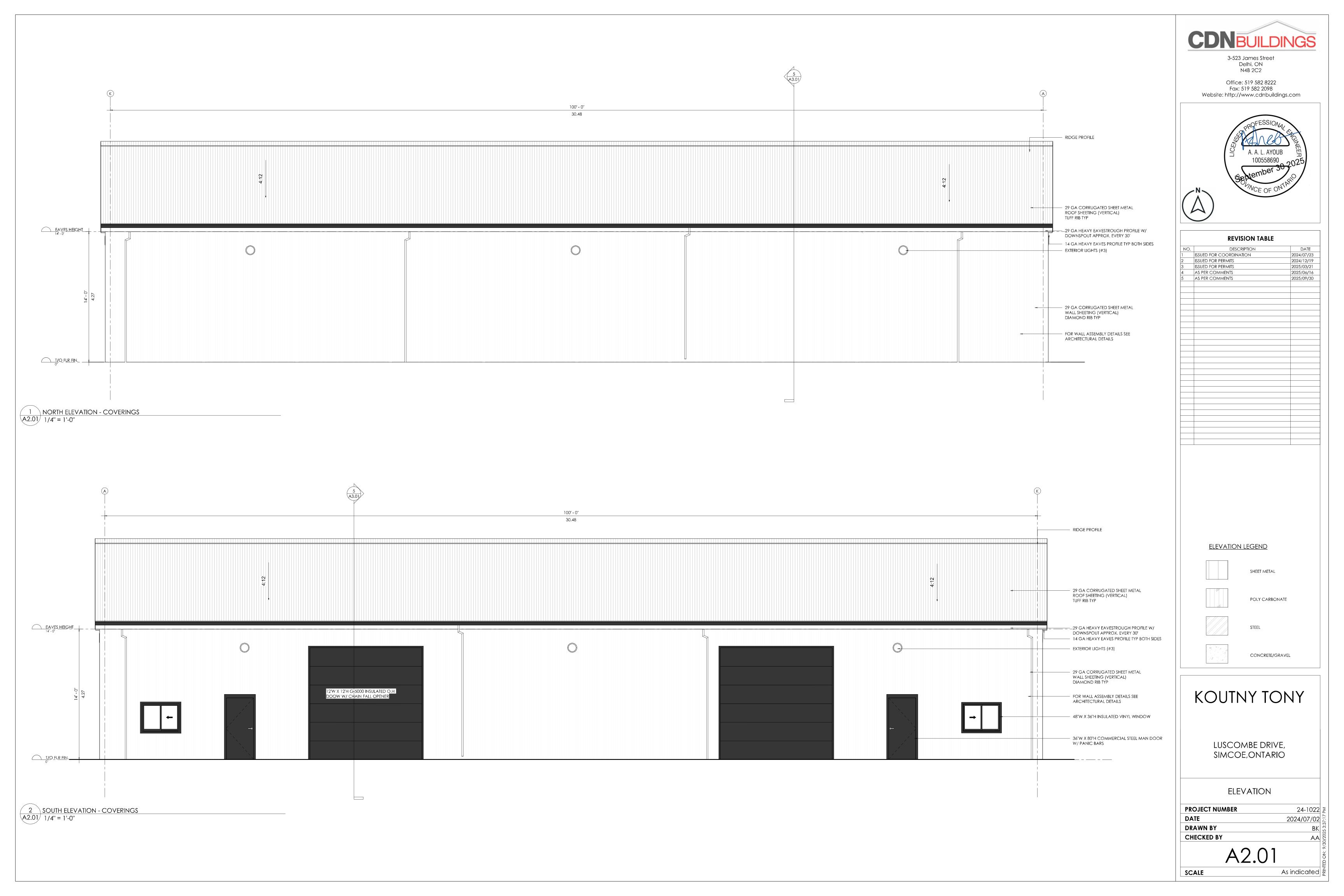
KOUTNY TONY

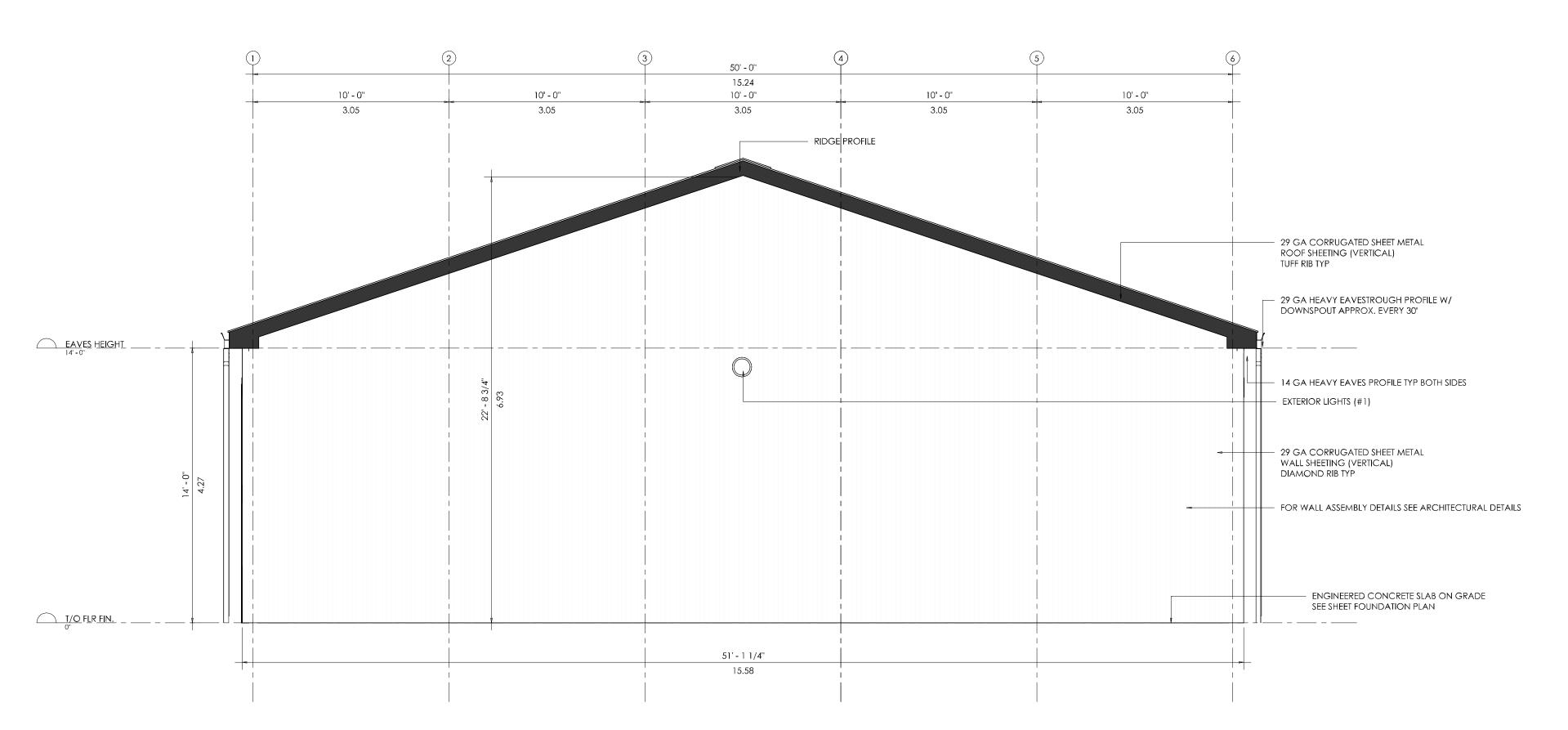
LUSCOMBE DRIVE, SIMCOE,ONTARIO

FLOOR PLAN

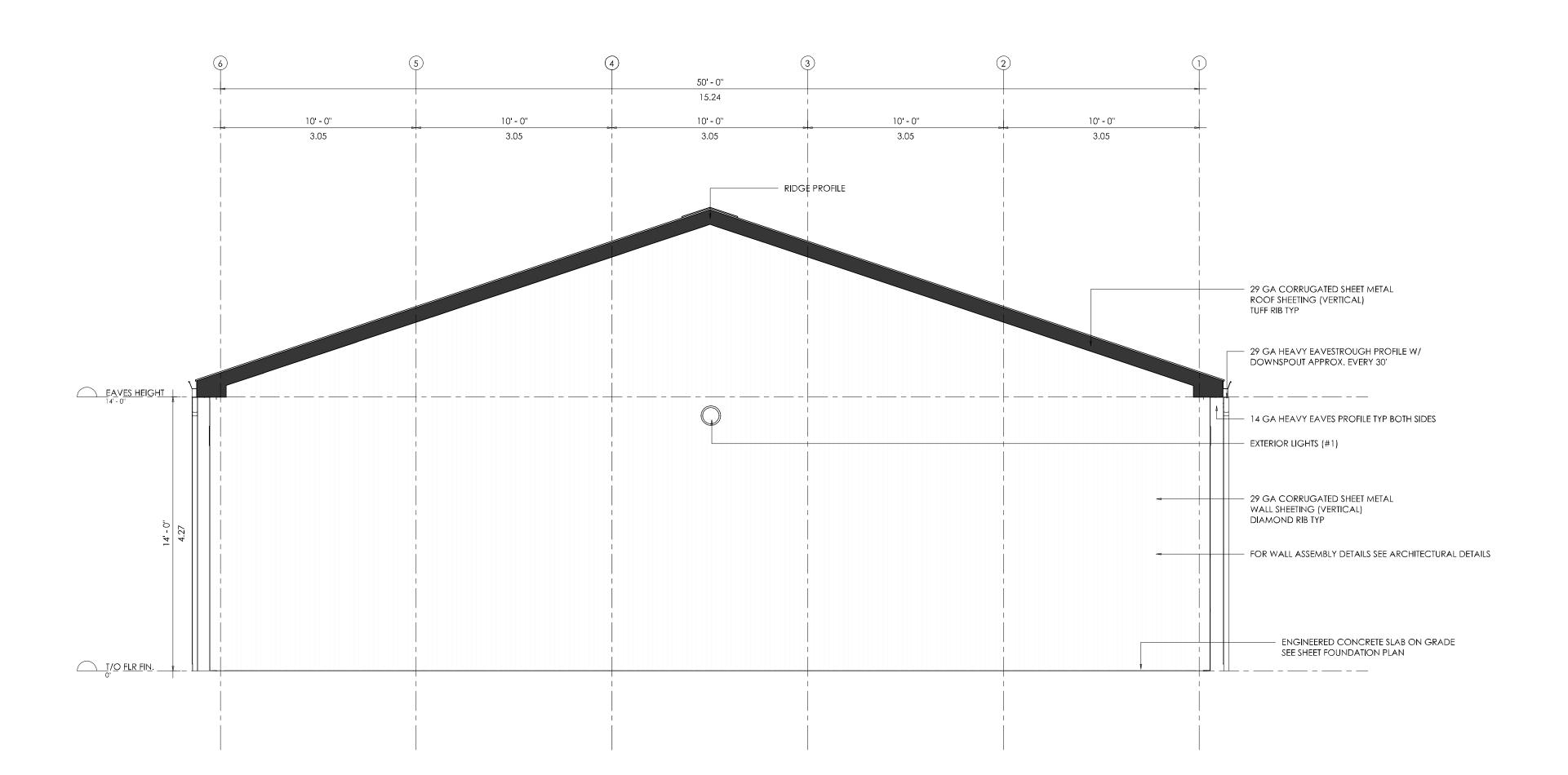
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SCALE As indicated





1 EAST ELEVATION - COVERINGS 1/4" = 1'-0"

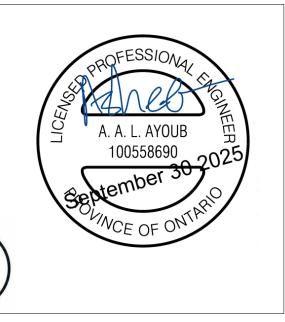


2 WEST ELEVATION - COVERINGS A2.02 1/4" = 1'-0"



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1	ISSUED FOR COORDINA	ATION	2024/07
2	ISSUED FOR PERMITS		2024/12
3	ISSUED FOR PERMITS		2025/03
4 5	AS PER COMMENTS AS PER COMMENTS		2025/06
5	AS PER COMMENTS		2025/09
	ELEVATION	N LEGEND	
		SHEET METAL	
		POLY CARBONATE	
		STEEL	
	4	CONCRETE/GRAVE	L

KOUTNY TONY

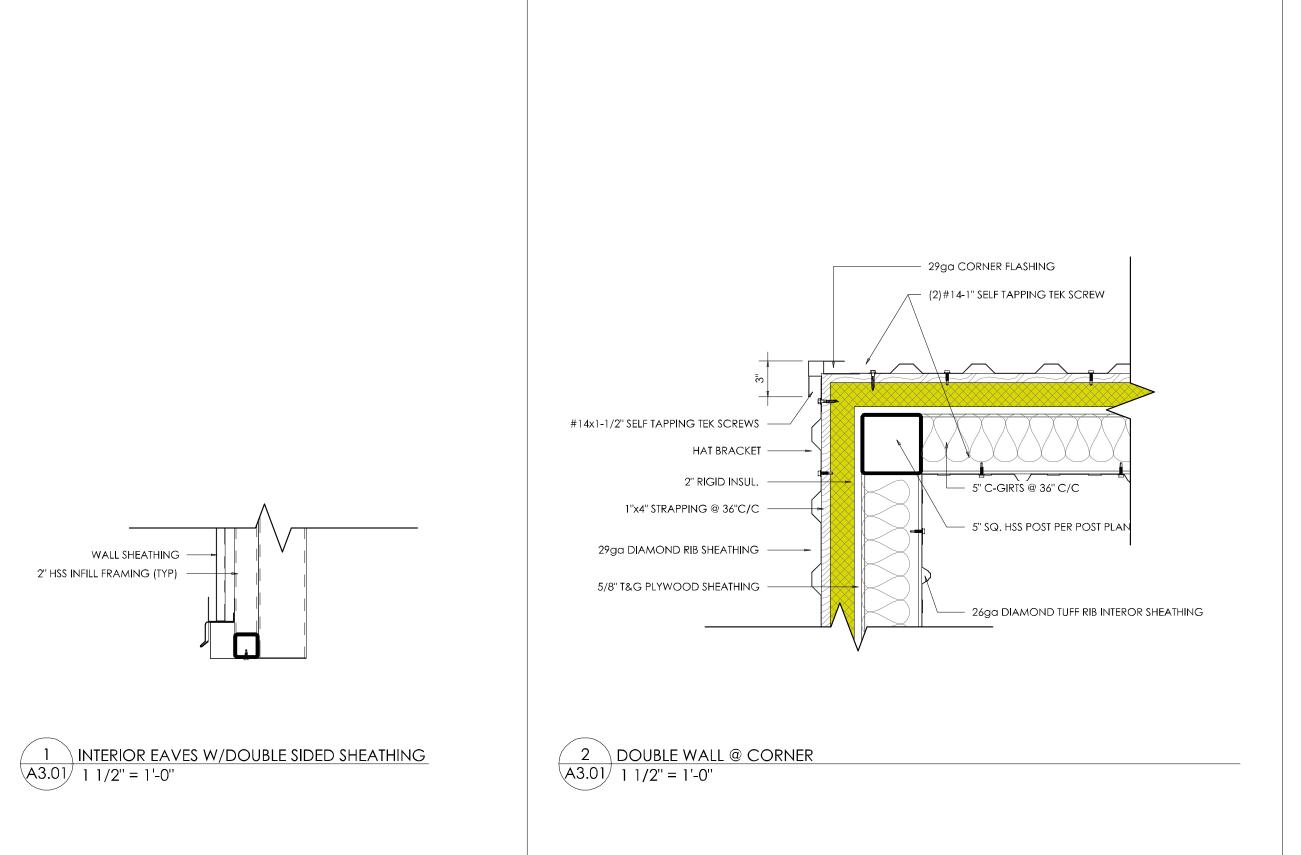
LUSCOMBE DRIVE, SIMCOE,ONTARIO

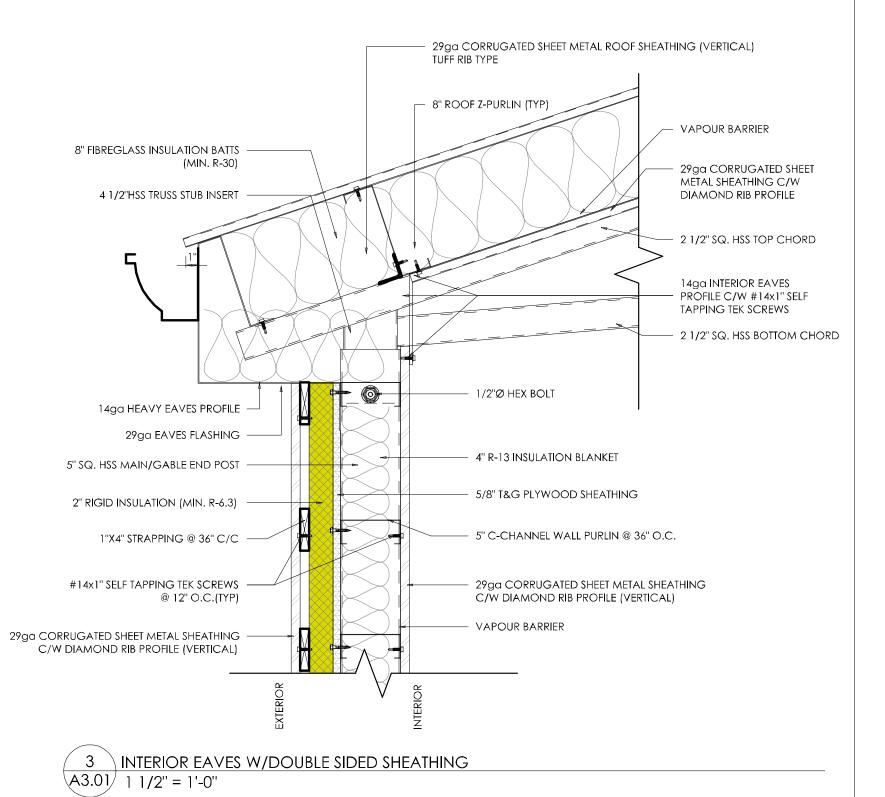
ELEVATION

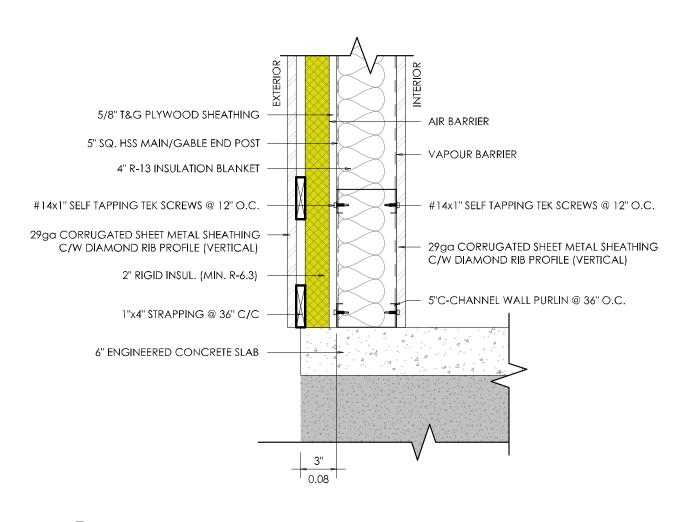
PROJECT NUMBER	24-1022
DATE	2024/07/02
DRAWN BY	Bk
CHECKED BY	AA

A2.02

SCALE As indicated







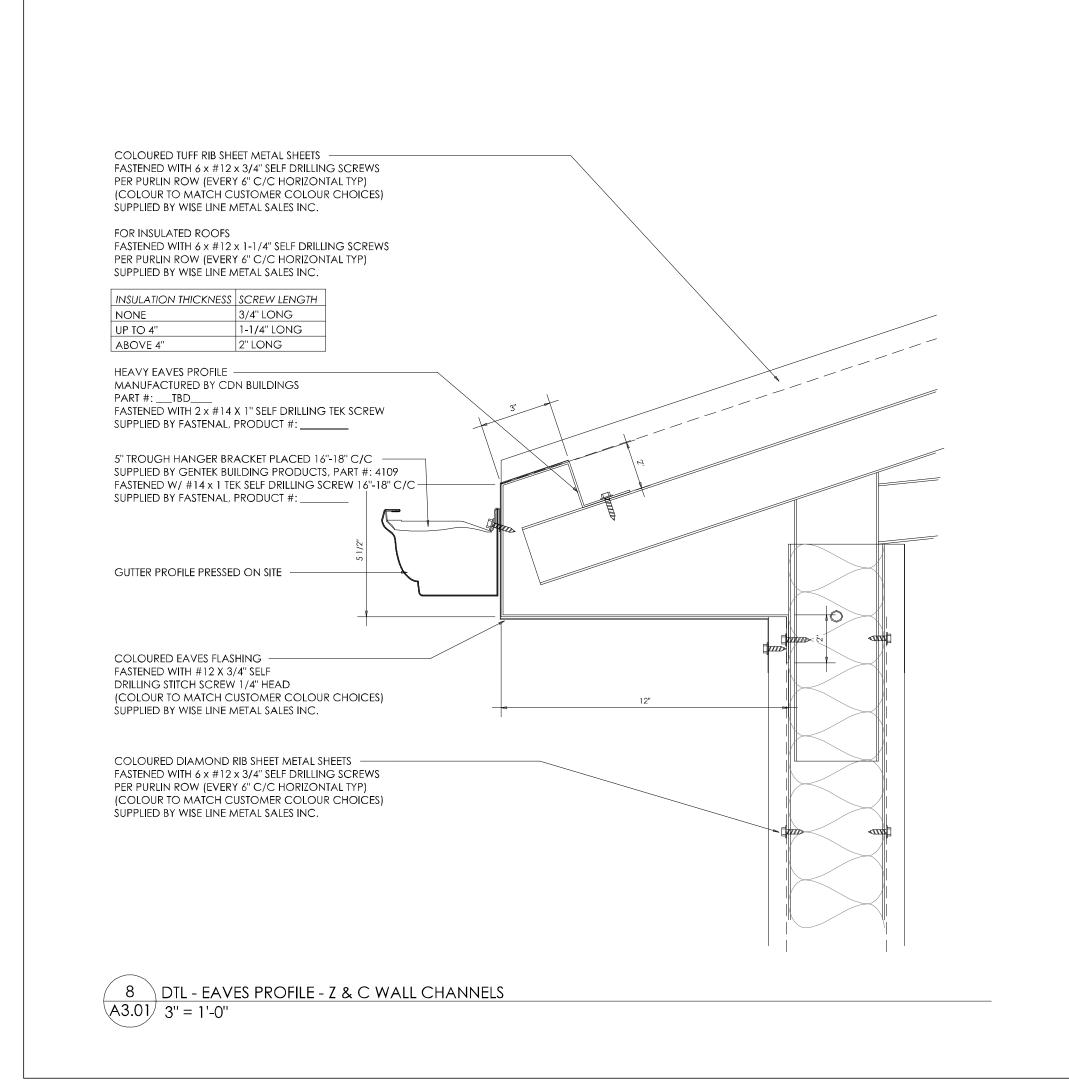


Delhi, ON N4B 2C2 Office: 519 582 8222 Fax: 519 582 2098

Website: http://www.cdnbuildings.com

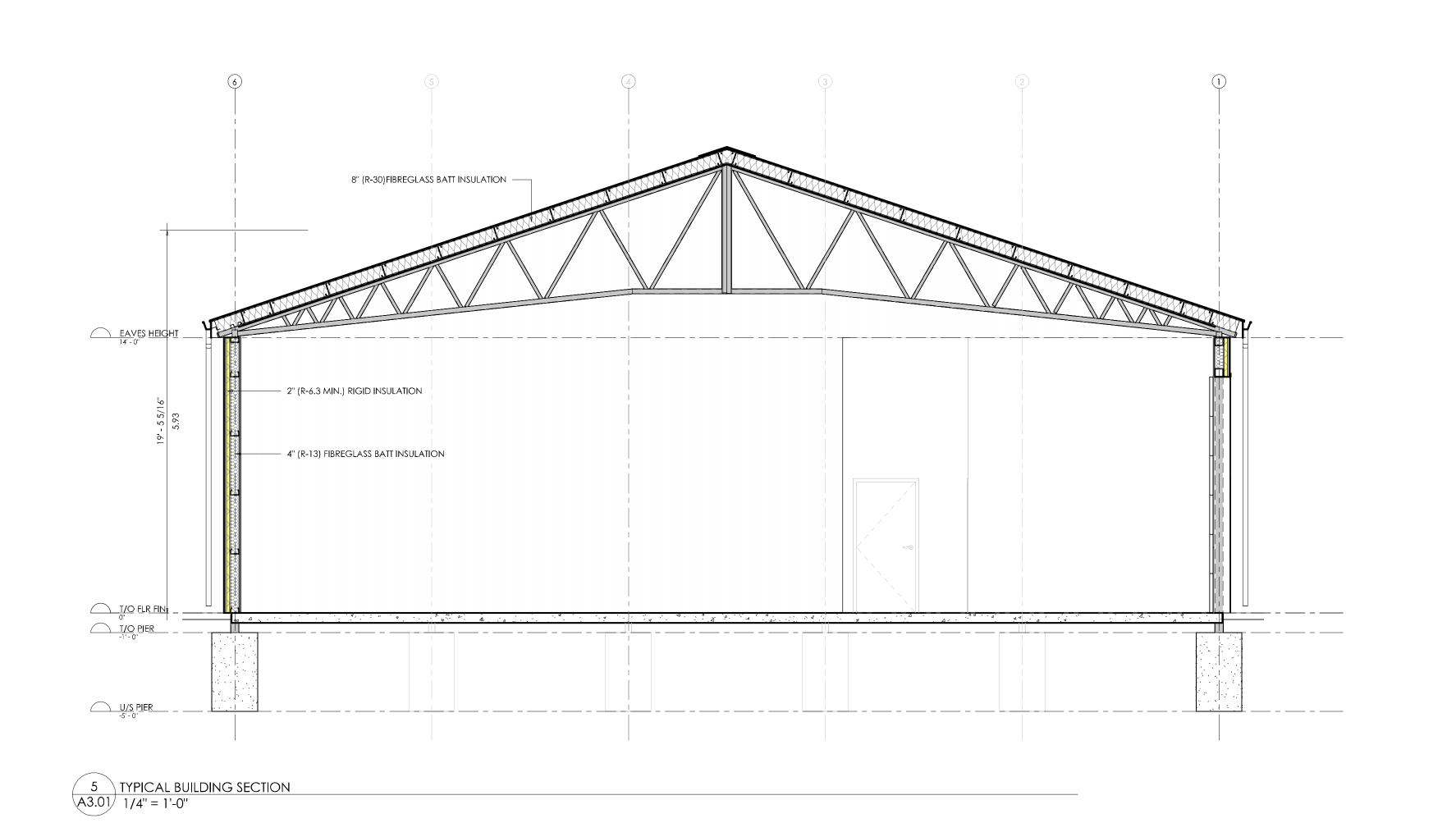


NO.	DESCRIPTION	DAT
1	ISSUED FOR COORDINATION	2024/07/
2	ISSUED FOR PERMITS	2024/12/
3	ISSUED FOR PERMITS	2025/03/
4	AS PER COMMENTS	2025/06/
5	AS PER COMMENTS	2025/09/



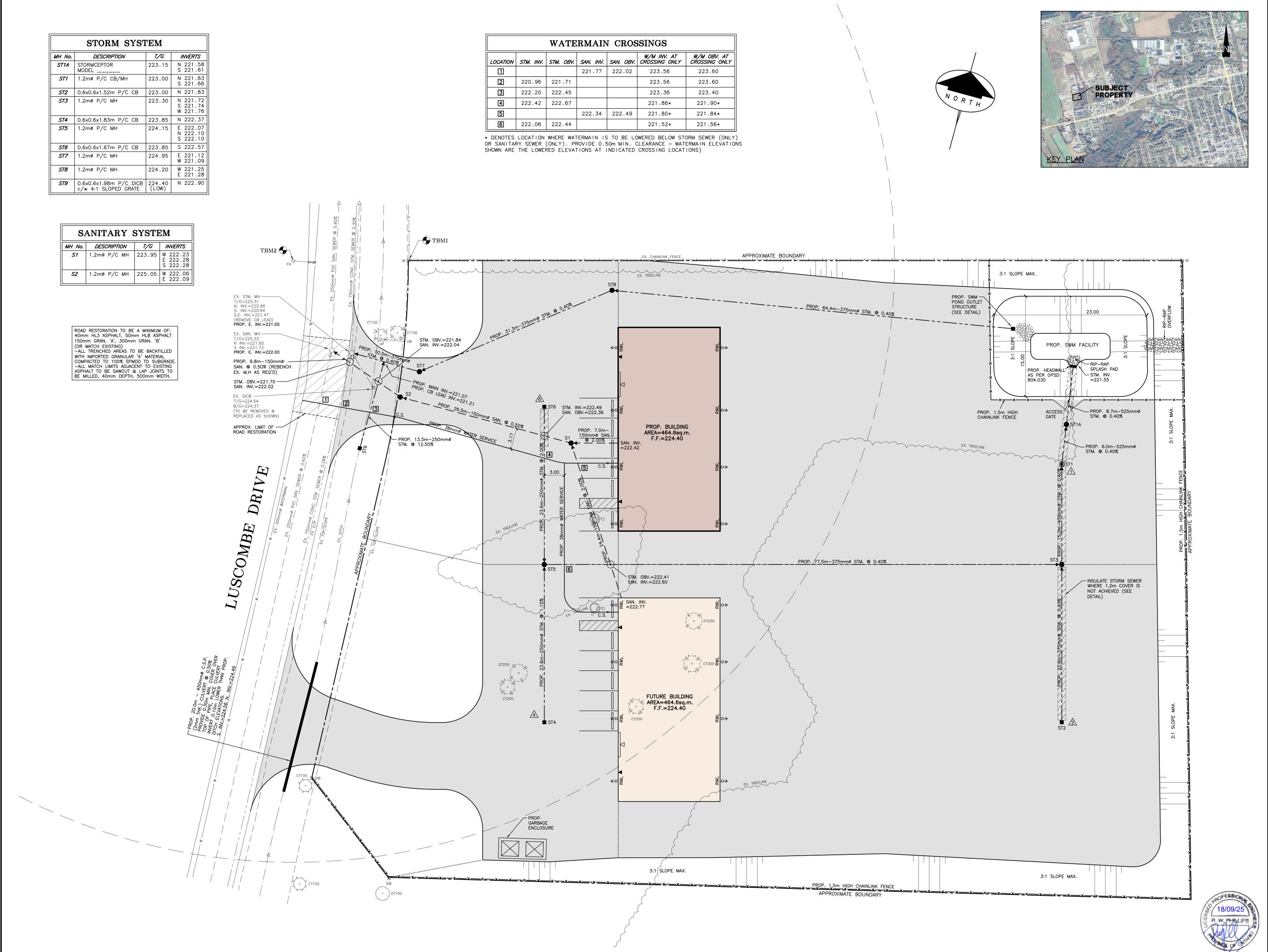
WALL SHEATHING -

2" HSS INFILL FRAMING (TYP) -



KOUTNY TONY LUSCOMBE DRIVE, SIMCOE, ONTARIO DETAILS 24-1022 ≳ PROJECT NUMBER

DATE	2024/07/02
DRAWN BY	WR
CHECKED BY	AA
A3.0	D1
SCALE	As indicated



THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND ABOVEGROUND UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE COMMENCING WORK, THE CONTRACTOR SHALL FAMILIARIZE HIMSELF OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

LEGEND:

ST EXISTING SANITARY MANHOLE

ST1 EXISTING STORM MANHOLE

□ CB EXISTING CATCHBASIN

- → FH EXISTING FIRE HYDRANT

EXISTING VALVE & BOX

*WB EXISTING WATER BOX

● S1 PROPOSED SANITARY MANHOLE

PROPOSED SANITARY SERVICE

PROPOSED STORM SERVICE

PROPOSED STORM MANHOLE

PROPOSED CATCHBASIN MANHOLE

■ CB PROPOSED CATCHBASIN

PROPOSED WATER SERVICE & CURBSTOP

PROPOSED RAIN WATER LEADER

T.B.M. No. 1 ELEV. = 225.25m (GEO)

TOP OF IRON BAR @ NORTHWEST CORNER OF PROPERTY AS SHOWN (FROM JOETOPO SURVEY)

T.B.M. No. 2 ELEV. = 226.10m (GECTOP NUT OF FIRE HYDRANT WEST SIDE OF LUSCOMBE DRIVE, AS SHOWN (TAKEN BY S. KIPP JAN. 7, 2025)

AS PER COUNTY COMMENTS

08/01/25 K.P.B.

REVISION

DATE
(MM/DD/YY)

BY



J.H. COHOON ENGINEERING LIMITED

CONSULTING ENGINEERS

440 HARDY ROAD , UNIT #1 , BRANTFORD — ONTARIO , N3T 5L8
TEL. (519) 753—2656 FAX. (519) 753—4263 www.cohooneng.com

ROJECT:

PROPOSED

MANUFACTURING BUILDING

LUSCOMBE DRIVE, SIMCOE

NORFOLK COUNTY

TONY KOUNTY

SERVICING PLAN

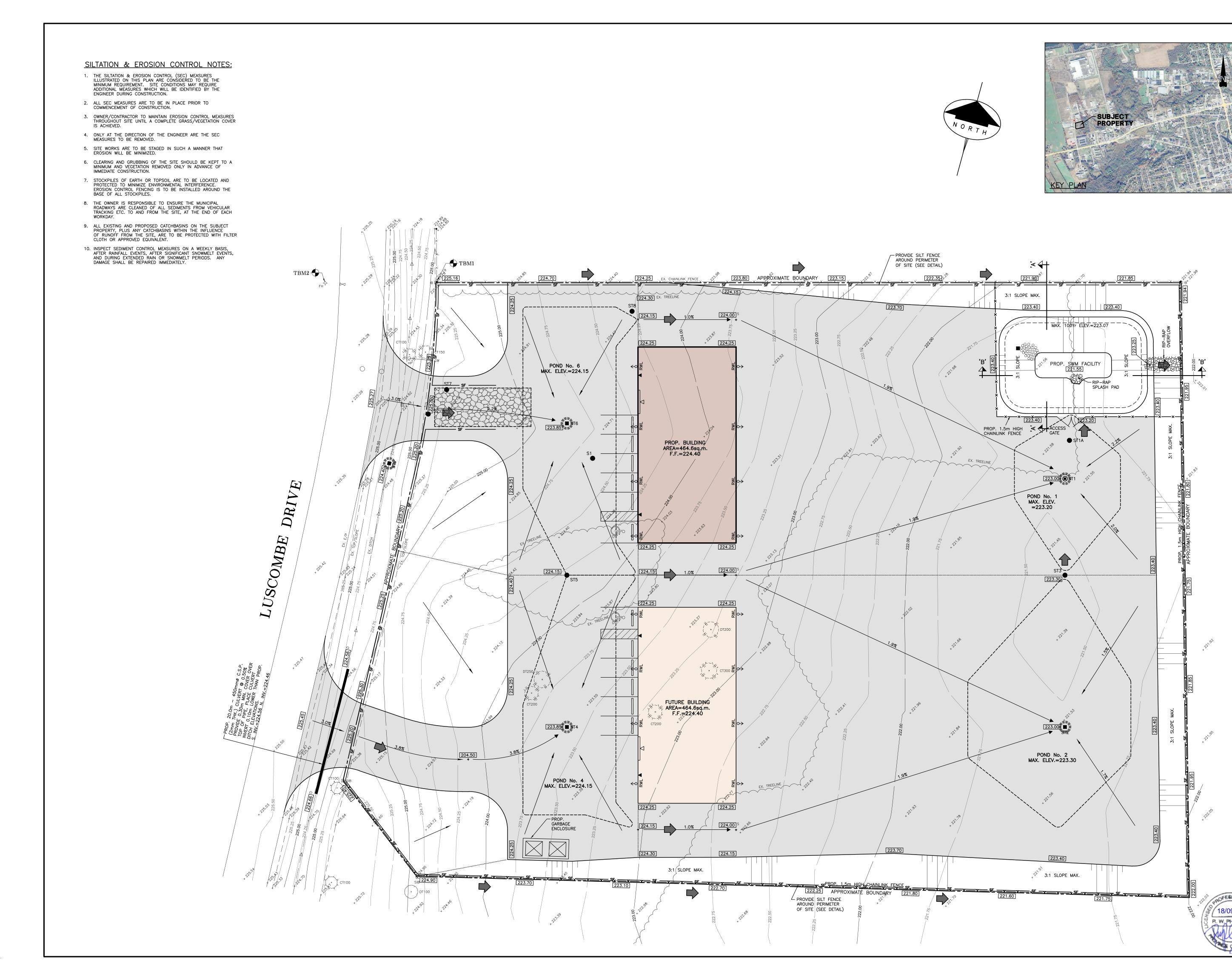
MAR. 20/25

IGN: R.W.P. SCALE: 1:250

WN: S.L.M./K.P.B. JOB No: 17233

CCKED: R.W.P. DWG. No:

17233-3



THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND ABOVEGROUND UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE COMMENCING WORK, THE CONTRACTOR SHALL FAMILIARIZE HIMSELF OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

LEGEND:

EXISTING ELEVATIONS

200.00 PROPOSED ELEVATIONS

200.00 PROPOSED SWALE ELEVATIONS

200.00 PROPOSED DITCH ELEVATIONS

PROPOSED SWALE

GENERAL DRAINAGE

PROPOSED RAIN WATER LEADER LOCATION

PROPOSED PRE—ASSEMBLED SILTATION
CONTROL FENCING

SILT SACK AS SHOWN

EX. TREES TO REMAIN

EX. TREES TO BE REMOVED

DENOTES MANDOOR LOCATIONS

DENOTES OVERHEAD DOOR LOCATIONS

4-2 14-2

OVERLAND FLOW ROUTE

LIMIT OF S.W.M. POND (100 Yr. EVENT)

MAX. POND ELEVATION AS NOTED

NOTES

1. ALL ELEVATIONS SHOWN ARE METRIC.

 BUILDER/OWNER TO VERIFY COMPLIANCE WITH ZONING BYLAWS (ie. SIDEYARDS, SETBACKS, REARYARDS ETC.)

TOPOGRAPHIC SURVEY PROVIDED BY JOETOPO SURVEYS AND CADD INC. DATED NOV. 14, 2024.

T.B.M. No. 1 ELEV. = 225.25m	(GEO)
OP OF IRON BAR @ NORTHWEST CORNER OF PROPERTY FROM JoeTOPO SURVEY)	AS SHOWN

T.B.M. No. 2 ELEV. = 226.10m (GEO)

TOP NUT OF FIRE HYDRANT WEST SIDE OF LUSCOMBE DRIVE, AS SHOWN (TAKEN BY S. KIPP JAN. 7, 2025)

1	AS PER COUNTY COMMENTS	08/01/25	ĸ.
۱0.	REVISION	DATE (MM/DD/YY)	
	·	•	



J.H. COHOON
ENGINEERING
LIMITED
CONSULTING ENGINEERS

440 HARDY ROAD , UNIT #1 , BRANTFORD — ONTARIO , N3T 5L8 TEL. (519) 753—2656 FAX. (519) 753—4263 www.cohooneng.com

ROJECT:

PROPOSED

MANUFACTURING BUILDING

LUSCOMBE DRIVE, SIMCOE

NORFOLK COUNTY

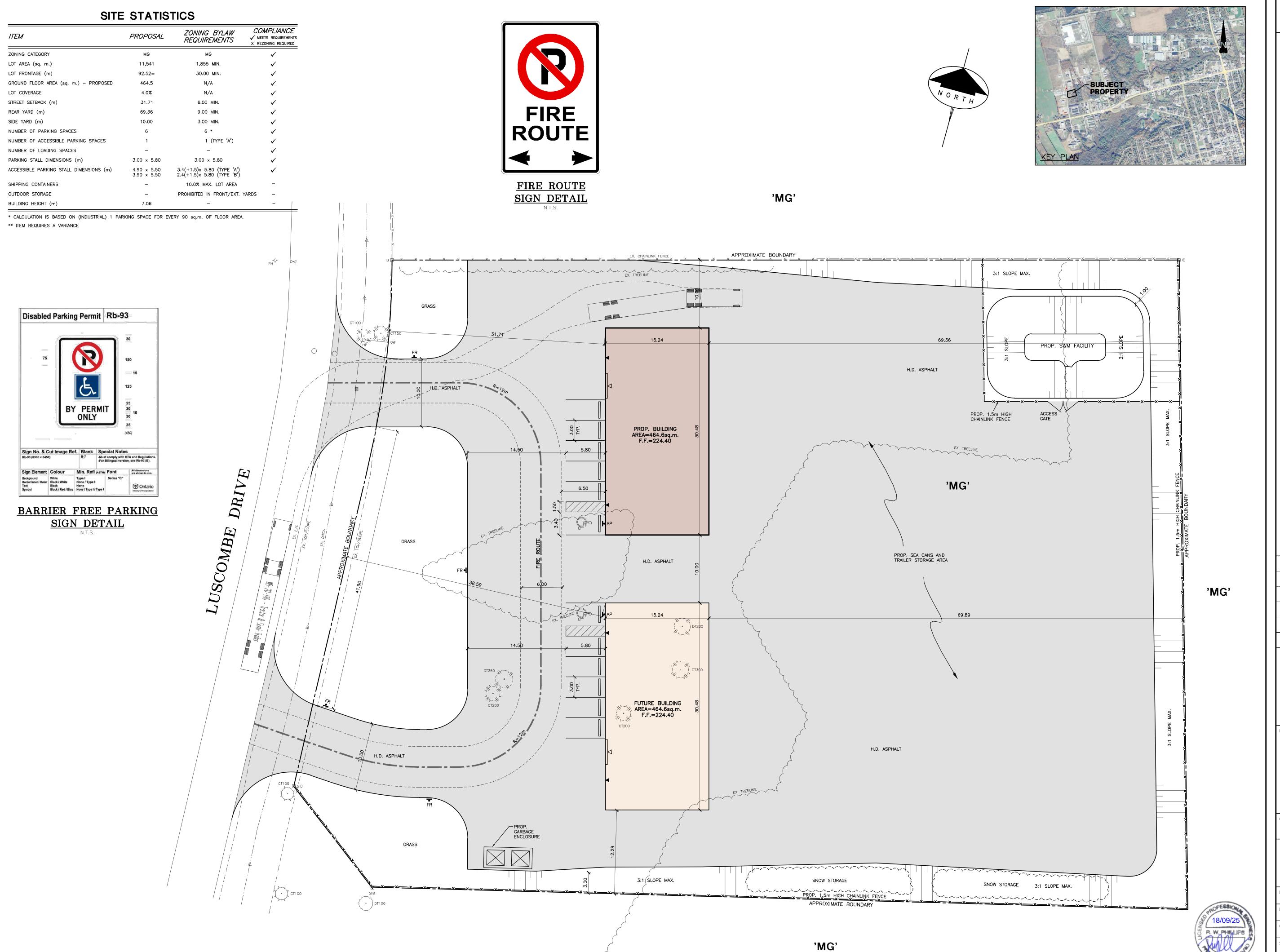
TONY KOUNTY

MAR. 20/25

GRADING AND
SILTATION & EROSION

CONTROL PLAN				
DESIGN:	R.W.P.	SCALE: 1:250		
DRAWN:	S.L.M./K.P.B.	JOB No:		
CHECKED:	R.W.P.	17233		
SHEET:	2 of 4	DWG. No:		

17233-2



THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND ABOVEGROUND UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE COMMENCING WORK, THE CONTRACTOR SHALL FAMILIARIZE HIMSELF OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

LEGEND:

▲ DENOTES MANDOOR LOCATIONS

Δ DENOTES OVERHEAD DOOR LOCATIONS

■ AP DENOTES ACCESSIBLE PARKING SIGN

• E)

EX. TREES TO REMAIN

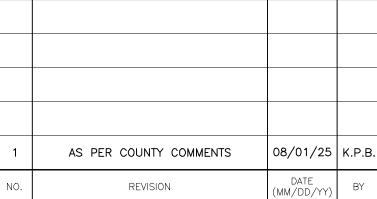
EX. TREES TO BE REMOVED

DENOTES FIRE ROUTE SIGN

NOTE

1. ALL ELEVATIONS SHOWN ARE METRIC.

 BUILDER/OWNER TO VERIFY COMPLIANCE WITH ZONING BYLAWS (ie. SIDEYARDS, SETBACKS, REARYARDS ETC.)





J.H. COHOON ENGINEERING LIMITED

440 HARDY ROAD , UNIT #1 , BRANTFORD — ONTARIO , N3T 5L8 TEL. (519) 753—2656 FAX. (519) 753—4263 www.cohooneng.com

ROJECT:

PROPOSED

MANUFACTURING BUILDING

LUSCOMBE DRIVE, SIMCOE

NORFOLK COUNTY

TONY KOUNTY

SITE PLAN

R.W.P. SCALE: 1:250

S.L.M./K.P.B. JOB No: 1773

CHECKED: R.W.P. 17233

SHEET: 1 of 4

DATE: MAR. 20/25

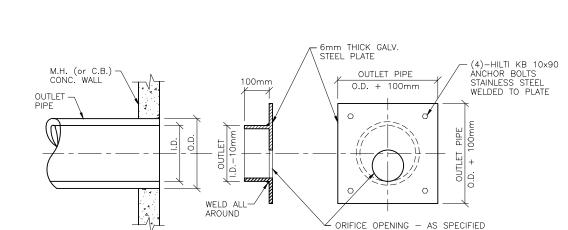
ROADWORKS:

- ROADWAYS & RELATED WORKS SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPLICABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLK DEVELOPMENT AND ENGINEERING STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS, BEST PRACTICES AND STANDARDS PRESCRIBED
- 2. CATCH BASIN CONNECTIONS TO BE PVC PIPE CSA B182.4, SDR 35 OR 'ULTRA RIB'. SINGLE CATCH BASIN LEADS TO BE MIN. 250mm DIA. DOUBLE CATCH BASIN LEADS TO BE MIN. 300mm DIA. REAR LOT CATCHBASIN LEADS AND DITCH INLET
- LEADS TO BE CSA A257.1 EXTRA STRENGTH CL3 CONC. 300mm DIA. AT 1.0% MIN. 3. SINGLE CATCH BASINS AS PER OPSD 705.010 FRAME AND COVER AS PER OPSD 400.110.
- 4. FINAL ROADWAY CROSSFALL TO BE 2.0%

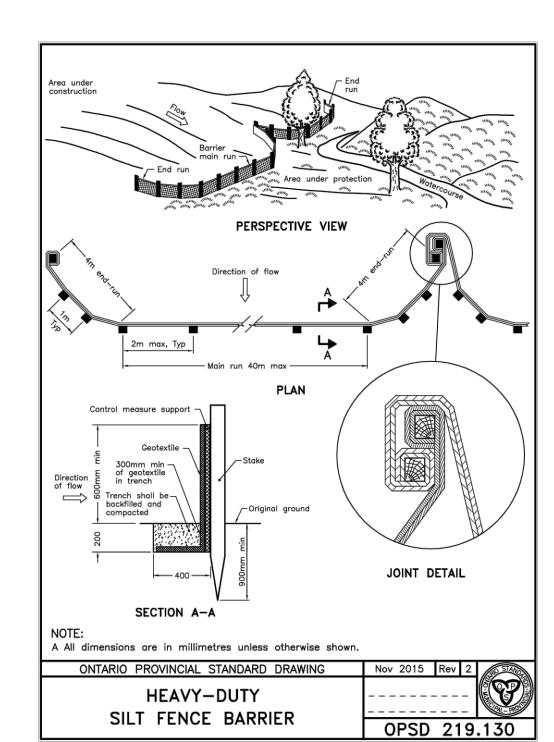
EROSION CONTROL PLAN.

- 5. VALVES, MANHOLES AND CATCH BASINS SHALL BE PLACED AT ASPHALT BINDER COURSE (HL4) ELEVATION AS DIRECTED BY THE ENGINEER.
- 6. AT SAG POINTS, CATCH BASIN ADJUSTMENT AND PAVING TO BE PLACED IN SUCH
- 7. FINAL ASPHALT COURSE (HL3) SHALL BE PLACED IN ACCORDANCE WITH APPROVED COUNTY OF NORFOLK STANDARDS FOR TIMING, AS DIRECTED BY THE ENGINEER AND
- AS PER THE REQUIREMENTS STIPULATED IN THE DEVELOPMENT AGREEMENT. 8. FOR MANHOLE AND CATCH BASIN TOP ADJUSTMENTS, ALL PERMANENT ADJUSTMENTS
- ARE TO BE POURED IN PLACE OR APPROVED EQUIVALENT (e.g. MODULOC). 9. ALL BEDDING AND BACKFILL MATERIAL, ROAD SUB-GRADES AND GRANULAR ROAD
- BASES SHALL BE COMPACTED TO MIN 100% SPD UNLESS OTHERWISE SPECIFIED. 10. SILTATION CONTROL BARRIERS SHALL BE PLACED AS DETAILED ON THE SILTATION AND
- 11. ADDITIONAL SILT CONTROL LOCATIONS MAY BE REQUIRED AS DETERMINED BY THE COUNTY AND/OR THE ENGINEER.

C.B. ORIFICI	E PLATE SIZING
ORIFICE PLATE CONFIGURATION NUMBER	DIAMETER OF ORIFICE PLATE OPENING
\triangle	175mm
<u>/2</u>	175mm
4	100mm
<u>^</u>	85mm



FLOW CONTROL DEVICE ORIFICE PLATE



GENERAL NOTES:

- CONSTRUCTION OF SEWERS, WATERMAINS AND RELATED APPURTENANCES SHALL BE UNDERTAKEN IN ACCORDANCE WITH THE CURRENT STANDARD DRAWINGS OF THE COUNTY OF NORFOLK. AND THE ONTARIO PROVINCIAL STANDARDS DRAWINGS (OPSD). THE COUNTY OF NORFOLK DRAWINGS SHALL TAKE PRECEDENCE OVER THE OPSD DRAWINGS.
- INFORMATION REGARDING ANY EXISTING SERVICES AND/OR UTILITIES SHOWN ON THE APPROVED SET OF CONSTRUCTION DRAWINGS IS FURNISHED AS THE BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL INTERPRET THIS INFORMATION AS THEY SEE FIT WITH THE UNDERSTANDING THAT THE OWNER AND HIS AGENTS DISCLAIM ALL RESPONSIBILITY FOR ITS ACCURACY AND/OR SUFFICIENCY.
- 3. ALL DIMENSIONS SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO ANY CONSTRUCTION AND HE SHALL REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ENGINEER.
- 4. RELOCATION OF EXISTING SERVICES AND/OR UTILITIES SHALL BE CONSTRUCTED AS SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN ALL PERMITS FOR CONSTRUCTION.
- 6. FOR ALL SEWERS AND WATERMAIN IN FILL SECTIONS, THE COMPACTION SHALL BE VERIFIED PRIOR TO LAYING OF PIPE.
- 7. NO SUBSTITUTIONS WILL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE COUNTY OF NORFOLK OR THE ENGINEER.
- 8. NO BLASTING WILL BE PERMITTED.
- 9. ALL EXCAVATIONS TO BE BACKFILLED WITH SELECT NATIVE MATERIAL, APPROVED BY THE ENGINEER, TO 95% S.P.D.
- 10. THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR INSTALLING AND MAINTAINING (UNTIL ROAD CONSTRUCTION IS FINISHED) SILT CONTROL DEVICES AS SHOWN ON THE DRAWINGS AND AS DIRECTED BY THE ENGINEER
- 11. TREE PROTECTION PROCEDURES TO BE IMPLEMENTED IN ACCORDANCE WITH COUNTY OF NORFOLK STANDARDS.
- 12. ALL WORKS SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPLICABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLK DEVELOPMENT & ENGINEERING STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS AND STANDARDS PRESCRIBED BY THE COUNTY.
- 13. ALL BOULEVARD AREAS TO BE RESTORED WITH #1 NURSERY SOD ON A MINIMUM 100mm OF
- 14. ALL TRENCH BACKFILL UNDER EXISTING ROADWAYS SHALL BE COMPACTED IN MINIMUM 230mm LIFTS TO 98% STANDARD PROCTOR DENSITY. A GEOTECHNICAL ENGINEER'S REPRESENTATIVE SHALL BE ON SITE DURING THE WORK TO VERIFY THE COMPACTION OF EACH LIFT. THE
- 15. AN ENGINEER IS REQUIRED TO BE ONSITE FOR INSPECTION OF ALL UNDERGROUND SERVICES.

CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COSTS OF RE-TESTING.

ELEV.=223.40

16. DRIVEWAYS MUST HAVE MINIMUM 1.0m CLEARANCE FROM CATCHBASINS, VALVES, HYDRANTS STREETLIGHT POLES, TRANSFORMERS, CANADA POST COMMUNITY MAILBOX LOCATIONS ETC.

WATERMAINS:

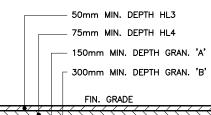
BY THE COUNTY.

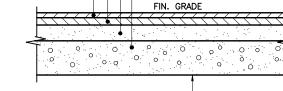
- 1. WATERMAINS AND RELATED APPURTENANCES SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPLICABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLK DEVELOPMENT AND ENGINEERING STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS, BEST PRACTICES AND STANDARDS PRESCRIBED
- 2. WATERMAINS TO BE INSTALLED WITH A MINIMUM DEPTH OF COVER OF 1.70m BELOW
- 3. WATERMAINS TO BE INSTALLED IN ACCORDANCE WITH OPSD 802.010 TYPE 2. TRENCH BEDDING TO BE GRANULAR 'A' UNLESS OTHERWISE NOTED.
- 4. WATERMAINS TO BE PVC DR-18 IN ACCORDANCE WITH AWWA C900 & CSA B137.3. THE PIPE SHALL BE SHIPPED TO THE SITE WITH THE ENDS FACTORY CAPPED IF APPLICABLE.
- 5. FOR PVC WATERMAIN DEFLECTION: - MAXIMUM ALLOWABLE DEFLECTION OF 1 DEGREE PER JOINT SHALL NOT BE EXCEEDED. - EACH JOINT SHALL BE DEFLECTED AN EQUAL AMOUNT.
- 6. ALL WATER MAINS TO BE SWABBED, TESTED, DISINFECTED AND FLUSHED UNDER THE SUPER-VISION OF THE ENGINEER TO THE SATISFACTION OF THE COUNTY OF NORFOLK PRIOR TO CONNECTION TO THE EXISTING MUNICIPAL SYSTEM. REFER TO OPSS 701.07.25, AWWA C651 & COUNTY OF NORFOLK GENERAL WATERMAIN DISINFECTION PROCEDURES.
- 7. A REDUCED PRESSURE DOUBLE BACKFLOW PREVENTER IS REQUIRED ON THE TEMPORARY SUPPLY LINES USED FOR FILLING AND FLUSHING/SWABBING OF WATERMAINS AND TO BE TESTED AND CERTIFIED ON SITE.
- 8. UPON COMPLETION OF INSTALLATION, THE CONTRACTOR SHALL PERFORM A PRESSURE TEST ON THE WATERMAINS AS PER OPSS 701.07.24 AND COUNTY OF NORFOLK SPECIFICATIONS. WATERMAIN IS TO BE TESTED UNDER THE SUPERVISION OF THE ENGINEER PRIOR TO CONNECTION TO EXISTING WATERMAINS USING TEMPORARY CAPS OR PLUGS IF APPLICABLE.
- 9. PIPE CLOSURES WHERE REQUIRED, ARE TO BE SUPPLIED BY THE CONTRACTOR. THE CONTRACTOR SHALL ALSO SUPPLY AND INSTALL ALL ADAPTOR PIECES IN ORDER TO CONNECT EXISTING WATERMAINS.
- 10. ALL WATER SERVICE CONNECTIONS 38mm DIA. ASTM B88 TYPE 'K' SOFT COPPER AS PER OPSD 1104.01 & COUNTY OF NORFOLK ENGINEERING STANDARDS, WITH SAND BEDDING.
- 11. NO WORK ON WATER SERVICES CAN TAKE PLACE WITHOUT SUPERVISION OF A LICENSED NORFOLK COUNTY WATER OPERATOR ON-SITE.

- 3:1 SLOPED GRATE

SANITARY & STORM SEWERS:

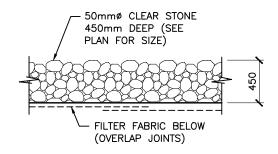
- 1. SANITARY & STORM SEWERS & RELATED APPURTENANCES SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPLICABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLK DEVELOPMENT AND ENGINEERING STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS, BEST PRACTICES AND STANDARDS PRESCRIBED BY THE COUNTY.
- 2. COVER AND BEDDING MATERIAL FOR CONCRETE PIPE AS PER OPSD 802.030 CLASS 'B' BEDDING SHALL BE GRANULAR 'A' MATERIAL UNLESS OTHERWISE INDICATED.
- 3. COVER AND BEDDING MATERIAL FOR PVC PIPE AS PER OPSD 802.010 TYPE 2 TRENCH BEDDING SHALL BE GRANULAR 'A' MATERIAL UNLESS OTHERWISE INDICATED.
- 4. PVC PIPE WILL REQUIRE SPECIAL CONSTRUCTION PROCEDURES FOR LEAKAGE AND TESTING, PIPE DEFLECTION, ETC.
- 5. ALL SEWERS TO BE FLUSHED & VIDEOED PRIOR TO THE SUBMISSION OF THE FIRST INTERIM COMPLETION CERTIFICATE AND PRIOR TO THE FINAL COMPLETION CERTIFICATE.
- 6. ALTERNATE MATERIALS MAY BE ACCEPTABLE, PROVIDED APPROVAL HAS FIRST BEEN OBTAINED FROM THE COUNTY OF NORFOLK AND ENGINEER IN WRITING.
- 7. ALL STORM MANHOLES AND CATCHBASINS TO BE PRECAST CONCRETE STRUCTURES MANUFACTURED IN PRE QUALIFIED PLANTS IN ACCORDANCE WITH THE LATEST APPLICABLE ONTARIO PROVINCIAL STANDARD (OPS) DRAWINGS AND SPECIFICATIONS.
- 8. ALL SEWER INSTALLATIONS TO CONFORM WITH OPSD 802.031 TYPE 3 SOIL.
- 9. ALL MANHOLE FRAMES AND COVERS TO CONFORM WITH OPSD 401.010 TYPE 'A' CLOSED
- 10. MANHOLES SHALL BE SUPPLIED TO THE SITE PRE-BENCHED UNLESS OTHERWISE NOTED.
- 11. PRIVATE SANITARY & STORM DRAINS TO BE LOCATED AS PER THE TYPICAL LOT SERVICING
- 12. PRIVATE SANITARY DRAINS TO 150mmø PVC DR28 PIPE.
- 13. A 38x89mm x 2.0m LONG MARKER IS TO BE PLACED FROM THE CAPPED LATERAL AND EXTEND 1.0m ABOVE GROUND AND PAINTED GREEN FOR SANITARY AND WHITE FOR STORM.
- 14. BEDDING FOR PRIVATE SANITARY & STORM DRAINS AS PER OPSD 1006.02 TYPE 2 TRENCH WITH GRANULAR 'A' BEDDING AND COVER MATERIAL.
- 15. MINIMUM FALL FOR PRIVATE SANITARY & STORM DRAINS TO BE 2.0%
- 16. ALL APPLICABLE PERMITS ARE TO BE APPLIED FOR PRIOR TO THE INSTALLATION OF ANY SERVICES.
- 17. ALL SANITARY SEWER LATERAL CONNECTIONS SHALL BE INSTALLED USING PREFABRICATED TEES. THE USE OF SERVICE SADDLES MUST BE APPROVED BY NORFOLK COUNTY ENVIRONMENTAL SERVICES DIVISION. ALL CONNECTIONS SHALL CONFORM TO CURRENT OPSD 1006.010 AND OPSS 410.
- 18. NO DEFLECTIONS OF SANITARY LATERALS ALLOWED FROM MAIN TO PROPERTY LINE. NEW CONNECTIONS MUST BE 3.0m FROM PROPERTY LINE BARS OR AS APPROVED BY THE MANAGER OF ENVIRONMENTAL SERVICES. AFTER THE PROPERTY LINE ONLY 22.5° FITTINGS ARE ALLOWED OR 1.2m. 45° SWEEPS. CLEANOUTS ARE REQUIRED EVERY 30.5m AND PRIVATE MANHOLES EVERY 91.3m.
- 19. SPATIAL SEPARATION FROM WATER SERVICE CONNECTIONS OF NOT LESS THAN 2.44m MEASURED HORIZONTALLY FROM UNDISTURBED OR COMPACTED EARTH OR AS APPROVED BY THE MANAGER OF ENVIRONMENTAL SERVICES.

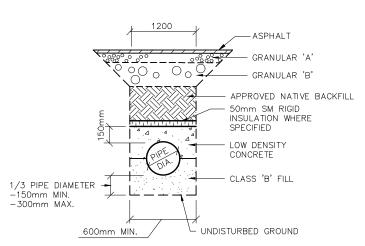




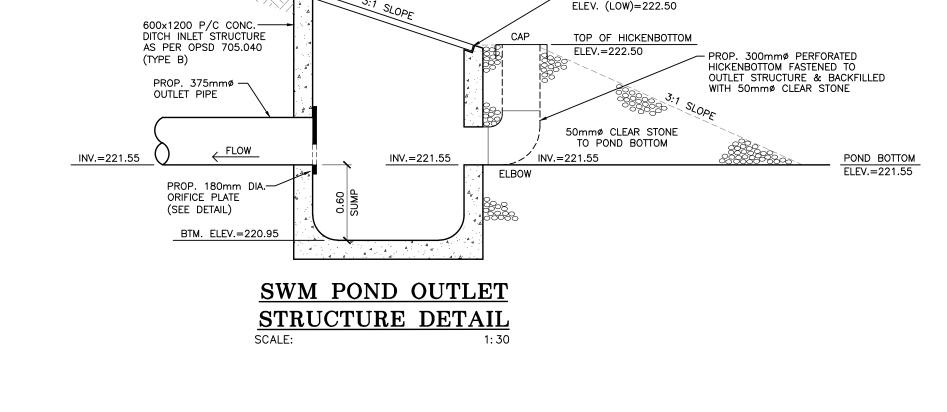
SUBGRADE -

HEAVY DUTY ASPHALT DESIGN

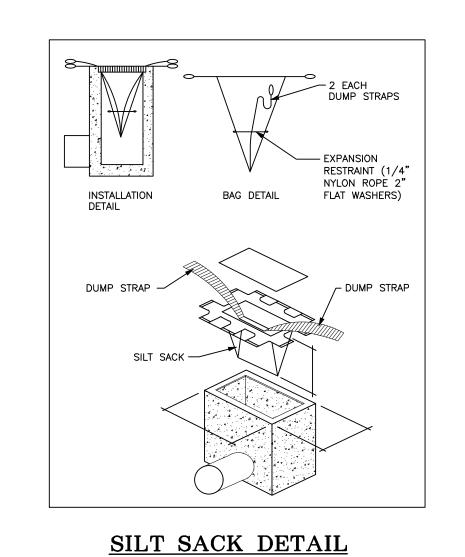


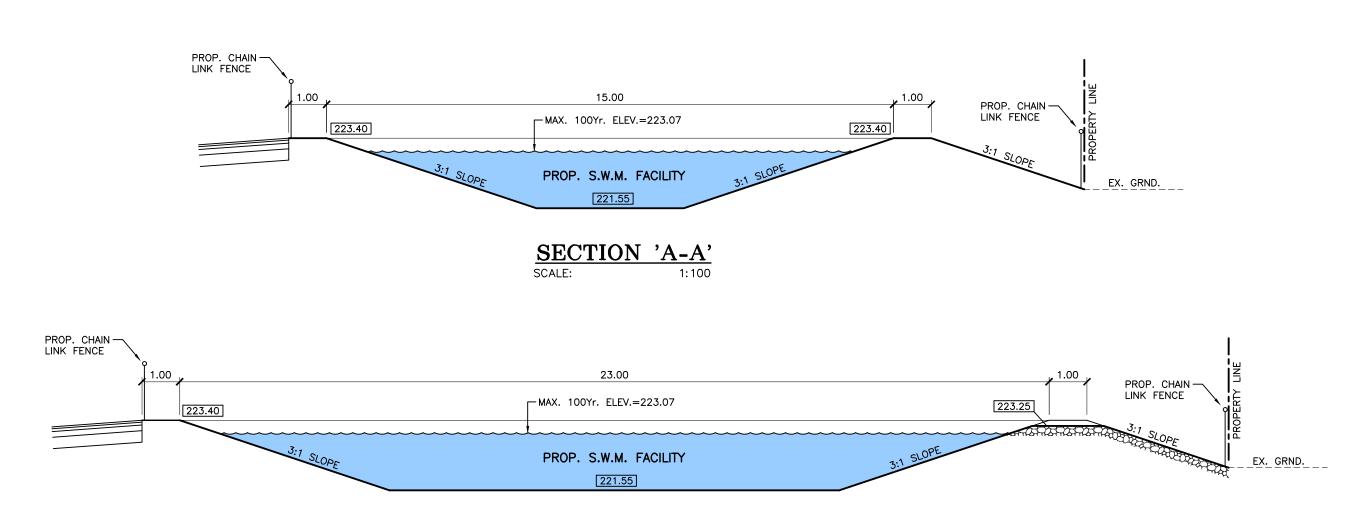


TYP. PIPE INSULATION DETAIL



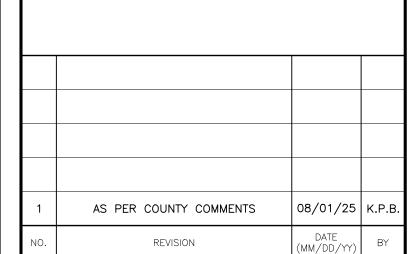
AS PER OPSD 403.010





SECTION 'B-B'
SCALE: 1:100





THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND ABOVEGROUND UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE

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440 HARDY ROAD , UNIT #1 , BRANTFORD - ONTARIO , N3T 5L8

PROPOSED MANUFACTURING BUILDING LUSCOMBE DRIVE, SIMCOE NORFOLK COUNTY

TONY KOUNTY

MAR. 20/25

NOTES & DETAILS

ESIGN:	R.W.P.	SCALE: AS SHOWN
RAWN:	S.L.M./K.P.B.	JOB No:
HECKED:	R.W.P.	17233
HEET:	4 of 4	DWG. No:
ATE:	MAR 20/25	17233-4

APPRAISAL REPORT

PROPERTY:

Luscombe Drive, Simcoe, Ontario

PREPARED FOR:

Authorized Client and User CDNbuildings

ATTENTION:

Bill Dendekker

EFFECTIVE DATE OF APPRAISAL:

July 1, 2025

PREPARED BY:

R. A. (SANDY) MOIR, P. APP., AACI, H. B. COMM.
MOIR LUFT REAL ESTATE
APPRAISAL SERVICE LTD.
34178 Bush Line,
R.R. #2 Pt. Stanley, Ontario
N5L 1J2

EXECUTIVE SUMMARY

DATE OF REPORT:	July 7, 2025
DATE OF INSPECTION:	July 1, 2025
EFFECTIVE DATE OF VALUATION:	July 1, 2025
ROLL NUMBER:	403-025-03825-0000
REGISTERED OWNERS:	2712300 Ontario Inc.
MUNICIPAL ADDRESS:	Luscombe Drive, Simcoe, Ontario
AREA OF SUBJECT PROPERTY:	290.35' X Irr. – 2.87 acres (Tax Notice)
OFFICIAL PLAN:	`Protected Industrial`
ZONING:	"General Industrial Zone - (MG)"
EXISTING USE:	Vacant Industrial Land
HIGHEST AND BEST USE:	Vacant Industrial Land
EXPOSURE TIME:	49 to 240 days

MARKET VALUE ESTIMATE

One approach to value was used in the valuation of the subject as a vacant parcel of industrial land. This approach utilized market data to support the resulting value estimate or range. This approach represents the most reasonable current market value estimate, for the subject property, as of the effective date of valuation.

ESTIMATE OF THE CURRENT MARKET VALUE BY THE:

COST APPROACH TO VALUE: Not Applicable

INCOME APPROACH TO VALUE:

Not Applicable

DIRECT COMPARISON APPROACH: \$861,000.00

FINAL ESTIMATE OF THE CURRENT MARKET VALUE: \$861,000.00

The foregoing opinion of value is subject to the limiting conditions appearing in the following report.

The estimated market value is based on `Fee Simple` rights and does not include consideration of any extraordinary financing, rental or income guarantees, special tax considerations or any other typical benefit which may influence the ordinary market value of the property, unless the effects of such considerations have been specifically described and measures in this report.

EFFECTIVE DATE OF APPRAISAL

The principal of change is an important consideration in any appraisal assignment. Change is a constant process with economic factors, land use patterns, development trends, etc. occurring all the time. Therefore, an estimate of the Market Value must be effective as of a specific date or dates.

This appraisal is effective as of July 1, 2025.

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ADDENDUM

Official Plan

Permitted Uses (2 Pages)

Zoning Map

Site Map

Comparable Location Map

Direct Comparison Chart

Photographs (3 Pages)

Moir Luft Real Estate Appraisal Services Ltd.

July 7, 2025

Authorized Client and User CDNbuildings

RE: Luscombe Drive, Simcoe, Ontario

Pursuant to the request, an examination of the above-mentioned property, a vacant parcel of industrial land, was completed on July 1, 2025.

The purpose of the appraisal is to estimate the Current Market Value, of the subject property, in the Fee Simple, as of July 1, 2025.

This report is a `Narrative Appraisal Report` in accordance with the Canadian Uniform Standards of Professional Appraisal Practice (CUSPAP).

After a thorough analysis of all information available at this time, it is my opinion that the Current Market Value, of the subject property, as of July 1, 2025, is:

---EIGHT HUNDRED AND SIXTY-ONE THOUSAND DOLLARS--- (\$861,000.00)

The foregoing opinions of value are subject to the limiting conditions appearing in the following report.

Respectfully submitted
MOIR LUFT
REAL ESTATE APPRAISAL SERVICES LTD.

R. A. (SANDY) MOIR, P. App., AACI, H.B. Comm

Date Signed: July 11, 2025

37148 Bush Line, R.R. #2 Pt. Stanley, ON N5L 1J2 Tel: (519) 769-1234

Non A (Sarky) main

- 1. This report has been prepared at the request of the Authorized Client and User CDNbuildings, for the Purpose of providing an estimate of the Current Market Value, for Luscombe Drive, Simcoe, Ontario. The Intended Use of the report is to assist in internal matters and **NOT FOR LENDING PURPOSES**. It is not reasonable for any person other than to whom this report is addressed to rely upon this appraisal without first obtaining written authorization from the client and the author of this report. This report has been prepared on the assumption that no other person will rely on it for any other purpose and all liability to all such persons is denied.
- 2. This report has been prepared at the request of, and for the exclusive (and confidential) use of, the recipient as named herein and for the specific purpose and function as stated herein. All copyright is reserved to the author and this report is considered confidential by the author and the client. Possession of this report, or a copy thereof, does not carry with it the right to reproduction or publication in any manner, in whole or in part, nor may it be disclosed, quoted from or referred to in any manner, in whole or in part, without the prior written consent and approval of the author as to the purpose, form and content of any such disclosure, quotation or reference. Without limiting the generality of the foregoing, neither all nor any part of the contents of this report shall be disseminated or otherwise conveyed to the public in any manner whatsoever or through any media whatsoever or disclosed, quoted from or referred to in any report, financial statement, prospectus, or offering memorandum of the client, or in any documents filed with any governmental agency without the prior written consent and approval of the author as to the purpose, form and content of such dissemination, disclosure, quotation or reference.

- 3. The estimated market value of the real estate which is the object of this appraisal pertains to the value of the freehold estate in the real property, based on vacant possession. The property rights appraised herein exclude mineral rights, if any.
- 4. The estimate of value contained in this report is founded upon a thorough and diligent examination and analysis of information gathered and obtained from numerous sources. Certain information has been accepted at face value; especially if there was no reason to doubt its accuracy. Other empirical data required interpretive analysis pursuant to the objective of this mandate. For these reasons, the analyses, opinions and conclusions contained in this report are subject to the following assumptions and limiting conditions:
- 5. The property has been valued on the basis that title to the real estate herein appraised is good and marketable.
- 6. The author of this report cannot accept responsibility for legal matters, questions of survey, opinions of title, hidden or unapparent conditions of the property, toxic wastes or contaminated materials, soil or sub-soil conditions, environmental, engineering or other technical matters which might render this property more or less valuable than as stated herein. If it came to our attention as the result of our investigation and analysis that certain problems may exist, a cautionary note has been entered in the body of this report.

- 7. The legal description of the property and the area of the site was obtained from plans provided by the client. Further, the plans and sketches contained in this report are included solely to aid the recipient in visualizing the location of the property, the configuration and boundaries of the site and the relative position of the improvements on the said lands.
- 8. The property has been valued on the basis that the real estate is free and clear of all value influencing encumbrances, encroachments, restrictions or covenants except as may be noted in this report and that there are no pledges, charges liens or special assessments outstanding against the properties combined other than as stated and described herein.
- 9. The property has been valued on the basis that there are no outstanding liabilities except as expressly noted herein, pursuant to any agreement with a municipal or other government authority, pursuant to any contract or agreement pertaining to the ownership and operation of the real estate or pursuant to any lease or agreement to lease, which may affect the stated value or saleability or the subject properties combined or any portion thereof.

- 10. The property has been valued on the basis that the real estate complies in all material respects with any restrictive covenants affecting the site and has been built and is occupied and being operated, in all material respects, in full compliance with all requirements of law, including all zoning, land use classification, building, planning, fire and health by-laws, rules, regulations, orders and codes of all federal, provincial, regional and municipal governmental authorities having jurisdiction with respect thereto. (It is recognized there may be work orders or other notices of violation of law outstanding with respect to the real estate and that there may be certain requirements of law preventing occupancy of the real estate as described in this report. However, such possible circumstances have not been accounted for in the appraisal process.)
- 11. Investigations have been undertaken in respect of matters which require the use of land. However, no inquiries have been placed with the fire department, the building inspector, the health department or any other government regulatory agency, unless such investigations are expressly represented to have been made in this report. The subject property Hypothetical As If Complete must comply with such regulations and, if it does not comply, its non-compliance may affect the market value of this property. To be certain of such compliance, further investigations may be necessary.
- 12. The property has been valued on the basis that there is no action, suit, proceeding or investigation pending or threatened against the real estate or affecting the titular owners of the property, at law or in equity of before or by any federal, provincial or municipal department, commission, board, bureau, agency or instrumentality which may adversely influence the value of the real estate herein appraised.

- 13. The data and statistical information contained herein was gathered from reliable sources and are believed to be correct. However, this data is not guaranteed for accuracy, even though every attempt has been made to verify the authenticity of this information as much as possible.
- 14. Should the author of this report be required to give testimony or appear in court or at any administrative proceeding relating to this appraisal, prior arrangements shall be made therefore, including provisions for additional compensation to permit adequate time for preparation and for any other of these assumptions and limiting conditions is an attempt to limit the use that might be made of this report should it properly become evidence in a judicial proceeding. In such a case, it is acknowledged that it is the judicial body which will decide the use of this report which best serves the administration of justice.
- 15. Because market conditions, including economic, social and political factors, change rapidly and, on occasion, without notice or warning, the estimate of market value expressed herein, as of the effective date of this appraisal, cannot necessarily be relied upon as of any other date without subsequent advice of the author of this report.
- 16. The distribution of value between land, buildings and other improvements applies only in regard to the purpose and function of this appraisal, as outlined in the body of this report.
- 17. The value expressed herein is in Canadian dollars.
- 18. This report is only valid if it bears the original signature(s) of the author(s).

Extraordinary Assumptions & Limiting Conditions

Page 6

Extraordinary Assumption

Values contained in this appraisal are based on market conditions as at the time of this report. This appraisal does not provide a prediction of future values. In the event of market instability and/or disruption, values may change rapidly and such potential future events have been NOT been considered in this report. As this appraisal does not and cannot consider any changes to the property appraised or market conditions after the effective date, readers are cautioned in relying on the appraisal after the effective date noted herein.

The value derive in this report is based on the subject site being used for a purpose that meets the current "Permitted Uses', as set out in the zoning bylaws for Norfolk County.

Hypothetical Condition

None

SCOPE OF APPRAISAL

Each appraisal assignment is unique, depending upon the type and size of the property being appraised, the complexity and significance of the appraisal problem, and the intended function of the appraisal.

The appraisal that is the focus of this engagement has been discussed and defined with the client ordering the appraisal. The specific tasks and items necessary in completing this assignment included the following:

Assembly and analysis of relevant information pertaining to the property being appraised, including acquisition or listing particulars within the last three years prior to the date of this appraisal.

A final estimate of value by the relevant approach or approaches to value. An inspection of the subject properties combined and surrounding area.

Assembly and analyses of pertinent economic and market data. Inclusion of all appropriate photographs, maps and sketches. Reconciliation of the collected data into an estimate of market value as at the effective date of the appraisal.

Nothing in this report is intended as legal opinion as to the state of title. This report is prepared on the premise that the improvements do not encroach onto adjacent lands. The appraisal assumes that title is good and marketable.

I am not qualified in technical or professional matters related to improvements like legal surveys, engineering reports, the sizing and adequacy of the mechanical systems and components such as the heating, plumbing and electrical systems. No inspections were made with regards to termites. I am not qualified to inspect or identify if the subject property has a problem with rodents, small animals or insect infestation. If you have a concern with any of these mentioned items it is up to you to obtain the proper inspectors to help identify any of these problem areas.

Audits and Technical Investigations: I did not complete technical investigations such as: Inspections or engineering review of the structure, roof or mechanical systems. If there is an on-site oil tank, I have assumed that this tank is insured against leakage or spillage. Technical review of the utility servicing. Investigations into the bearing, percolation or other qualities of the soils. An environmental review, an archaeological survey or an environmental review of the property. Audits of financial and legal arrangement reported by the owner concerning any of the leases. The inspection is not to confirm building code compliance. These items are beyond the expertise of the appraiser.

Verification of Third-Party Information: The time and cost to confirm third party information can exceed a reasonable appraisal budget. Consequently, the analysis set out in this report relied on written and verbal information from primary and heresy sources, some of which we did not confirm. Except as described herein, I did not verify client-supplied information, which I have assumed to be correct. The new privacy laws limit the information that real estate agents can reveal to us.

Building size for the subject improvement was obtained from one of the following sources. A builder's floor plan, management office, listing information, regional tax office, and municipal offices or from measurements made during the inspection. Building sizes for the comparables is obtained from the same sources noted above or by using a formula that adds up the above grade room sizes from a listing and then multiplying by a factor based on the age of the building to obtain an approximate square footage.

Zoning Information: The zoning information in this report is provided for convenience only without any assumption for liability. Zoning doesn't remain constant. By-laws are amended and at times interim controlling by-laws are passed for a specified period. An attempt is made in most cases to verify zoning. Due to time constraints, it is sometimes not possible to wait for a return call from a zoning officer. It is up to you to satisfy yourself as to the specific zoning of the property. This appraisal assumes that the subject properties cannot be subdivided.

Comparable Sale Data: The sources of our data included Land Registry/Land Titles records, builder sales, local Multiple Listing Services & listing details, of comparable properties that were concluded to provide evidence of market value for the subject property. The comparable sales evidence relied on in this appraisal are believed to be from reliable sources. All of the comparable MLS listings were examined. It is not possible to inspect and research them fully. A concerted effort is made to verify the accuracy of the information contained within this report. The value conclusions are subject to the accuracy of said data.

The information used in this report was gathered from a personal inspection, which were completed on July 1, 2025 and from survey/tax notice provided by the client. During this inspection the subject site was photographed.

This report is considered to be a `Narrative Appraisal Report` in accordance with CUSPAP. The Highest and Best Use of the subject, has been considered - `As Is`.

All information used in this report is considered accurate and reliable and all normal measures, to ensure the accuracy of this information, have been undertaken.

DEFINITION OF REASONABLE EXPOSURE TIME

The estimated length of time the property being appraised would have been offered on the market prior to the `Hypothetical` sale at Current Market Value, as of the effective date of valuation. This is a retrospective estimate based upon an analysis of past events of a competitive and open market - comparable sales.

The fact that exposure time is always presumed to occur prior to the effective date of valuation is substantiated by related facts in the appraisal process; supply/demand, as of the effective date of valuation, the use of current cost information, analysis of historical sales data (after the property had been exposed and the sale has been completed) and the analysis of future income streams, estimated from the effective date of valuation.

ESTIMATE OF EXPOSURE TIME

EXPOSURE TIME

The comparable sales used in Direct Comparison Approach had exposure times of between 49 - 240 days.

Based on a current market value of \$861,000.00, research of market trends for competing properties indicates an exposure time (prior to the effective date) of 49 - 240 days; however, 4 of the 5 sales had exposure times of 166 days or less.

The "Purpose of the Appraisal" is to estimate the Current Market Value, of the subject property, a vacant parcel of industrial land, as of July 1, 2025, the date of valuation. The "Intended Use" is to assist in internal matter, only and **NOT FOR FINANCING PURPOSES.**

"Market Value" is defined by The Canadian Uniform Standards of Professional Appraisal Practice (CUSPAP) as:

"The most probable price which a property should bring in a competitive and open market under all conditions requisite to a fair and typical sale, the buyer and seller each acting prudently and knowledgeably, and assuming the price is not affected by undue stimulus."

Implicit in this definition are the consummation of a sale as of a specified date and the passing of title from seller to buyer under conditions whereby:

- 1. buyer and seller are typically motivated;
- 2. both parties are well informed or well advised, and acting in what they consider their best interests;
- 3. a reasonable time is allowed for exposure in the open market;
- 4. payment is made in terms of cash in Canadian dollars or in terms of financial arrangements comparable thereto;
- 5. the price represents the normal consideration for the property sold unaffected by special or creative financing or sales concessions granted by anyone associated with the sale."

PROPERTY RIGHTS APPRAISED

The property rights being appraised are those of the `Fee Simple`, which is defined as:

"An absolute fee; a fee without limitations to any particular class of heirs or restrictions, but subject to the limitations of eminent domain, escheat, police powers and taxation"

This does not necessarily imply that the subject or the comparable properties are free and clear of any encumbrances such as mortgages, liens, easements or rights-of-way.

According to the Tax Notice, the subject property is registered in the name of:

2712300 Ontario Inc.

IDENTIFICATION AND LEGAL DESCRIPTION

According to the Tax Notice, the subject property is known municipally as:

Luscombe Drive, Simcoe, Ontario

According to the Tax Notice, the subject property is known legally as:

WDM Con. 14, Pt. Lot 4; R.P. 37R-11237, Part 1; in the County of Norfolk, (a single-tier Municipality) Province of Ontario

ASSESSMENT AND TAXES

Assessment: \$147,000.00 (2024)

Taxes: \$ 3,554.20 (2024)

OFFICIAL PLAN

The Official Plan is defined in The Planning Act as follows:

"A document approved by the Minister of Municipal Affairs and Housing containing objectives and policies established primarily to provide guidance for the physical development of a municipality while having regard to relevant social, economic and environmental matter."

The Official Plan consists of the texts and maps describing such objectives and policies approved by the Ministry of Housing from time to time as provided in the Planning Act. The purpose of the Official Plan is to provide a formally adopted text of public policies and standards as guidelines for the future development of the community. It is a projection of anticipated growth and planning needs.

The Official Plan does not directly regulate the use of private lands and therefore must be supported by Zoning By-laws. The by-laws implement the policies established by the Official Plan guidelines. Requirements change from time to time and it is often necessary for the municipality to reassess the relevance of their plan to the supply and demand situation. There are, therefore, provisions for amendments to the Official Plan where reasonable grounds exist.

The Official Plan designates this area as `Protected Industrial`.

ZONING Page 14

A zoning by-law is a precise document used by a municipality to regulate the use of land. It states exactly what land uses are currently permitted in a community and provide detailed information such as;

- a) where building or structures may be located
- b) types of uses and dwellings permitted
- c) standards for lot size, parking requirements, building heights, side yard dimensions and setback from street

The subject property, is located in an area designated "General Industrial Zone - (MG)". The 'Permitted Uses' and 'Minimum Requirements', for the County of Norfolk, can be found in the addendum.

As the subject property is currently vacant, it is assumed the it is in conformity with the 'Permitted Uses' and `Minimum Requirements`, as set out in the zoning by-laws for the County of Norfolk.

RECENT SALES HISTORY

According to, MLS and the owner, there have been no changes in ownership within the prior 36 months.

According to MLS, there are no known current agreements for sale, options, listings or marketing of the subject property, within the last 12 months.

The subject property is located on the east side of Luscombe Drive, one lot south of Grigg Drive, near the west boundary of the Town of Simcoe, in County of Norfolk.

The immediate surround area is an industrial park, composed of improved properties and vacant land sites. The improved properties range from small size service operations to larger industrial plants to mini-storage facilities.

West of the improved properties, on the west side of Luscombe Drive, the land use changes to agricultural; but the area also includes residential developments at the corner of Queensway West and Luscombe and across Queensway West, further south.

To the east of Luscombe, along Queensway, land use also begins to change to more commercial in nature and then residential on the side streets.

Luscombe Drive is a two-way, paved, north-south roadway with open ditches and gravel shoulders, that extends from Queensway West (A.K.A. Highway #3), past the subject, ending one block north of Grigg Drive.

Grigg Drive is an east-west, paved road that extends from Luscombe Drive, for one block, ending at Park Road. Queensway West (A.K.A. Highway #3), is a major roadway, which extends from Windsor to Fort Erie and it experiences high volumes of commercial, commuter, tourist and local traffic.

Approximately 3-4 blocks east of Luscombe, Queensway West, intersects with Norfolk Street North (A.K.A. Highway #24), which is a major north-south route that eventually interests with Highway #403 and the City of Brantford.

Overall, the subject has a good location, for a future industrial use, because of the excellent surrounding road systems; Highways #3 and #24, which provide quick and direct access to other major highways – Highways #401 and #403; as well as many large urban areas.

There were no adverse influences noted at the time of inspection that would affect the marketability of the subject.

According to the information provided by the client, the subject site is a slightly irregular-shaped parcel of land with a frontage on Luscombe Drive of 290.35' and a total site area of 2.87 acres (Municipal Tax Notice).

The site is vacant and heavily covered by natural vegetation; so, the topography was difficult to ascertain, but it appears to be rolling and not completely level.

The site is contiguous to an improved industrial operation and from the block plan provided by the client, the subject's south boundary, will be defined by a proposed roadway, leading eastward and providing access to several other vacant sites, including one that backs to the subject (See Site Sketch).

As the site is currently, it is not serviced; however, when eventual development begins, it is assumed that services will be connected to the site.

The appraiser assumes that the subject property currently conforms to the zoning bylaws and regulations. There were no negative features noted at the time of this report. The subject is a suburban property, bordering on both industrial and vacant land parcels.

There are no apparent adverse soil conditions and no local indicators of potential contamination; however, the subject is contiguous to an industrial operation. With that being said this report **DOES NOT** warrant that the property is Free and Clear of any adverse soil conditions or contamination and if any are found then the appraiser has the right to re-evaluate the current market value estimate derived at in this report.

MUNICIPAL SERVICES, ROADS AND UTILITIES

It is assumed that the subject site will be fully serviced when developed and current all services are at the lot line, but not installed.

Currently the subject property can be accessed from Luscombe Drive, only, which is a two-way paved roadway with open ditches and gravel shoulders and no streetlights.

The subject property is vacant land; so, there is no building description.

The principle of "Highest and Best Use" is fundamental to the concept of value. It could be defined as:

"that particular legal land use, which at the date of appraisal, is most likely to produce, the greatest net return (money and/or amenities) over a given period of time (the foreseeable future)."

The Highest and Best Use analysis provides a focus for the choice of an appropriate valuation methodology. If a property's current use is its highest and best use, then its most probable selling price would be based on its existing use. Should the use of either the land alone, 'As If Vacant', or an alternative use of the land and improvement, 'As Improved', be deemed as the highest and best use, then the market value of the subject will likely be based on this alternative.

Is in analyzing the "Highest and Best Use" of the subject property, both for the site 'As If Vacant' and 'As Improved', the following main factors have been considered:

- Official Plan
- 2. Zoning By-Laws
- Locational Features
- 4. Lot size, shape, frontage and flankage
- 5. Municipal and private services available
- 6. Easements
- 7. Availability and cost of financing
- 8. Building and Site Improvements
- 9. Present Use
- 10. Condition of buildings (depreciation evident)
- 11. Type of operation to which the buildings are suited, adaptability to alternative use
- 12. Income Producing levels
- 13. Parking availability
- 14. Investment potential

- 15. Competition is in the area
- 16. Real Estate Market Trends
- 17. Demand

After a thorough inspection of the subject property and analysis of the foregoing factors, it is my opinion that the "Highest and Best Use" of the subject property, `As If Vacant and Unimproved` and 'As Improved' is some form of industrial use.

APPROACHES TO VALUE

Is in estimating the market value of an improved property, there may be three important factors:

- The current cost of replacing a property, including the cost of the land `As If Vacant`, less the accrued depreciation from all sources.
- ii) The value indicated by market activity including recent sales of comparable properties and the availability of similar facilities offered for sale.
- iii) The value that the property's actual or potential net income would support based on a market capitalization of that income stream.

The result of the above considerations is three basic approaches to value:

- I) The Cost Approach
- ii) The Direct Comparison Approach
- iii) The Income Approach

If all three approaches are applicable, then the result is three value estimates. These three resulting estimates are then analysed and reconciled to arrive at a final estimate of value for a property.

The subject property is a vacant parcel of industrial land; so, only the Direct Comparison Approach is relevant.

An extensive search was undertaken to locate similar types of properties, for the Direct Comparison. Not all components could be verified from the market, but the resulting analyses are considered to be reasonable and realistic.

PREAMBLE TO VALUATION COST APPROACH TO VALUE

1. Estimate the Value of the Subject Site as if Vacant and Unimproved

The land value estimate is determined by analyzing recent sales of similar parcels in the area. This Approach is referred to as the Direct Comparison Approach to Value, and is the preferred approach to valuing a site because it reflects typical buyer and seller reactions is in the market place. Through a process of adjustments for differences between the comparable sale and the subject property, each comparable sale becomes a basis for indicating the value of the site being appraised. The adjustments considered include but are not limited to; time, location, content and general utility. The time adjustment compensates for changing market conditions between the sale dates of the comparables and the effective date of valuation. The location adjustment compensates for factors such as access, proximity to major markets, land use patterns and exposure. A site content adjustment is often required since larger tracts of land tend to trade at a lesser rate per unit than small parcels. The general utility adjustment considers factors such as topography and plottage of the site and the effect these factors may have on the utility and/or potential site.

2. Estimate the Replacement Cost New of All Improvements

The Replacement Cost New estimate has been prepared using the Boeckh Building Valuation Manuals. This manual includes a wide variety of materials and figures connected with the building trades such as charts, specifications, depreciation tables and definitions.

PREAMBLE TO VALUATION (CONTINUED) COST APPROACH TO VALUE

3. Estimate the Accrued Depreciation from All Causes

Accrued Depreciation "is a measure of the loss is in utility experienced by the improvement is in its present condition as compared to the utility of the same improvement as though new and representing the highest and best use of the site." Accrued Depreciation is broken down into three classifications:

- 1. Physical Depreciation
- 2. Functional Obsolescence
- Economic Obsolescence
- 1. Physical depreciation refers to a loss is in value due to wear and tear, lack of regular maintenance and normal aging of the improvements, service systems and structural components.
- 2. Functional Obsolescence refers to a loss is in value due to an over or under improvement of the site, poor floor plan layout, outdated building materials, construction techniques and service systems.
- 3. Economic Obsolescence refers to a loss value due to an external source such as lack of conformity of land use is in an area or proximity to a noxious use.
- 4. Deduct the Estimated Accrued Depreciation from the Estimated Replacement Cost
 New to determine the Depreciated Replacement Cost of the Building Improvement.
- 5. Combine the Depreciated Replacement Cost Estimated and the Land Value Estimate as if Vacant to determine the final estimate of Market Value for the subject property by the Cost Approach to Value.

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VALUATION ANALYSIS COST APPROACH TO VALUE

As the subject is a vacant parcel of industrial land and because of the purpose of this report, the Cost Approach is not a Relevant Approach to Value.

PREAMBLE TO VALUATION INCOME APPROACH TO VALUE

The Income Approach involves an analysis of the actual or the estimated potential income of a property which is then converted into an estimate of market value by a capitalization process. The process provides an indication of the present worth of a future income stream. The rate at which the net income is capitalized is relative to the quality, quantity and durability of that income stream. Returns for a property are divided into two categories. First, there is the annual net income stream generated by the property and second, there is the residual value of the property, at the end of the investment cycle.

An investment cycle is the period of time that an investor is willing to commit capital. Generally, the investment cycle is related to the expected remaining productive life of a property and the term needed for the investor to meet objectives. Some factors that affect the determination of an investment cycle are location, market trends, the age and/or condition of the improvements, the cost of money, alternative investment vehicles and the tax implications of the property and/or the investor.

The probability of a property increasing is in value over the investment cycle is an important consideration when estimating a capitalization rate. If there is no probability of a property increasing is in value, then the income stream must be responsible for providing the full return of the investment. This situation will result is in a high capitalization rate, when other investment vehicles offer real growth potential. If an investment is expected to decrease is in value over the length of the investment, then the income stream is not only expected to provide a full return on the investment, it is also required to recapture the depreciation of the asset. Again, the capitalization rate will be higher because it includes both a return on and a return of the investment. If there is a probability of the property appreciating is in value then the capitalization rate will be low because the growth or gain is in value will allow all or part of the investments return to be achieved and there is no depreciation that needs to be recaptured.

PREAMBLE TO VALUATION (CONTINUED) INCOME APPROACH TO VALUE

1. Estimate the Annual Potential Gross Income

The Potential Gross Annual Income is an estimate of the total possible income if the property was fully and continuously rented for a full year at current market rents with no rental loss.

2. Deduct an Allowance for Vacancy and Bad Debts

Vacancy and Bad Debt Allowance may result from vacancy during remodelling, tenant turnover, or non-payment of rents by a tenant. This amount is calculated as a percentage of the gross income.

3. Effective Gross Income

The Effective Gross Income equals the Potential Gross Annual Income minus the Vacancy Bad Debt Allowance.

4. Estimate and Deduct the Annual Operating Expenses

The Operating Expenses are only those expenses directly related to the operation of the building. These expenses are divided into three classifications.

- 1. Fixed Expenses
- 2. Variable Expenses
- 3. Reserves for Maintenance and Replacement

PREAMBLE TO VALUATION (CONTINUED) INCOME APPROACH TO VALUE

4. (Continued)

- 1. Fixed Expenses include items that are not influenced by tenancy such as realty taxes and insurance premiums.
- 2. Variable Expenses include items that vary with tenancy such as utilities, management fees, heating and cooling.
- 3. Reserves for Maintenance and Replacement include items such as roof repairs, exterior plumbing, structural work, heating and plumbing equipment. This category also includes an allotment for items is in the structure and equipment with shorter economic or useful life expectancies such as stoves, refrigerators, machinery and equipment, carpets and specific building components.
- 5. Establish the Net Operating Income

This is the income remaining after deducting operating expenses from the Effective Gross Income, but before Debt Service (mortgage payments or Depreciation (Capital Cost Allowance).

6. Capitalize the Net Operating Income into an Indication of Market Value by the Income Approach

The overall Capitalization Rate used is derived by dividing the Net Operating Income by the total sale price. The rate included a return on monies invested and return of monies invested is in the wasting asset, which is the building only.

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VALUATION ANALYSIS INCOME APPROACH TO VALUE

As the subject is a vacant parcel of industrial land and because of the purpose of this report, the Income Approach is not a Relevant Approach to Value

PREAMBLE TO VALUATION DIRECT COMPARISON APPROACH TO VALUE

The Direct Comparison Approach to Value is based upon locating recent sales of similar properties that have sold within a reasonable length of time of the effective date of the appraisal. These comparable sales are then adjusted is in plus or minus percentages is in respect to the subject property to compensate for differences between the two. After completing the adjustments an indication of value for the subject property is determined.

Adjustments generally considered include time (date of sale versus date of appraisal to compensate for changing market conditions), location (proximity to downtown business core), access and egress, traffic flow levels, availability of municipal and community services, conformity and abutting land usage, economic outlook for the area, zoning bylaws, general condition of building (quality of materials, and workmanship, levels of maintenance exhibited, accrued depreciation evident, style, age, size, gross income), site sizes and shapes, availability of parking, landscaping, Real Estate trends (supply and demand), and any motivational factors affecting the sales. After a thorough analysis of the foregoing factors an estimate of Market Value for the subject property is determined.

This Approach is most reliable when there is adequate Market data available, for both comparison purposes, but also to for adjustment purposes.

Five comparable sales and one current listing, for the subject property – vacant industrial land, were considered relevant and have been included in this analysis. The sales all occurred between September and November 2024; while the listing is current and has been on the market for 90+ days. These comparables are considered the best and most recent comparables for the subject property, available at this time.

The sales are all located in urban areas of varying sizes and are all considered to be superior in location, except the listing with considered similar/comparable.

The comparables sold for between \$425,000 and \$2,500,000, with site areas between 1.00 and 6.04 acres. The comparables' site areas encompass the subject and they sold on a price per acre from \$175,000 to \$777,778.

Comparable #3 is the third most recent sale and the most similar in site area, but it is far superior in location and zoning, includes a quality leased residence and also get income from a Bell tower lease. Because of the afore-mentioned reasons, this sale is far superior to the subject, per acre.

Comparable #4 is the second oldest sale. This sale is larger in site area, but it is immediately ready for development and because of its proximity to the new VW battery plant, far outweigh the larger site area, making this sale superior to the subject, per acre.

Comparable #1 is the most recent sale and it is larger in site area. This sale has a similar development time and is considered to be overall inferior in location. The larger site area and overall inferior location, makes this inferior to the subject property, per acre.

Comparable #5 is the oldest sale and is considered to be comparable in location. This sale is smaller in site area, superior in development time and is improved with paved parking and a fenced compound.

The smaller site area, superior development time and current site improvements, makes this sale superior to the subject property, per acre.

Comparable #1 is considered to be inferior to the subject property, per acre and Comparable #5 is considered to be superior to the subject, per acre. These two comparable set a range of between \$201,471 and \$425,000, per acre; which is a relatively wide range.

DIRECT COMPARISON APPROACH TO VALUE (CONTINUED) Page 30

Because of the subject's location, site area, site readiness (development lag time), and zoning, a current market value estimate, for the subject property, near the mid-point of this value range, is considered reasonable.

After making reasonable adjustments, it is my opinion that the current market value of the subject property, as of July 1, 2025, is;

---THREE HUNDRED THOUSAND DOLLARS PER ACRE--- (\$300,000.00 per acre)

Therefore;

2.87 acres X \$300,000.00 per acre = \$861,000.00

---EIGHT HUNDRED AND SIXTY-ONE THOUSAND DOLLARS--(\$861,000.00)

SUMMATION AND FINAL ESTIMATE OF VALUE

Page 31

COST APPROACH TO VALUE:

Not Applicable

INCOME APPROACH TO VALUE:

Not Applicable

DIRECT COMPARISON APPROACH:

\$861,000.00

The Cost Approach to Value is best for newer buildings, where estimates for replacement cost and depreciation are easier to determine, and where vacant land values can be obtained. As the subject is a vacant parcel of industrial land and for the purpose of this report, the Cost Approach is not a Relevant Approach to Value.

The Income Approach to Value is best when recent, reliable and accurate figures can be obtained for income and expenses. As the subject is a vacant parcel of industrial land and for the purpose of this report, the Income Approach is not a Relevant Approach to Value.

The Direct Comparison Approach is best when a reasonable number of similar properties, that have recently sold, can be found. The comparables that were used are considered to be average to above-average. This approach to value is the only relevant approach and will be used as the basis for the final Current Market Value estimate for the subject property.

Therefore, after making reasonable adjustments, it is my opinion that the final estimate of the Current Market Value, of the subject property, as of July 1, 2025, is;

---EIGHT HUNDRED AND SIXTY-ONE THOUSAND DOLLARS--- (\$861,000.00)

I hereby certify:

- 1. that I personally inspected the subject property on July 1, 2025
- 2. that I have no present financial or contemplated financial interests is in the property appraised
- 3. that I have no personal interest or bias with respect to the subject matter of this appraisal report or the parties involved
- 4. to the best of my knowledge and belief the statements of fact contained is in the appraisal report, upon which the analysis, opinions and conclusions expressed herein are based, are true and correct, subject to the stated underlying assumptions and limiting conditions
- 5. that no pertinent or important facts relevant to the value of the property appraised have been withheld from this report
- 6. this appraisal report sets forth all the limiting conditions (imposed by the terms of my assignment or by the undersigned) affecting the analysis, opinions and conclusions is in this report
- 7. that the compensation for rendering my opinion of value as outlined in this appraisal report is not contingent upon the final estimate of value
- 8. that my analyses, opinions and conclusions were developed, and that this report has been prepared is in conformity with the Canadian Uniform Standards of Professional Appraisal Practice
- 9. that no one provided significant professional assistance to the person(s) signing this report.
- 10. it was my opinion that the estimated Current Market Value of the subject property, as if free and clear of any mortgage financing, and based on all cash transactions, as of July 1, 2025, being the date of valuation, is;

---EIGHT HUNDRED AND SIXTY-ONE THOUSAND DOLLARS--- (\$861,000.00)

Respectfully submitted MOIR LUFT REAL ESTATE APPRAISAL SERVICE LTD.

Non A (Sarky) moin

R. A. (SANDY) MOIR, AACI, P. App., H.B. Comm

Date Signed: July 11, 2025

ROSS A. (SANDY) MOIR, P. App., AACI, H.B. COMM. Moir Luft Real Estate Appraisals Services Ltd. 37148 Bush Line, R.R. #2 Pt. Stanley, Ontario N5L 1J2 Telephone: (519) 769-1234

E-mail: mlappraisals@hotmail.com

PROFESSIONAL AFFILIATIONS:

Member of the Appraisal Institute of Canada since 1991 Member of the Ontario Expropriation Association since 2011

EDUCATION:

UNIVERSITY - Graduated from the University of Guelph in May 1989

with an Honour Bachelor of Commerce degree.

Curriculum included courses in Economics, Accounting,
 Finance, Building Design, Data Entry and Cost

Evaluation.

A.I.C. COURSES - Have successfully completed all of the three appraisal

specific courses. The first course dealt with residential and income properties using mainly the Direct Sales Approach to Value. The second course dealt more with the use and understanding of the Income Approach to Value. The third course dealt with a more in depth

analysis of the Income Approach to Value.

BACKGROUND:

CITY PLANNING - Worked as a student planner for the St. Thomas

Elgin County Planning Office from May 1981 -

September 1981.

<u>JUNIOR APPRAISER</u> - Have worked as a Junior Appraiser for the firm of

J.V. Luft & Associates from September 1990 to

December 2001.

QUALIFIED FEE APPRAISER - December 5, 2001 - Present

GENERAL EXPERIENCE:

Experience Includes: Real Estate appraisals for Expropriation, Valuation Day, Legal Separations, Mortgage purposes, Assessment Appeals, Fire Insurance, Estates and Market Value appraisals for general purposes.

<u>Appraisal Assignments Include:</u> Residential properties, single-family dwellings and multi-family dwellings, condominium reserve fund studies, agricultural operations including; farms and hobby farms, vacant land, subdivisions of varying sizes, industrial and commercial properties including; hotels, motels, restaurants, bars, golf courses, aggregate operations, recreational properties, marinas, municipal buildings, institutional uses and railways. Properties are generally located in Midwestern and Southwestern Ontario.

Appraisals Have Been Performed For:

Law Firms Ministry of Transportation Ontario

Individual Property Owners Churches

Finance Companies Ministry of Defence

All Chartered Banks Local Builders and Developers

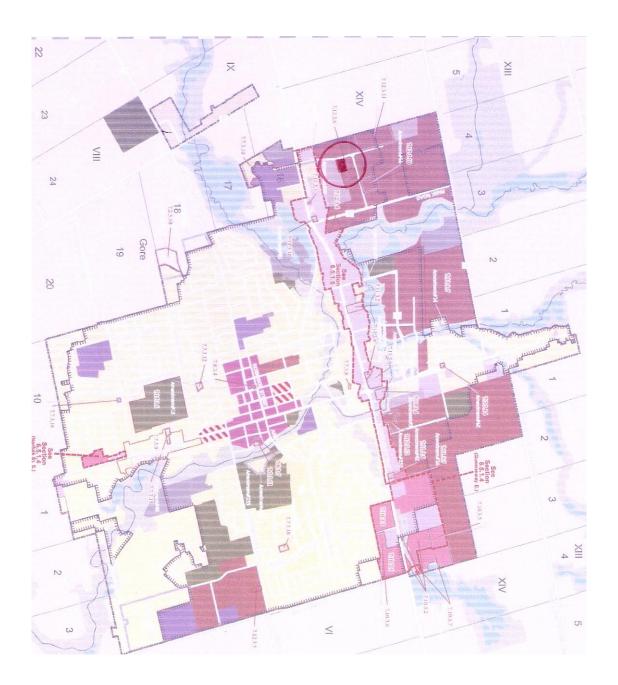
Smaller Lending Institutions Many Towns and Municipalities in Southwestern Ontario

Insurance Companies Department of Public Works

Ontario Hydro City of St. Thomas
Chartered Accountants Local Townships

Local and Out of Town Business Conservation Authorities and Industries Condominium Corporations





Industrial Zones 7.0

General Industrial Zone (MG) 7.1

7.1.1 Permitted Uses

In an MG Zone, no land, building or structure shall be used except in accordance with the following types of uses:

- ambulance service
- animal hospital b)
- auction centre c)
- bus terminal d)
- call centre
- Cannabis Production and Processing, subject to General Provisions 3.21 [25-Z-2018]
- construction shop g)
- contractor's yard
- crematorium i)
- fire hall j)
- food processing, excluding abattoir k)
- general material manufacturing 1)
- graphics and design
- industrial supply
- material processing, excluding asphalt plant, cement works and concrete 0) batching
- merchandise service shop p)
- office, industrial, accessory to a permitted use q)
- personal and health services for employees, accessory to an industry on the same lot
- research and development facility S
- retail sales accessory to an industry on the same lot t)
- storage u)
- taxi terminal V)
- w) telecommunications and data processing
- trade school X)
- transportation y)
- veluicle services and repair, including automobile body shop and industrial z) garage
- wholesale outlet.

High Requirement for Municipal Public Water and Sanitary Sewage Treatment 7.1.2

Notwithstanding Subsection 7.1.1, uses with a high requirement for municipal treatment in either strength or quantity of waste, or a high volume requirement of municipal water shall not be permitted.

Exemption for Certain Industries with a High Requirement for Municipal Sewage Treatment

Any manufacturing or processing plant existing at the date of passing of this By-Law which has a high requirement of municipal sewage treatment in either strength or quantity of wastes or a high volume requirement of municipal water shall be allowed to enlarge or expand provided the enlargement or expansion does not constitute a significantly higher level of municipal sewage treatment or water requirements.

7.1.4 Zone Provisions

In an MG Zone, no building or structure shall be erected or altered except in accordance with the following provisions:

a) b) c) d) e) f) g)	minimum lot area: minimum lot frontage: minimum front yard: minimum exterior side yard: minimum interior side yard: i) abutting a residential Zone minimum rear yard: maximum building height:	1,855 square metres 30 metres 6 metres 6 metres 20 metres 9 metres subject to a 45 degree angular plane measured from the edge of any residential, commercial or institutional Zoned lots
----------------------	--	---

outdoor storage:

prohibited in any front yard or any required

exterior side yard

7.1.5 Retail Sales Accessory to an Industry

Retail sales accessory to an industry shall be permitted up to 15 percent of the floor area of an industry to a maximum of 150 square metres of usable floor area provided the retail sales area is separated from the principal industrial or warehouse use by solid partition walls.

Additional Zone Provisions for Crematoriums [6-Z-2018]

Notwithstanding certain Zone provisions in Subsection 7.1.4, the following additional provisions shall apply to a crematorium:

2 hectares minimum lot area: a) minimum setback to any

residential zone, school, community centre or place of

recreation:

200 metres

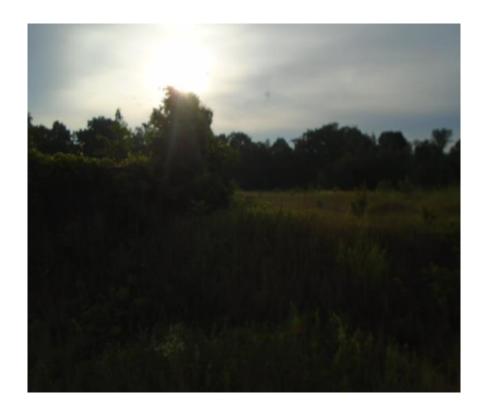




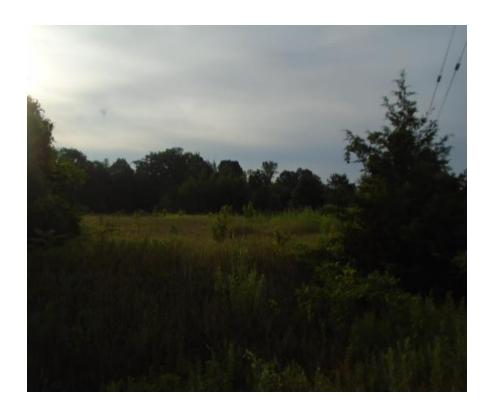
DIRECT COMPARISON CHART - SALES

COMP.	LOCATION	DATE OF SALE	SELLING PRICE	SITE AREA	S. P PER ACRE	COMMENTS	
1.	White Street, Blk. 2 Aylmer	11/15/24	\$1,027,500	5.10 acs.	\$201,471	- larger lot in a newly developing industrial park area in Aylmer - inferior location, services at the lot line, similar development time - corner lot, 122-day exposure time	
2.	1 Innovation Way Tillsonburg	10/28/24	\$1,057,000	6.04 acs.	\$175,000	- larger lot in a newly developing industrial park area in Tillsonburg - similar location, shovel ready, quicker development time, corner lot - sold privately, 49-day exposure time	
3.	3050 Dingman Drive London	10/22/24	\$1,680,000	2.16 acs.	\$777,778	- slightly smaller site area, fully serviced, site includes an existing residence - far superior location, site includes bell tower also bringing in rental income - zoning is commercial and is next to large commercial operations - 97-day exposure time, single frontage	
4.	10 Highbury Avenue South Lot 1 St. Thomas	10/21/24	\$2,500,000	5.00 acs.	\$500,000	- larger lot in a well-established industrial area, full services at the lot line - immediate development, across the road from the new VW battery factory - far superior location, adjacent to an improved site and vacant lots - single frontage, 240-day exposure time	
5.	99 Townline Road Tillsonburg	09/04/24	\$425,000	1.00 acs.	\$425,000	- smaller lot in an older, established industrial area in Tillsonburg - similar location, full services at the lot line, similar development time - existing paved and fenced parking area, adjacent to existing improved sites - 166-day exposure time, single frontage	
6.	33 Park Road Simcoe	Active Listing	Asking Price \$524,900	0.80 acs.	Asking Price \$656,125	- much smaller lot, located one block to the east of the subject - overall similar/comparable location, established industrial area - full services at the lot line, superior development time, cleared site - adjacent to existing improved sites, has been on the market for 90 days+	

SUBJECT PHOTOGRAPHS



APPROXIMATE NORTH BOUNDARY OF THE SUBJECT SITE - LOOKING EAST



SUBJECT SITE - LOOKING SOUTHEAST

SUBJECT PHOTOGRAPHS



LUSCOMBE DRIVE - LOOKING SOUTH



APPROXIMATE SOUTH BOUNDARY OF THE SUBJECT SITE - LOOKING EAST

SUBJECT PHOTOGRAPHS



SUBJECT SITE - LOOKING NORTHEAST



LUSCOMBE DRIVE - LOOKING NORTH

FUNCTIONAL SERVICING REPORT PROPOSED MANUFACTURING BUILDING SIMCOE, ONTARIO

Norfolk County

Prepared By:

J.H. Cohoon Engineering Limited 440 Hardy Road, Unit 1 Brantford, Ontario N3T 5L8 Phone (519) 753-2656 Fax (519) 753-4263

Job: 17233 September 2025

September 2025

INTRODUCTION

The following Functional Servicing Report was prepared by J.H. Cohoon Engineering Limited for Tony Kounty in support of planning applications relating to the site located on Luscombe Drive in the Town of Simcoe, Ontario in Norfolk County. This report was prepared to demonstrate the servicing scheme for the proposed industrial development that is to occur on the subject lands.

The development approach is to develop the site in a single-phase two phase development located on the east side of the Luscombe Drive in the Town of Simcoe. The development consists of two 464.6 sq.m. industrial buildings to be constructed in two phases.

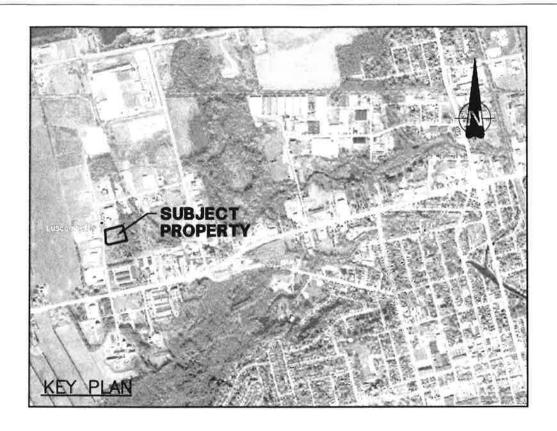
The site is located on the east side of Luscombe Drive north of the intersection of Luscombe Drive and Highway 3 within the Town of Simcoe, in Norfolk County. The overall subdivision area is 1.154 Ha in size

The objective of this report is to document the servicing strategy to be utilized for the site in a proposed initial development. The proposal is to have full municipal services will be installed within the development and with connections to the existing municipal system in the existing municipal road allowance as applicable. The owner will assume full responsibility for the installation and maintenance of the services on the property.

PROPOSED DEVELOPMENT CONCEPT

The proposed development is to be constructed on the east side of Luscombe Drive in the Town of Simcoe, Ontario. As indicated the parcel of land is some 1.154 hectares in size. A key map illustrating the site location is provided in Figure 1.

The anticipated development is intended to be a two phase industrial development consisting of two 464.6 sq.m. industrial buildings (constructed in two phases) on the property. The development is illustrated on the plans prepared by J H Cohoon Engineering Limited being drawings which have been included within Appendix 'A' of this report



Site Location – Key Plan Figure No. 1

SANITARY SEWERS & APPURTENANCES

Design Flows

This site is proposed to be constructed on full services extending from the existing municipal services located in front and adjacent to the site.

The proposed servicing plans are shown generally on the engineering plans included within Appendix 'A' of this report.

The design of the system(s) was carried out in accordance with the requirements of the Ontario Building Code including / based upon the following assumptions for the design of the system

Industrial Component

2 – 464.6 sq.m. industrial buildings
Anticipated occupant load is 5 persons per building
The average daily flow the average daily flow is based upon 75

litres per person per day\

 $2 \times 5 \times 75 = 750$ litres per day

= 0.0087 litres per second

Total Average Design Flow = 0.0087 litres per second.

Within the Norfolk County Design Criteria, the anticipated sewage flows are to be based upon 55 cu.m. / hectare / day. In this case, that would correspond to 63.47 cu.m. per day which is substantial on the basis of the proposed development. In this case, we have estimated the occupant load for the building at 5 per building which is a reasonable estimate of the sanitary effluent from this site.

On the basis of the Harmon Peaking Factor for the industrial, and a total population for this site being 10 persons, the peaking factor of 3.532 (between 2 and 5.0) was applied resulting in a peak design flow for this building being 0.031 litres per second.

This was calculated on the basis of the following

$$Me = 0.80 * (1 + (14 / (4 + Pe^{0.5})))$$

Where the equivalent population is in the order of 10 persons or Pe=0.01 which results in a peaking factor of 3.532

With the consideration of Infiltration on this site as follows:

Site Area = 1.154 hectares \pm /-

Infiltration Rate = 0.28 litres per second per hectare

Infiltration Allowance

= 0.3231 litres per second

Summary of Results

Average Flow Rate (including Infiltration) = 0.332 litres per sec Peak Flow Rate (including infiltration) = 0.354 litres per sec

WATER SERVICING AND FIRE PROTECTION

The provision of water to the proposed buildings will be provided through the provision of water services to the subject lands in accordance with the requirements of the Ontario Building Code.

We note that the site is also serviced with on site municipal water and local fire hydrants.

The domestic demand for would be similar to the sanitary sewer effluent calculations detailed in the Section on Sanitary Sewer Design flows and would be as follows:

2 - 464.6 sq.m. Industrial Bldg. Average Daily Flow Rate Peak Daily Flow Rate

(Litres per second)

(Litres per second)

0.0087

0.0158

As noted in the Norfolk County design criteria, a peaking factor of 2.25 was used for the calculation of the peak flow rate.

The proposed fire protection to this development will be handled by the existing fire hydrants adjacent to the property.

Utilizing the requirements of the Fire Underwriters Survey 2020, the following outlines the water demand for the overall building area of the subject building.

The analysis was carried out on a typical building on this site to determine the maximum fire demand for each of the buildings on the site.

This largest building in this development would be a 464.6 sq.m. industrial facility (1 story). Therefore, in this case, a building area of 464.6 sq.m. is being considered in the calculations.

Utilizing the Fire Underwriters Survey Document, our estimation of the required fire demand is as follows:

Estimate of Fire Flow Required

220 * C * SQRT (A)

Where C

Coefficient related to type of

Construction

Ordinary Construction

1.5

A = Total Area of the Building (As outlined above) 464.6sq. m.

= 220 x 1.5 * SQRT (142.2)

= 7,113.01 litres per m

Rounded

= 7,000 litres per min

Modifications

Occupancy Normal Hazard Occupancy

-15%

= -1050 litres per min

Net Fire Demand = 5,950 litres per min

Further Modifications Automatic Sprinkler

System = 0%

No Reduction

Spatial Exposure (Estimated)

 North
 >30 m
 + 0%

 East
 >30 m
 + 0%

 West
 Street
 + 0%

 South
 10.0 m
 + 15%

 Total
 + 15%

Increase

892.5 litres per min

Total Fire Demand 6,842.5 litres per min

Or 114.0 litres per sec.

STORM SEWERS & APPURTENANCES

Storm Sewers / Storm water Management

The site is intended to be serviced through the existing storm sewer system located on Luscombe Drive in the Town of Simcoe, Ontario adjacent to the site. The site is intended to provide its own stormwater management controls on the property to reduce the impact of the site on the existing drainage system. The

overflow from this site will be directed into the existing storm sewer that is located adjacent to the property within the Luscombe Drive right-of-way.

Pre-development Condition

The site presently drains in an easterly direction towards the rear of the property onto the abutting neighbour's lands. The site is with municipal storm sewers which are to be designed to handle the 5-year storm event. The overall stormwater management system is to be consistent with the current policies of Norfolk County whereas the proposed development is required to reduce the runoff in the post development flows to below the pre-development rates for all storm events up to and including the 100-year event.

The analysis is carried out utilizing the Norfolk County rainfall IDF curves shall be based on the following local rainfall data for Norfolk County:

$i = \underline{a}$ $(t + \underline{a})$	b)c	mm/hr		
Storm Event	A	В	C	
5	583.017	3.007	0.703	
10	670.324	3.007	0.698	
25	721.533	2.253	0.679	
50	766.038	1.898	0.668	
100	801.041	1.501	0.657	

The following table outlines the pre-development runoff for the site.

Pre-Development Runoff Results

Design Storm	Pre-Development Runoff Rate (cms)
5-year storm	0.074
10-year storm	0.117
25-year storm	0.180
50-year storm	0.224
100-year storm	0.267

The flows outlined above were calculated utilizing the MIDUSS Stormwater Simulation Program (Ver 2). The computer results are included within the Appendix 'B' of this report.

Post-Development Condition

With the development of the lots, and the associated paved areas, an increase in the runoff will occur on this site. The results are detailed in the Appendix 'C' of this report. The same IDF curves were utilized in the analysis as the predevelopment condition.

Post-Development Runoff Results

Design Storm	Pre-Development Runoff Rate (cms)	Post Development Runoff Rate (cms)		
		W/O SWM	W/Swm	
5-year storm	0.074	0.251	0.065	
10-year storm	0.117	0.299	0.103	
25-year storm	0.180	0.360	0.135	
50-year storm	0.224	0.406	0.182	
100-year storm	0.267	0.448	0.225	

The flows were calculated utilizing the MIDUSS Stormwater Simulation Program (Ver 2). The computer results are included within the Appendix 'B' of this report.

GRADING

Road grades will be established for the proposed development and are illustrated on the plans appended to the report. Minimum (0.50%) and maximum (6.0%) grades have been used in accordance with Norfolk County design criteria.

UTILITIES

Coordination of these services will be required with Union Gas, the hydro utility, Bell, and the local cable tv provider (if available)

CONCLUSIONS

The preceding sections of this report outline the servicing and grading requirements for the proposed industrial development on this site. Based on the work completed to date, it may be concluded that the proposed development may be developed with full municipal services.

Report Prepared By:

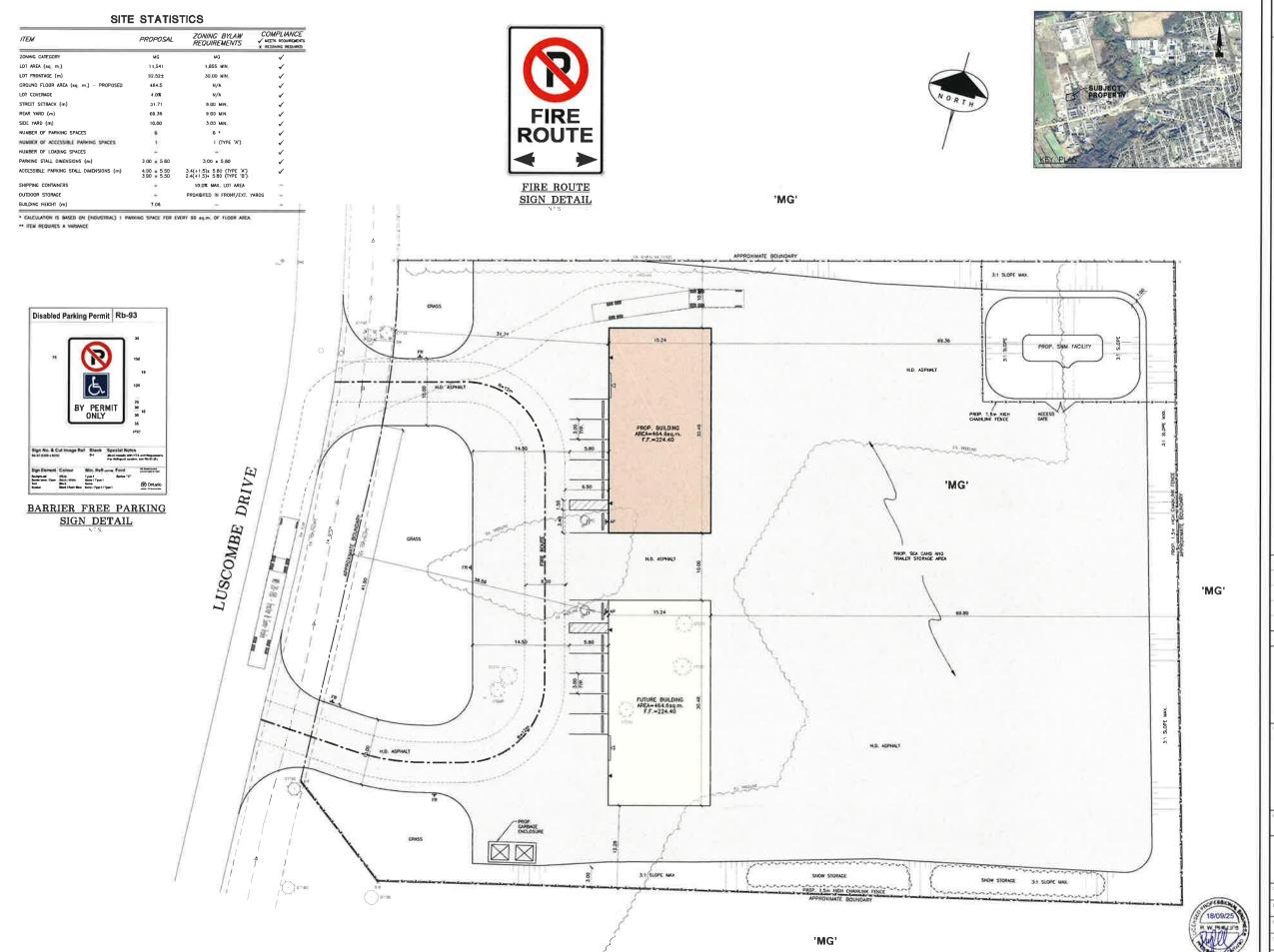
J.H. COHOON ENGINEERING IN 1910

8 99 05

R.W. Phillips, P.Eng

September 2025

Appendix 'A'
Proposed Industrial Development Engineering Plans
as prepared by J H Cohoon Engineering Limited



THE POSITION OF POLE LINES, CONDUITS, WATERMAINS, SEMERS AND OTHER UNDERGROUND AND ABOVERROUND UTILITIES AND STRUCTURES AND INTER UNDERGROUND AND ABOVERROUND UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE CONTRACT DRAWNES, AND WHERE SHOWN, THE ACCUMACY OF THE POSITION CONNECTION OF THE CONTRACTOR SHALL FAMILIARIZE PHASEIS OF THE EXACT LOCATION OF ALL SHALL INTIMES AND STRUCTURES OF THE EXACT LOCATION OF ALL SHALL INTIMES AND STRUCTURES.

LEGEND:

- DENOTES MANDOOR LOCATIONS
- DENOTES OVERHEAD DOOR LOCATIONS DENOTES ACCESSIBLE PARKING SIGN
- FR DENOTES FIRE ROUTE SIGN

CX THEES TO HEM

DE THEES

EX. THEES TO BE HEMOVED

NOTES:

1 ALL ELEVATIONS SHOWN ARE METRIC

2. BUILDER/OWNER TO VERIFY COMPLIANCE WITH ZONING BYLAWS (In. SIDEYARDS, SETBACKS, REARYARDS ETC.)

AS PER COUNTY COMMENTS 08/01/25 K.P.E



J.H. COHOON
ENGINEERING
LIMITED
CONSULTING ENGINEERS

440 HARDY ROAD UNIT #1 , BRANTFORD — ONTARIO N3T SLB TEL (519) 753-2656 FAX (519) 753-4263 www.cohooneng.com

PROPOSED

MANUFACTURING BUILDING
LUSCOMBE DRIVE, SIMCOE
NORFOLK COUNTY

TONY KOUNTY

SITE PLAN

DRISCA R.W.P. 50% 1:250

DRINK SLM./K.P.B. 17233

R.W.P. 1 of 4

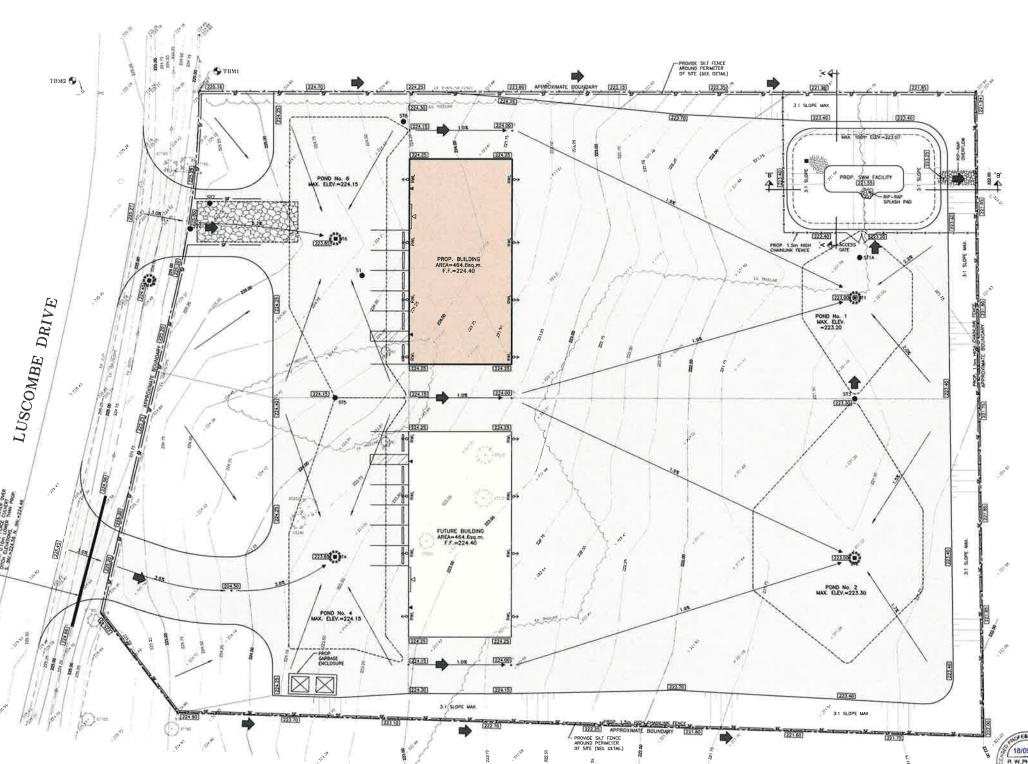
DRICCA 1 T233—1

SILTATION & EROSION CONTROL NOTES:

- 2 ALL SEC MEASURES ARE TO BE IN PLACE PRIOR TO COMMENCEMENT OF CONSTRUCTION
- 3 OWNER/CONTRACTOR TO MAINTAIN EROSION CONTROL MEASURES THROUGHOUT SITE UNTIL A COMPLETE GRASS/VEGETATION COVER IS ACHIEVED.
- 4. ONLY AT THE DIRECTION OF THE ENGINEER ARE THE SEC MEASURES TO BE REMOVED.
- 5 SITE WORKS ARE TO BE STAGED IN SUCH A MANNER THAT EROSION WILL BE MINIMIZED.
- 6 CLEARING AND CRUBBING OF THE SITE SHOULD BE KEPT TO A MINIMUM AND VEGETATION REMOVED ONLY IN ADVANCE OF IMMEDIATE CONSTRUCTION.







LEGEND:

EXISTING ELEVATIONS 200 00 PROPOSED ELEVATIONS 200.00 200.00 PROPOSED DITCH ELEVATIONS PROPOSED PRE-ASSEMBLED SILTATION CONTROL FÉNCING SET SACK AS SHOWN

EX TREES TO REMAIN

EX. TREES TO BE REMOVED

LIMIT OF S.W.M. POND (100 Yr. EVENT) MAX POND ELEVATION AS NOTED

NOTES:

- BUILDER/OWNER TO VERIFY COMPLIANCE WITH ZONING BYLAWS (ie. SIDEYARDS, SETBACKS, REARYARDS ETC.)
- TOPOGRAPHIC SURVEY PROVIDED BY JOETOPO SURVEYS AND CADD INC. DATED NOV. 14, 2024.

T.B.M. No. 1 ELEV = 225.25m

T.B.M. No. 2 ELEV. = 226.10m

AS PER COUNTY COMMENTS 08/01/25 K.P.E



J.H. COHOON ENGINEERING LIMITED CONSULTING ENGINEERS

440 HARDY ROAD , UNIT #1 , BRANTFORD - ONTARIO , N3T 5LB
TEL (519) 753-2556 FAX (519) 753-4263 www.cohooneng.com

PROPOSED MANUFACTURING BUILDING LUSCOMBE DRIVE, SIMCOE NORFOLK COUNTY

TONY KOUNTY

GRADING AND SILTATION & EROSION CONTROL PLAN

	R.W.P.	1:250		
DRIME	S.L.M./K.P.B.	-01 tu		
CHEMIN:	R.W.P.	17233		
5=(/-	2 of 4	DEC. 59.		
DATE	MAR 20/25	17233-2		

STORM SYSTEM					
DESCRIPTION	1/6	INVERTS.			
STORMCEPTOR MODEL	223 15	N 221 58 S 221 61			
1 2mø P/C CB/MH	223 00	N 221 63 S 221 66			
0.6±0.6±1.52m P/C CB	223.00	N 221 83			
1.2m# P/C MH	223_30	N 221 72 S 221 74 W 221 76			
0.6x0.6x1.83m P/C CB	223 85	N 222 37			
1 2m# P/C MH	224_15	E 222 07 N 222 10 S 222 10			
0.6x0.6x1.67m P/C CB	223.85	5 222.57			
1.2m# P/C MH	224 95	E 221 12 W 221 09			
1.2m# P/C MH	224 20	W 221 25 E 221 28			
0.6x0.6x1.98m P/C DICB c/w 4:1 SLOPED GRATE	224 40 (LOW)	N 222 90			
	### ### ##############################	### ACSCRPTION 1/6			

WATERMAIN CROSSINGS						
LOCATION	STM INV	STM OBV.	SAN. INV.	SAN, OBV.	W/M INV. AT CROSSING ONLY	W/M OBV. AT CROSSING ONLY
11			221 .77	222 02	223,56	223 60
2	220 96	221 .71			223,56	223 60
3	222 20	222 45			223, 36	223 40
4	222 42	222 67			221 86+	221,90+
5			222 34	222_49	221_80+	221 ,84+
6	222 06	222 44			221 52+	221 56•

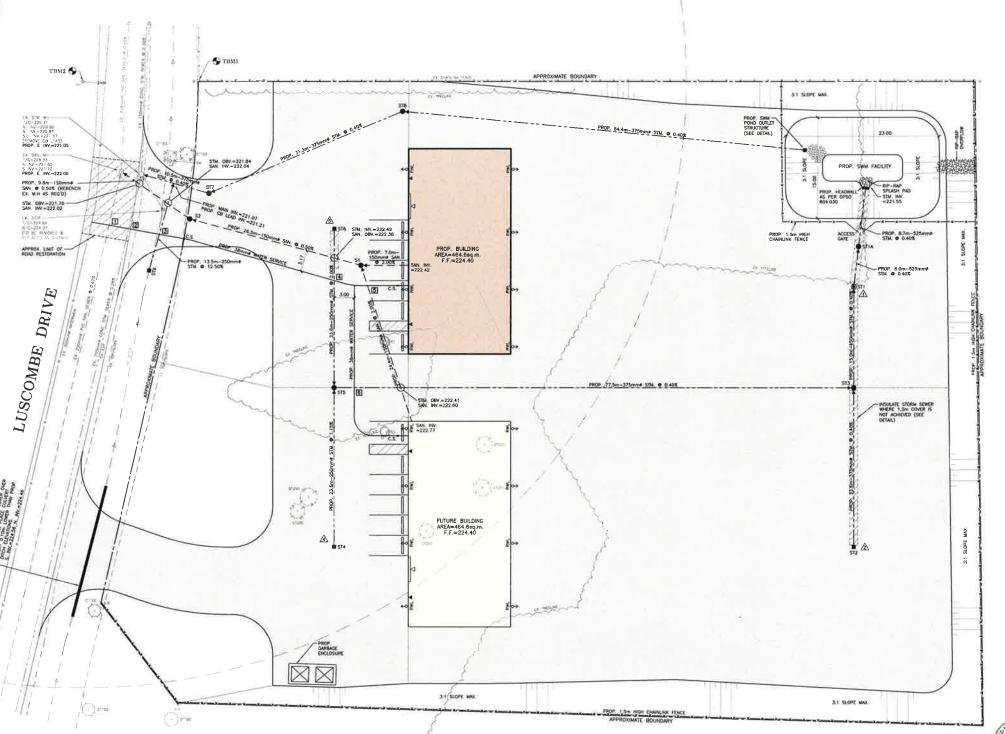
* DENOTES LOCATION WHERE WATERMAIN IS TO BE LOWERED BELOW STORM SEWER (ONLY) OR SANITARY SEWER (ONLY) PROVIDE 0.50 MIN CLEARANCE — WATERMAIN LELVATIONS SHOWN ARE THE LOWERED ELEVATIONS OF INDICATED LOOSING LOCATIONS)





S	ANITARY	SYSTI	EM
MN No.	DESCRIPTION	1/0	INVERTS
51	1.2mø P/C MH	223 95	W 222 2 E 222 26 S 222 26
52	1.2m@ P/C MH	225 .05	W 222 06 E 222 09

ROAD RESTORATION TO BE A MINIMUM CAP-40mm HLB ASPHALT, SORTH HLB ASPHALT ISBIRM GRAN, "A, JOOMM GRAN "B' (OR MATCH EXISTING) -ALL TRENCHED AREAS TO BE BACKFILLED WITH IMPORTED GRAVILLAR "A" MATERIAL COMPACTED TO 100% SPINDD TO SUBGRAD -ALL MATCH LIMITS ADMACENT TO EXISTING



THE POSITION OF POLE LINES, CONQUITS, WATERMAINS, SEWERS AND OTHER OUDERFROUND AND ABOYCROUND UILLIES AND STREET OF THE CONTRICT OF THE CONTRI

LEGEND:

S EXISTING SANITARY MANHOLE

ST EXISTING STORM MANHOLE

EXISTING CATCHBASH

♦ EXISTING SATCHBASH

♦ EXISTING WAYE, & BOX

143 EXISTING WAYE, & BOX

143 EXISTING WAYER BOX

■ ST PROPOSED SANITARY MANHOLE

PROPOSED SANITARY MANHOLE

PROPOSED STORM MANHOLE

■ ST1 PROPOSED CATCHBASH MANHOLE

■ ST1 PROPOSED CATCHBASH MANHOLE

■ CO PROPOSED CATCHBASH MANHOLE

■ CO PROPOSED CATCHBASH MANHOLE

■ ST1 PROPOSED CATCHBASH MANHOLE

■ ST1 PROPOSED CATCHBASH MANHOLE

■ ST1 PROPOSED CATCHBASH MANHOLE

■ CO PROPOSED CATCHBASH MANHOLE

PROPOSED RAIN WATER LEADER

T.B.M. No. 1 ELEV. = 225,25m (GEO)

T. B.M. No. 2 ELEV. = 226,10m (GEO)

70 NUT OF FRE -YORKY WSS 50 OF LISCOVER DRIVE AS

50 DW (TAKEN IF S. 429 AN 7, 7072)

1 AS PER COUNTY COMMENTS 08/01/25 K.P.



J.H. COHOON
ENGINEERING
LIMITED
CONSULTING ENGINEERS

440 HARDY ROAD , UNIT #1 , BRANTFORD — ONTARIO , N3T 5L8 FEL (519) 753-2856 FAX (519) 753-4263 www.cohooneng.com

MOJ CI

PROPOSED

MANUFACTURING BUILDING
LUSCOMBE DRIVE, SIMCOE
NORFOLK COUNTY

TONY KOUNTY

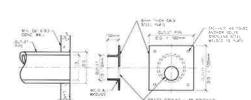
SERVICING PLAN

DES-CN	R.W.P.	1:250		
ZWARC	S.L.M./K.P.B.	,00 No		
DHENTS	R.W.P.	17233		
5r(E).	3 of 4	Date Au		
DATE	MAR. 20/25	17233-3		

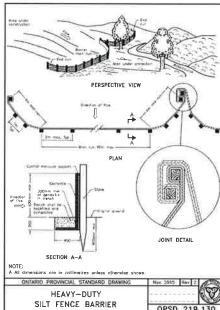
ROADWORKS:

- CATCH BASIN CONNECTIONS TO BE PVC PIPE CSA B182.4, SOR 35 OR 'ULITHA RIB' SNGLE CATCH BASIN LEADS TO BE NIN, 250mm DIA. DÖUBLE CATCH BASIN LEADS TO BE MIN. 300mm DIA. REAR LOT CATCHBASIN LEADS AND DITCH INLET LEADS TO BE CSA A2571 EXTRA STRENGTH CL3 CONC. 300mm DIA. AT 1 DX MIN.
- 3. SHIGLE CATCH BASINS AS PER OPED 705 DID FRAME AND COVER AS PER OPED 400 SID
- 4 FINAL HOADWAY CHOSSFALL TO BE 2 OK
- 5 VALVES, MANHOLES AND CATCH BASINS SHALL BE PLACED AT ASPHALT BINDER COURSE (HL4) ELEVATION AS DIRECTED BY THE ENGINEER.
- 6 AT SAG POINTS, CATCH BASIN ADJUSTMENT AND PAVING TO BE PLACED IN SUCH MANNER THAT WILL NOT OBSTRUCT DRAINAGE.
- 8 FOR MANHOLE AND CATCH BASIN TOP ADJUSTMENTS, ALL PERMANENT ADJUSTMENTS ARE TO BE POURED IN PLACE OR APPROVED EQUIVALENT (e.g., MODULOC)
- 9. ALL BEDDING AND BACKFILL MATERIAL, ROAD SUB-GRADES AND GRANULAR ROAD BASES SHALL BE COMPACTED TO MIN 100% SPD UNLESS OTHERWISE SPECIFIED.
- 10. SILTATION CONTROL BARRIERS SHALL BE PLACED AS DETAILED ON THE SILTATION AND EROSION CONTROL PLAN

C.B. ORIFICE PLATE SIZING 175mm



FLOW CONTROL DEVICE ORIFICE PLATE



GENERAL NOTES:

- CONSTRUCTION OF SEWERS, WATERMAINS AND RELATED APPURTENANCES SHALL BE UNDERTRAKEN IN ACCORDANCE WITH THE CURRENT STANDARD DRAWNOS OF THE COUNTY OF HORFICKL AND THE CONTRAID PROVINCIAL STANDARDS DRAWNOS (OPSD). THE COUNTY OF HORFICKL DRAWNOS SHALL TAKE PRECEDENCE OVER THE OFSD DRAWNOS
- INFORMATION REGARDING ANY EXISTING SERVICES AND/OR UTILITIES SHOWN ON THE APPROVED SET OF CONSTRUCTION DRAWNGS IS FURNISHED AS THE BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL INTERPRET IT THIS REFORMAND AS THEY SEE FIT WITH THE UNDERSTANDING THAT THE CONTRACTOR SHALL INTERPRET THIS REPORTAND AS THEY SEE FIT WITH THE UNDERSTANDING THAT THE CONTRACT AND HIS ADDITIONS.
- ALL DIMENSIONS SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO ANY CONSTRUCTION AND HE SHALL REPORT ANY DISCREPANCIES IMMEDIATELY TO THE BUSINESS
- RELOCATION OF EXISTING SERVICES AND/OR UTILITIES SHALL BE CONSTRUCTED AS SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE ENGINEER
- FOR ALL SEWERS AND WATERMAIN IN FILL SECTIONS, THE COMPACTION SHALL BE VERIFIED FROM TO LAYING OF PUPE.
- 7. NO SUBSTITUTIONS WILL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE COUNTY OF NORFOLK OR THE ENGINEER.
- A NO BLASTING WILL BE PERMITTED
- ALL EXCAVATIONS TO BE BACKFILLED WITH SELECT NATIVE MATERIAL, APPROVED BY THE ENGINEER, TO 95% S.P.D.
- THE DEVELOPER AND/OR CONTRACTOR IS RESPONSIBLE FOR INSTALLING AND MAINTAININ (UNTIL ROAD CONSTRUCTION IS FINISHED) SILT CONTROL DEVICES AS SHOWN ON THE DRAWINGS AND AS DIRECTED BY THE ENGINEER.
- ALL WORKS SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPLICABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLK DEVELOPMENT & ENCONCERNO STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS AND STANDARDS PRESCRIBED BY THE COUNTY.
- 13 ALL BOULEYARD AREAS TO BE RESTORED WITH #1 NURSERY SOD ON A MINIMUM IDOmm OF SELECT TOPSOIL.
- 14. ALL TERIORI BACKFILL UNDER EXISTING ROADWAYS SHALL BE COMPACTED IN MINIMUM 230mm UTTS TO BAS STANDARD PROCICER DIDISTY. A GEOTECHNICAL ENGNEER'S REPRESENTATIVE SHALL BE ON SITE DURING THE WORK TO VERIEY THE COMPACTION OF EACH LIFT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COSTS OF RE-TESTING.
- 15 AN ENGINEER IS REDUIRED TO BE ONSITE FOR INSPECTION OF ALL UNDERGROUND SERVICES.
- 16 DRIVEWAYS MUST HAVE MINIMUM I Om CLEARANCE FROM CATCHBASINS, VALVES, HYDRANTS STREETLICHT POLES, TRANSFORMERS, CANADA POST COMMUNITY MAILEOX LOCATIONS ETC.

500x1200 P/C CONC.— DITCH INLET STRUCTURE AS PER CIPSO 205.040 (TYPE B)

PROP. 375mme

PROP. 180mm (MA. ORIFICE PLATE (SEE DETAIL)

MTW (LEV.=220.95

768 W = 1465 L GRATE AS PER OPSD 403 010

INV =221.55

SWM POND OUTLET STRUCTURE DETAIL

TOP OF POND ELEV.=223.40

WATERMAINS:

- WATERMANS AND RELATED APPURTENANCES SHALL BE DESIGNED AND CONSTRUCTED SO AS TO COMPLY WITH APPUCABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF NORFOLD REVLOUPMENT MAD REMINIERMENT STANDARDES AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REQULATIONS, BEST PRACTICES AND STANDARDS PRESCRIBED BY THE COUNTY.
- WATERMAINS TO BE INSTALLED WITH A MINIMUM DEPTH OF COVER OF 1.70m BELOW FINISHED GRADE.
- WATERMAINS TO BE INSTALLED IN ACCORDANCE WITH OPSO BO2 010 TYPE 2 TRENCH BEDDING TO BE GRANULAR A UNLESS OTHERWISE NOTED.
- WATERMAINS TO BE PVC DR-18 IN ACCORDANCE WITH AWWA 0900 & CSA 8137.3. THE PIPE SHALL BE SHIPPED TO THE SITE WITH THE ENDS FACTORY CAPPED IF APPLICABLE.
- MAXIMUM ALLOWABLE DEFLECTION OF 1 DEGREE PER JOINT SHALL NOT BE EXCEEDED.
 EACH JOINT SHALL BE DEFLECTED AN EQUAL AMOUNT.

- UPON COMPLETION OF INSTALLATION, THE CONTRACTOR SHALL PERFORM A PRESSURE TES ON THE WATERWAINS AS PER 0955 70.07.24 AND COUNTY OF NORFOLK SPECIFICATIONS WATERWAIN IS TO BE ITESTED UNDER THE SUBHEMBORO OF THE OCHRETE PRIOR TO CONNECTION TO EXISTING WATERWAINS USING TEMPORATY CAPS OR PLUGS IF APPLICABLE

- NO WORK ON WATER SERVICES CAN TAKE PLACE WITHOUT SUPERVISION OF A LICENSED NORFOLK COUNTY WATER OPERATOR ON-SITE.

TOP OF HICKENBOTTO

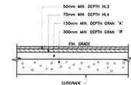
50mm# CLEAR STONE TO POND BOTTOM

8W.=221.55 ELEOW

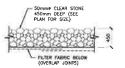
SANITARY & STORM SEWERS:

- SANITARY & STORM SEWERS & RELATED APPURITDANCES SHALL BE DESIGNED AND CONSTRUCTED ON A 5TO COMPTY WITH APPULGABLE LAW, TO BE CONSISTENT WITH THE COUNTY OF MORPHILD OPERCOPAGET AND ENGINEERING STANDARDS AND IN ACCORDANCE WITH CURRENT GUIDELINES, CODES, REGULATIONS, BEST PRACTICES AND STANDARDS PRESCRIBED BY THE COUNTY.
- COVER AND BEDDING MATERIAL FOR PVC PIPE AS PER OPSD 802.010 TYPE 2 TRENCH BEDDING SHALL BE GRANULAR 'A' MATERIAL UNLESS OTHERWISE INDICATED.
- 4. PVC PIFE WILL REQUIRE SPECIAL CONSTRUCTION PROCEDURES FOR LEAKAGE AND TESTING, THE DESIGNATION FOR
- 5. ALL SEWERS TO BE FLUSHED & VIDEOED PRIOR TO THE SUBMISSION OF THE FIRST INTERIM COMPLETION CERTIFICATE AND PRIOR TO THE FIRST COMPLETION CERTIFICATE

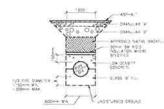
- B ALL SEWER PISTALLATIONS TO CONFORM WITH OPSO 802.031 TIPE 3 SOL
- 9 ALL MANHOLE FRAMES AND COVERS TO CONFORM WITH OPSD 401,010 TYPE 'A" CLOSED COVER.
- 10 MANHOLES SHALL BE SUPPLIED TO THE SITE PRE-BENCHED UNLESS OTHERWISE NOTED. 11. PRIVATE SANITARY & STORM DRAINS TO BE LOCATED AS PER THE TYPICAL LOT SERVICING DRAWING.
- 12 PRIVATE SANITARY DRAINS TO 150mm# PVC DR28 PIPE
- 13 A 38x89mm x 2.0m LONG MARKER IS TO BE PLACED FROM THE CAPPED LATERAL AND EXTEND 1,0m ABOVE GROUND AND PAINTED GREEN FOR SANITARY AND WHITE FOR STORM
- 14. BEDDING FOR PRIVATE SANTARY A STORM DRAWS AS PER OPED 1006.02 TYPE 2 TRENCH WITH GRANULAR 'A' BEDDING AND COVER MATERIAL.
- 15 MINIMUM FALL FOR PRIVATE SANITARY & STORM DRAINS TO BE 2.0%
- 16 ALL APPLICABLE PERMITS ARE TO BE APPLIED FOR PRIOR TO THE INSTALLATION OF ANY SERVICES
- 18. NO DEFLECTIONS OF SANTANT LATERALS ALLOHED FROM MAIN TO PROPERTY LINE. NEW CONNECTIONS MUST BE 3.00 FROM PROPERTY LINE. BAYS OR AS APPROVED OF THE MANAGER OF ENVIRONMENTAL SEMPOSES. AFTER THE PROPERTY LINE. ONLY 22.5 FITTINGS ARE ALLOWED OR 1.2m. 45° SWEEPS. CLEANOUTS ARE RECOURDED EVERY 30 5m. AND PAWALE MANAGES. DEVEY 91.3n.
- SPATIAL SEPARATION FROM WATER SERVICE CONNECTIONS OF NOT LESS THAN 2.44m MEASURED HORIZONTALLY FROM UNDISTURBED OR COMPACTED EARTH OR AS APPROVED BY THE MANAGER OF ENMRONMENTAL SERVICES



HEAVY DUTY ASPHALT DESIGN

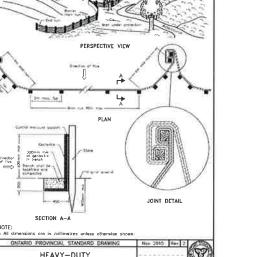


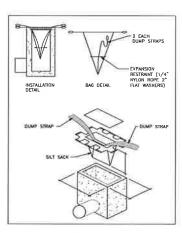
MUD MAT DETAIL



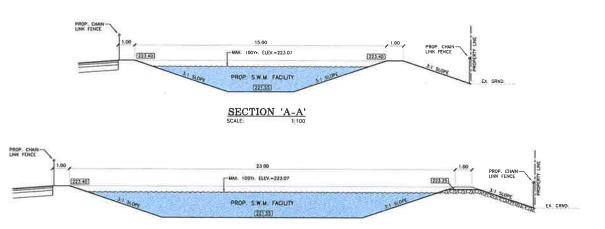
TYP. PIPE INSULATION DETAIL







SILT SACK DETAIL



SECTION 'B-B'



AS PER COUNTY COMMENTS



ENGINEERING LIMITED CONSULTING ENGINEERS

J.H. COHOON

440 HARDY ROAD . UNIT #1 . BRANTFORD — ONTARIO . N3T 5LB EL (519) 753-2656 FAX. (519) 753-4263 www.cuhooneng.co

PROPOSED MANUFACTURING BUILDING LUSCOMBE DRIVE, SIMCOE NORFOLK COUNTY

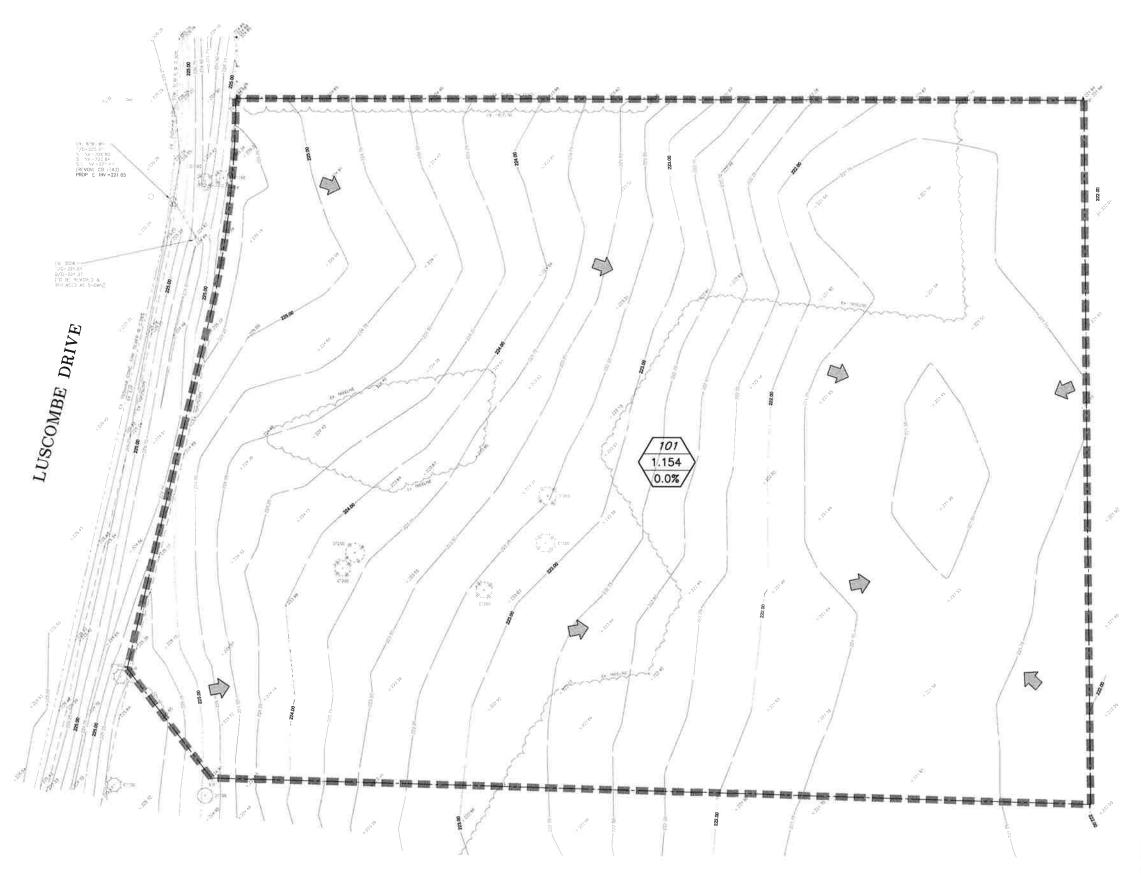
TONY KOUNTY

NOTES & DETAILS

DES/GN	R.W.P	AS SHOWN
HAN'S	SLM/KPB	o/ 60.
D-SCKED	R.W.P.	17233
=66](4 of 4	DRC No
M/V.	MAD 20/25	17233-4

September 2025

Appendix 'B' MIDUSS Computer Simulation Results Pre and Post Development Results



LEGEND

STORM DRAINAGE BOUNDARY



STORM DRAINAGE NUMBER
STORM AREA IN HECTARES

- % IMPERVIOUS



DRAINAGE ARROWS



PRE DEVELOPMENT STORM DRAINAGE AREAS

PROPOSED MANUFACTURING BUILDING LUSCOMBE DRIVE - NORFOLK COUNTY



J.H. COHOON ENGINEERING LIMITED CONSULTING ENGINEERS BRANTFORD

CLIENT: TONY KOUNTY SCALE: 1:500 JOB: 17233

```
77
                 MIDUSS Output ---->"
71
                 MIDUSS version
                                                        Version 2.25 rev. 473"
**
                MIDUSS created
                                                                February-07-10"
**
            10
                Units used:
                                                                     ie METRIC"
,,
                 Job folder:
                                                                  C:\swm\17233"
"
                 Output filename:
                                                                      pre5.out"
ŧţ
                 Licensee name:
                                                                           Bob"
**
                 Company
**
                 Date & Time last used:
                                                      12/07/2024 at 8:40:42 AM"
11
  31
              TIME PARAMETERS"
**
        10.000
                Time Step"
**
                Max. Storm length"
       180.000
**
      1500.000
                Max. Hydrograph"
**
  32
              STORM Chicago storm"
FF
             1
                Chicago storm"
.,
       583.017
                Coefficient A"
77
         3.007
                Constant B"
77
         0.703
               Exponent C"
**
         0.400
                Fraction R"
       180.000
                Duration"
11
         1.000
                Time step multiplier"
**
              Maximum intensity
                                          92.454
                                                    mm/hr"
11
              Total depth
                                          44.904
                                                    mm"
                         Hydrograph extension used in this file"
                005hyd
"
  33
              CATCHMENT 101"
**
                Rectangular"
**
             1
                Equal length"
             2
                Horton equation"
17
           101 No description"
**
         0.000 % Impervious"
**
         1.154 Total Area"
ŦŦ
       121.470 Flow length"
11
         3.000 Overland Slope"
FF
        1.154 Pervious Area"
**
       121.470 Pervious length"
77
         3.000 Pervious slope"
₹₹
         0.000 Impervious Area"
11
       121.470 Impervious length"
         3.000
                Impervious slope"
11
        0.250 Pervious Manning 'n'"
        25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
**
        0.500
                Pervious Lag constant (hours)"
,,
        7.500 Pervious Depression storage"
FF
        0.015
                Impervious Manning 'n'"
11
        0.000
                Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
                     0.074 0.000 0.000
                                                   0.000 c.m/sec"
             Catchment 101
                                               Impervious Total Area "
                                    Pervious
77
             Surface Area
                                   1.154
                                               0.000
                                                       1.154 hectare"
77
             Time of concentration 30.557
                                               4.691
                                                          30.557
                                                                   minutes"
             Time to Centroid 107.091
                                                          107.091 minutes"
                                               90.135
                                   44.904
518.19
             Rainfall depth
                                               44.904
                                                          44.904
                                                                   mm"
             Rainfall volume
                                               0.00
                                                          518.20
                                                                    c.m"
             Rainfall losses
                                   29.514
                                               2.000
                                                          29.514
                                                                     mm"
             Runoff depth
                                    15.391
                                               42.904
                                                          15.391
                                                                     mm"
             Runoff volume
11
                                   177.61
                                               0.00
                                                          177.61
                                                                     c.m"
11
             Runoff coefficient
                                  0.343
                                               0.000
                                                          0.343
```

"		Maximum flow	0.074	0.000	0.074	c.m/sec"
11	40	HYDROGRAPH Add Ru	unoff "			
**		4 Add Runoff"				
"		0.074	0.074 0.00	0.000) **	
***	38	START/RE-START TO	DTALS 101"			
7.7		3 Runoff Totals	on EXIT"			
**		Total Catchment a	area		1.154	hectare"
***		Total Impervious	area		0.000	hectare"
11		Total % imperviou	ıs		0.000"	
*1	19	EXIT"				

```
**
                 MIDUSS Output ---->"
ŦŦ
                 MIDUSS version
                                                       Version 2.25 rev. 473"
**
                MIDUSS created
                                                                February-07-10"
77
            10
                Units used:
                                                                     ie METRIC"
**
                 Job folder:
                                                                  C:\swm\17233"
,,
                 Output filename:
                                                                     pre10.out"
11
                 Licensee name:
                                                                           Bob"
**
                 Company
11
                 Date & Time last used:
                                                      12/07/2024 at 8:39:36 AM"
77
             TIME PARAMETERS"
**
       10.000 Time Step"
**
       180.000
                Max. Storm length"
"
      1500.000
                Max. Hydrograph"
**
  32
             STORM Chicago storm"
"
             1 Chicago storm"
77
       670.324
                Coefficient A"
11
         3.007
                Constant B"
ff
         0.698 Exponent C"
FF
         0.400 Fraction R"
77
       180.000
                Duration"
,,
         1.000
                Time step multiplier"
11
             Maximum intensity
                                         107.682
                                                   mm/hr"
**
             Total depth
                                         52.991
                                                   mm"
**
                005hyd Hydrograph extension used in this file"
             CATCHMENT 101"
77
             2
                Rectangular"
**
            1
                Equal length"
77
             2
                Horton equation"
          101 No description"
**
        0.000 % Impervious"
11
        1.154 Total Area"
**
       121.470 Flow length"
77
        3.000 Overland Slope"
77
        1.154 Pervious Area"
77
       121.470 Pervious length"
**
        3.000 Pervious slope"
        0.000 Impervious Area"
"
       121.470 Impervious length"
77
         3.000 Impervious slope"
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
        7.500 Pervious Depression storage"
11
        0.015 Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
        0.500 Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
                     0.117 0.000 0.000
                                                   0.000 c.m/sec"
             Catchment 101
                                   Pervious Impervious Total Area "
             Surface Area
                                   1.154
                                               0.000 1.154 hectare"
             Time of concentration 26.359
                                               4.413
                                                         26.358
                                                                   minutes"
             Time to Centroid
                                   108.198
                                               89.647
                                                         108.198 minutes"
                                  52.991 52.991
611.52 0.00
30.008 2.000
             Rainfall depth
                                               52.991
                                                         52.991
                                                                   mm"
             Rainfall volume
                                                         611.52
                                                                    c.m"
             Rainfall losses
                                               2.000
                                                         30.008
                                                                   mm"
77
             Runoff depth
                                   22.983
                                               50.991
                                                         22.983
                                                                     mm"
             Runoff volume 265.23
Runoff coefficient 0.434
                                             0.00
                                                         265.23
                                                                    c.m"
**
                                               0.000
                                                         0.434
```

" 40	Maximum flow HYDROGRAPH Add Runof	0.117 f "	0.000	0.117	c.m/sec"
77	4 Add Runoff "				
TT	0.117 0.	117 0.000	0.000"		
" 38	START/RE-START TOTAL	S 101"			
**	3 Runoff Totals on	EXIT"			
11	Total Catchment area		1	.154	hectare"
rr .	Total Impervious are	a	0	.000	hectare"
11	Total % impervious		0	.000"	
" 19	EXIT"				

```
11
                 MIDUSS Output ----->"
77
                                                        Version 2.25 rev. 473"
                 MIDUSS version
ŦŦ
                 MIDUSS created
                                                                 February-07-10"
11
            10 Units used:
                                                                     ie METRIC"
77
                 Job folder:
                                                                   C:\swm\17233"
**
                 Output filename:
                                                                      pre25.out"
                 Licensee name:
                                                                            Bob"
77
                 Company
ff
                 Date & Time last used:
                                                      12/07/2024 at 8:38:36 AM"
**
  31
              TIME PARAMETERS"
**
        10.000 Time Step"
77
       180.000
               Max. Storm length"
ŦŦ
      1500.000 Max. Hydrograph"
11
  32
              STORM Chicago storm"
11
             1 Chicago storm"
77
       721.533
               Coefficient A"
**
         2.253 Constant B"
77
         0.679 Exponent C"
77
               Fraction R"
         0.400
**
       180.000
               Duration"
**
         1.000 Time step multiplier"
77
              Maximum intensity
                                                  mm/hr"
                                          127.011
**
              Total depth
                                          63.151
                                                   mm"
11
                 005hyd
                        Hydrograph extension used in this file"
11
  33
              CATCHMENT 101"
             2
                 Rectangular"
.
             1
                 Equal length"
11
             2 Horton equation"
77
           101
                 No description"
         0.000 % Impervious"
ŧŧ
         1.154 Total Area"
11
       121.470 Flow length"
11
         3.000 Overland Slope"
11
         1.154 Pervious Area"
77
       121.470 Pervious length"
**
         3.000 Pervious slope"
..
         0.000 Impervious Area"
11
       121.470 Impervious length"
**
         3.000 Impervious slope"
71
        0.250 Pervious Manning 'n'"
        25.000
               Pervious Max.infiltration"
**
         5.000 Pervious Min.infiltration"
        0.500 Pervious Lag constant (hours)"
* *
        7.500 Pervious Depression storage"
        0.015
                Impervious Manning 'n'"
11
        0.000
                Impervious Max.infiltration"
         0.000
                Impervious Min.infiltration"
         0.500
                 Impervious Lag constant (hours)"
         2.000
                 Impervious Depression storage"
                     0.180 0.000 0.000
                                                    0.000 c.m/sec"
             Catchment 101
                                    Pervious
                                                Impervious Total Area "
              Surface Area
                                     1.154
                                                0.000
                                                          1.154
                                                                     hectare"
              Time of concentration 23.173
                                               4.131
                                                           23.173
                                                                     minutes"
11
             Time to Centroid 108.543
                                                89.310
                                                          108.543
                                                                     minutes"
                                   63.151 63.151
728.76 0.00
30.582 2.000
32.569 61.151
375.84 0.00
11
             Rainfall depth
                                                          63.151
                                                                     mm"
             Rainfall volume
                                                          728.76
                                                                      c.m"
             Rainfall losses
                                                          30.582
                                                                     mm"
             Runoff depth
                                                          32.569
                                                                     mm"
**
             Runoff volume
                                                          375.84
                                                                      c.m"
             Runoff coefficient 0.516
                                                0.000
                                                          0.516
```

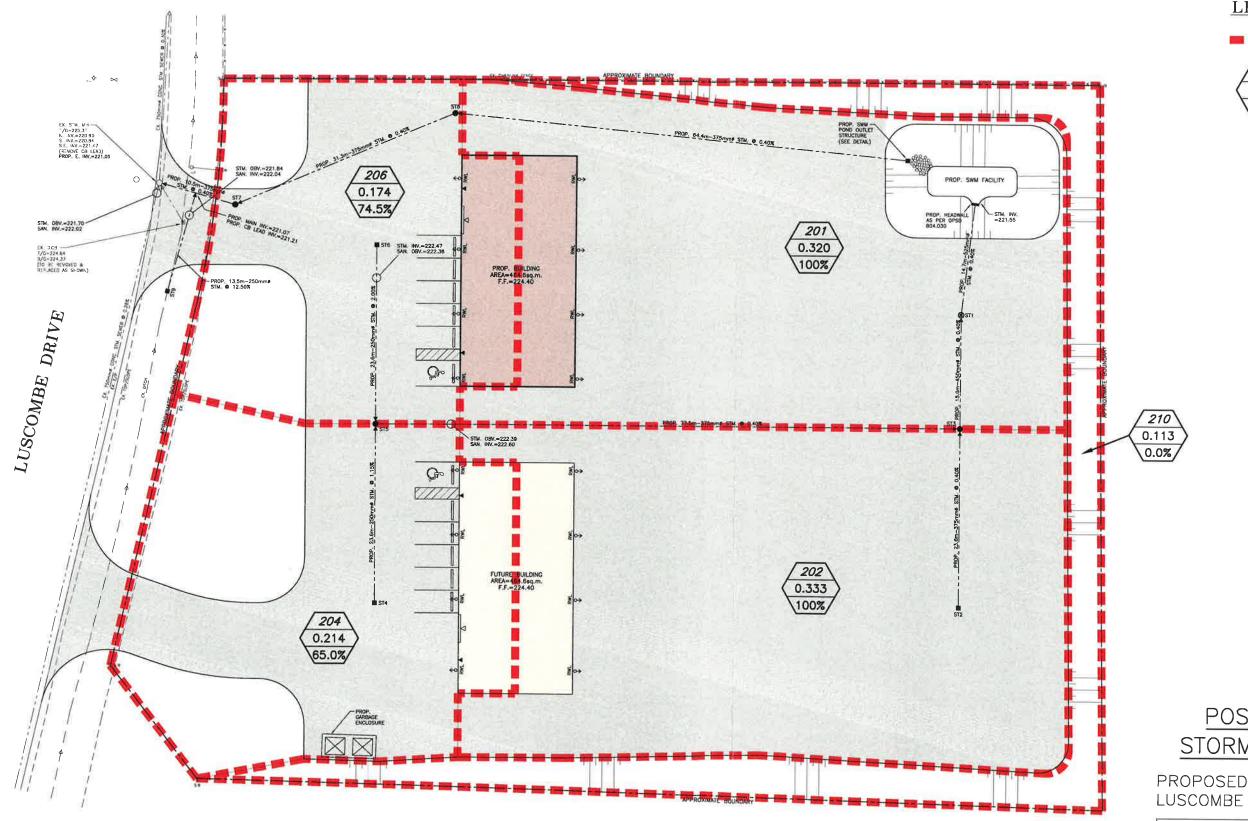
11		Maximum flow	0.1	80	0.000	0.180	c.m/sec"
**	40	HYDROGRAPH Add	Runoff "				
**		4 Add Runoff "					
**		0.180	0.180	0.000	0.000	τı	
**	38	START/RE-START	TOTALS 101"				
**		3 Runoff Total:	s on EXIT"				
7.7		Total Catchment	area			1.154	hectare"
\mathbf{H}		Total Impervious	s area			0.000	hectare"
11		Total % impervi	ous			0.000"	
r F	19	EXIT"					

```
11
                MIDUSS Output ---->"
7 7
                                                       Version 2.25 rev. 473"
                MIDUSS version
**
                                                               February-07-10"
                MIDUSS created
FF
           10
                Units used:
                                                                   ie METRIC"
11
                Job folder:
                                                                 C:\swm\17233"
ff
                Output filename:
                                                                    pre50.out"
**
                Licensee name:
                                                                          Bob"
* *
                Company
**
                Date & Time last used:
                                                    12/07/2024 at 8:37:38 AM"
ŦŦ
             TIME PARAMETERS"
11
       10.000 Time Step"
11
      180.000
              Max. Storm length"
**
     1500.000 Max. Hydrograph"
  32
             STORM Chicago storm"
77
            1 Chicago storm"
**
      766.038 Coefficient A"
**
        1.898 Constant B"
**
        0.668 Exponent C"
..
        0.400 Fraction R"
**
      180.000 Duration"
FF
        1.000
                Time step multiplier"
             Maximum intensity
                                                 mm/hr"
                                        141.545
**
             Total depth
                                        71.090
                                                  mm"
"
                005hyd Hydrograph extension used in this file"
11
  33
             CATCHMENT 101"
*1
            2
                Rectangular"
77
                Equal length"
            1
**
            2 Horton equation"
**
          101 No description"
**
        0.000 % Impervious"
"
        1.154 Total Area"
**
      121.470 Flow length"
77
        3.000 Overland Slope"
11
        1.154 Pervious Area"
* *
      121.470 Pervious length"
**
        3.000 Pervious slope"
        0.000 Impervious Area"
**
      121.470 Impervious length"
        3.000 Impervious slope"
**
        0.250 Pervious Manning 'n'"
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
"
        0.500 Pervious Lag constant (hours)"
11
        7.500 Pervious Depression storage"
        0.015 Impervious Manning 'n'"
* *
        0.000 Impervious Max.infiltration"
11
        0.000 Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
11
                    0.224 0.000 0.000
                                                  0.000 c.m/sec"
             Catchment 101
                                   Pervious
                                              Impervious Total Area "
             Surface Area
                                   1.154
                                              0.000
                                                         1.154
                                                                  hectare"
             Time of concentration 21.905
                                              3.956
                                                         21.905
                                                                  minutes"
             Time to Centroid 108.648
                                              89.121
                                                         108.648
                                                                 minutes"
**
             Rainfall depth
                                   71.090
                                              71.090
                                                         71.090
                                                                  mm"
"
             Rainfall volume
                                  820.37
                                              0.00
                                                         820.37
                                                                   C.m"
             Rainfall losses
                                  30.909
                                              2.000
                                                         30.909
                                                                   mm"
             Runoff depth
                                  40.180
                                              69.090
                                                        40.180
                                                                   mm"
77
             Runoff volume
                                  463.68
                                              0.00
                                                         463.68
                                                                    c.m"
             Runoff coefficient 0.565
                                              0.000
                                                         0.565
```

17		Maximum flow	0.224	0.000	0.224	c.m/sec"
**	40	HYDROGRAPH Add Ru	unoff "			
**		4 Add Runoff "				
**		0.224	0.224 0.000	0.000"		
**	38	START/RE-START TO	TALS 101"			
**		3 Runoff Totals	on EXIT"			
**		Total Catchment a	area	1	.154	hectare"
**		Total Impervious	area	0	.000	hectare"
**		Total % imperviou	ıs	0	.000"	
17	19	EXIT"				

```
* *
                MIDUSS Output ----->"
77
                MIDUSS version
                                                       Version 2.25 rev. 473"
77
                MIDUSS created
                                                                February-07-10"
**
           10
                Units used:
                                                                     ie METRIC"
"
                Job folder:
                                                                  C:\swm\17233"
77
                Output filename:
                                                                    pre100.out"
ŦŦ
                Licensee name:
                                                                           Bob"
**
                Company
**
                Date & Time last used:
                                                      12/07/2024 at 8:35:29 AM"
**
             TIME PARAMETERS"
  31
**
       10.000
                Time Step"
**
                Max. Storm length"
      180.000
11
      1500.000
                Max. Hydrograph"
77
  32
             STORM Chicago storm"
77
            1
                Chicago storm"
Ŧ F
      801.041
                Coefficient A"
**
        1.501 Constant B"
**
        0.657 Exponent C"
77
        0.400
                Fraction R"
77
      180.000
                Duration"
**
        1.000
                Time step multiplier"
             Maximum intensity
                                         155.782
                                                    mm/hr"
77
             Total depth
                                         78.830
                                                    mm"
                005hyd Hydrograph extension used in this file"
"
  33
             CATCHMENT 101"
**
            2
                Rectangular"
71
            1
                Equal length"
77
            2
                Horton equation"
77
          101 No description"
**
        0.000 % Impervious"
**
        1.154 Total Area"
**
      121.470 Flow length"
**
        3.000 Overland Slope"
**
        1.154 Pervious Area"
      121.470 Pervious length"
**
        3.000 Pervious slope"
11
        0.000 Impervious Area"
11
      121.470 Impervious length"
        3.000 Impervious slope"
77
        0.250 Pervious Manning 'n'"
       25.000 Pervious Max.infiltration"
11
        5.000 Pervious Min.infiltration"
        0.500
                Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
ŦŦ
        0.015
                Impervious Manning 'n'"
**
        0.000
                Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
                           0.000
                     0.267
                                      0.000
                                                   0.000 c.m/sec"
11
             Catchment 101
                                    Pervious Impervious Total Area "
             Surface Area
                                    1.154
                                               0.000
                                                       1.154 hectare"
77
             Time of concentration 21.014
                                               3.807
                                                          21.014
                                                                    minutes"
                                                          108.220
             Time to Centroid
                                    108.220
                                               0.000
                                                                     minutes"
             Rainfall depth
                                   78.830
909.70
                                               78.830
                                                          78.830
                                                                   mm"
             Rainfall volume
                                               0.00
                                                          909.70
                                                                     c.m"
             Rainfall losses
                                               78.830
                                   31.239
                                                          31.239
                                                                     mm"
             Runoff depth
                                   47.591
                                               0.000
                                                          47.591
                                                                     mm"
             Runoff volume
**
                                  549.20
                                               0.00
                                                          549.20
                                                                     c.m"
             Runoff coefficient
                                  0.604
                                               0.000
                                                          0.604
```

11				0.000	0.267	c.m/sec"
**	40	HYDROGRAPH Add Runoff	11			
**		4 Add Runoff "				
7.7		0.267 0.26	7 0.000	0.000	11	
11	38	START/RE-START TOTALS	101"			
***		3 Runoff Totals on EX	IT"			
**		Total Catchment area			1.154	hectare"
7.7		Total Impervious area			0.000	hectare"
**		Total % impervious			0.000"	
"	19	EXIT"				



LEGEND

STORM DRAINAGE BOUNDARY



STORM DRAINAGE NUMBER
STORM AREA IN HECTARES

→ % IMPERVIOUS



POST DEVELOPMENT STORM DRAINAGE AREAS

PROPOSED MANUFACTURING BUILDING LUSCOMBE DRIVE - NORFOLK COUNTY



J.H. COHOON ENGINEERING LIMITED CONSULTING ENGINEERS BRANTFORD

CLIENT: TONY KOUNTY SCALE: 1:500

JOB: 17233

Orifice Plate Calculations Proposed Industrial Development Luscombe Drive, Simcoe

Job: 17233

"March 2025

	C 0,63	0.63 0.63			O	0.63	0.63			U	0.63 0.63 0.63
		.84 .72	0.0241 sq.m. 0.4932 cu.m.			00	84 72	0.0241 sq.m 0.4932 cu.m.			00 12 67
	=(2gh)**0.5 0.0000	5.6072	0.02		=(2gh)**0.5	0.0000	5.3384 5.6072	0.02		=(2gh)**0.5	0.0000 6.7212 6.9367
	2gh	28.49805 31.44105			2gh	0	28.49805 31.44105			2gh	0 45.17505 48.11805
	(m)	1,4525 1.6025	Area of Orifice Structure Storage		ا (E	0	1.4525 1.6025	Area of Orifice Structure Storage		ح (ۋ	2.3025 2.4525
	Discharge (cms) 0.0000	0.0809 0.0850	221.61 175 mm 221.70		Discharge (cms)	0.0000	0.0809 0.0850	221.81 175 mm 221.90		Discharge (cms)	0.0000 0.0333 0.0343
		10.32 41.15				0.00	9.88 39.50				0.00 10.55 42.19
	Storage (cu.m.) 0.49	10.81 41.64			Storage (cu.m.)	0.49	10.37 39.99			Storage	2.49 13.04 44.68
	Depth (m)	0.15 0.30	Orifice Plate meter ifice Plate		Depth (m)	0.00	0.15	Orifice Plate meter ifice Plate		Depth (m)	0.00 0.15 0.30
Pond Area ST1	Stage (m) 223.00	223.15 223.30	Invert of Outlet/Orifice Plate Orifice Plate Diameter Centreline of Orifice Plate	Pond Area ST2	Stage (m)	223.00	223.15 223.30	Invert of Outlet/Orifice Plate Orifice Plate Diameter Centreline of Orifice Plate	Pond Area ST4	Stage	223.85 224.00 224.15

0.0079 sq.m. 2.491 cu.m.		=(2gh)**0.5 C 0.0000 0,63 5.2550 0.63 5.5279 0.63	0.0057 sq.m. 2.491 cu.m.	=(2gh)**0.5 C #NUM! 0.63 3.9120 0.63 5.3887 0.63 0.0201 sq.m
		2gh 0 27.61515 30.55815		2gh -0.3924 15.3036 29.0376
Area of Orifice Structure Storage		h (m) 0 1.41 1.56	Area of Orifice Structure Storage	h (m) -0.02 0.78 1.48 Area of Orifice Structure Storage
222.35 100 mm 222.40		Discharge (cms) 0.0000 0.0188	222.55 85 mm 222.59	Discharge (cms) 0.0010 0.0496 0.0683 221.54 160 mm 221.62
		0.00 11.25 45.00		0.00 99.14 262.66
		Storage (cu.m.) 0.49 11.74 45.49		Storage (cu.m.) 0.00 99.14 262.66
VOrifice Plate iameter orifice Plate	Dept	Depth (m) 0.00 0.15	/Orifice Plate iameter vrifice Plate	Depth (m) 0.00 0.80 1.50 1.50 ameter rrifice Plate
Invert of Outlet/Orifice Plate Orifice Plate Diameter Centreline of Orifice Plate		Stage (m) 223.85 224.00 224.15	Invert of Outlet/Orifice Plate Orifice Plate Diameter Centreline of Orifice Plate	SWM Pond Stage Depth (m) (m) 221.60 0.00 222.40 0.80 223.10 1.50 Invert of Outlet/Orifice Plate Orifice Plate Diameter Centreline of Orifice Plate

```
11
                MIDUSS Output ----->"
11
                MIDUSS version
                                                       Version 2.25 rev. 473"
                MIDUSS created
                                                               February-07-10"
            10
                Units used:
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                Job folder:
                                                C:\swm\17233\Update 2025-3-20"
                Output filename:
                                                                    pst54.out"
                Licensee name:
                                                                          Bob"
*
                Company
T.
                Date & Time last used:
                                                    25/07/2024 at 11:42:26 AM"
#
             TIME PARAMETERS"
        10.000
               Time Step"
"
      180.000
              Max. Storm length"
      1500.000 Max. Hydrograph"
             STORM Chicago storm"
  32
**
            1 Chicago storm"
11
       583.017 Coefficient A"
"
         3.007 Constant B"
         0.703 Exponent C"
              Fraction R"
        0.400
**
       180.000
                Duration"
**
         1.000
                Time step multiplier"
**
             Maximum intensity
                                          92.454
                                                   mm/hr"
**
             Total depth
                                                  mm"
                                         44.904
**
                005hyd Hydrograph extension used in this file"
11
  33
             CATCHMENT 201"
            2
                Rectangular"
"
            1
                Equal length"
11
            2 Horton equation"
11
          201
              No description"
      100.000 % Impervious"
"
        0.320 Total Area"
**
       80.000 Flow length"
11
        1.200 Overland Slope"
        0.000 Pervious Area"
**
       80.000 Pervious length"
11
        1.200 Pervious slope"
**
        0.320 Impervious Area"
tr
       80.000 Impervious length"
**
        1.200
                Impervious slope"
"
        0.250
                Pervious Manning 'n'"
**
       25.000
                Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
"
        0.500 Pervious Lag constant (hours)"
11
        7.500 Pervious Depression storage"
"
                Impervious Manning 'n'"
        0.015
**
        0.000
                Impervious Max.infiltration"
**
        0.000
                Impervious Min.infiltration"
**
        0.500
                Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
**
                     0.082 0.000 0.000
                                                  0.000 c.m/sec"
"
             Catchment 201
                                   Pervious
                                              Impervious Total Area "
**
             Surface Area
                                   0.000
                                              0.320
                                                      0.320
                                                                   hectare"
             Time of concentration 31.308
                                              4.806
                                                         4.806
                                                                   minutes"
11
             Time to Centroid 107.746
                                              90.205
                                                        90.205
11
             Rainfall depth
                                  44.904
                                              44.904
                                                        44.904
                                                                   mm"
11
             Rainfall volume
                                  0.00
                                              143.69
                                                         143.69
                                                                    c.m"
                                  29.514 2.000
15 391 42.904
"
             Rainfall losses
                                                        2.000
                                                                   mm"
**
             Runoff depth
                                                        42.904
"
             Runoff volume
                                  0.00
                                             137.29
                                                        137.29
                                                                    c.m"
             Runoff coefficient 0.000
**
                                              0.955
                                                        0.955
```

```
11
                                     0.000
              Maximum flow
                                               0.082
                                                         0.082
                                                                    c.m/sec"
**
  40
              HYDROGRAPH Add Runoff "
"
             4 Add Runoff "
ŦŦ
                      0.082
                               0.082
                                        0.000
                                                   0.000"
"
              PIPE DESIGN"
"
               Current peak flow
         0.082
                                     c.m/sec"
77
                Manning 'n'"
         0.013
**
         1.000
               Diameter
                           metre"
77
         1.000
                Gradient
77
              Depth of flow
                                            0.127
                                                    metre"
,,
              Velocity
                                            1.421
                                                    m/sec"
**
              Pipe capacity
                                            2.398
                                                    c.m/sec"
"
              Critical depth
                                            0.158
                                                    metre"
  53
              ROUTE Zero Route"
* *
                 Zero Route Reach length ( metre)"
,,
                      0.082 0.082 0.000 c.m/sec"
11
              HYDROGRAPH Combine 201"
  40
11
               Combine "
**
           201
                 Node #"
**
11
              Maximum flow
                                            0.082
                                                    c.m/sec"
              Hydrograph volume
                                          137.294
                                                    c.m"
**
                      0.082 0.082
                                          0.082
                                                   0.082"
  40
             HYDROGRAPH Start - New Tributary"
77
             2 Start - New Tributary"
**
                                                  0.082"
                      0.082
                              0.000
                                         0.082
77
  33
              CATCHMENT 202"
"
             2
                Rectangular"
77
             1
                Equal length"
             2
                Horton equation"
11
           202 No description"
n
       100.000 % Impervious"
17
        0.333 Total Area"
**
        83.250 Flow length"
,,
        1.200 Overland Slope"
11
        0.000 Pervious Area"
11
        83.250 Pervious length"
11
        1.200 Pervious slope"
11
        0.333 Impervious Area"
**
        83.250 Impervious length"
        1.200 Impervious slope"
"
        0.250 Pervious Manning 'n'"
        25.000 Pervious Max.infiltration"
77
        5.000 Pervious Min.infiltration"
        0.500
                Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
**
        0.015
                Impervious Manning 'n'"
..
        0.000
                Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
11
        0.500
                Impervious Lag constant (hours)"
77
        2.000
                Impervious Depression storage"
**
                      0.086
                            0.000
                                         0.082
                                                   0.082 c.m/sec"
             Catchment 202
                                    Pervious Impervious Total Area "
**
             Surface Area
                                    0.000
                                               0.333
                                                          0.333 hectare"
FF
             Time of concentration 32.065
                                               4.922
                                                          4.922
                                                                     minutes"
**
             Time to Centroid 108.382
                                               90.284
                                                          90.284
                                                                   minutes"
**
             Rainfall depth
                                    44.904
                                               44.904
                                                          44.904
                                                                     mm"
**
             Rainfall volume
                                               149.53
                                                          149.53
                                   0.00
                                                                     c.m"
**
                                   29.514
             Rainfall losses
                                               2.000
                                                          2.000
                                                                     mm"
11
             Runoff depth
                                    15.391
                                               42.904
                                                          42.904
                                                                     mm"
```

```
**
              Runoff volume
                                     0.00
                                                142.87
                                                                      \texttt{C.m}^{\prime\prime}
                                                          142.87
                                   0.000
              Runoff coefficient
                                              0.955
                                                           0.955
ŦŦ
                                     0.000
             Maximum flow
                                                0.086
                                                           0.086
                                                                    c.m/sec"
  40
             HYDROGRAPH Add Runoff "
77
             4 Add Runoff "
**
                     0.086
                            0.086
                                        0.082
                                                0.082"
             PIPE DESIGN"
  51
**
         0.086 Current peak flow c.m/sec"
11
         0.013 Manning 'n'"
         1.000 Diameter metre"
**
         1.000 Gradient %"
**
              Depth of flow
                                           0.129
                                                    metre"
"
                                           1.438
             Velocity
                                                    m/sec"
                                           2.398 c.m/sec"
              Pipe capacity
**
             Critical depth
                                           0.161
                                                     metre"
  53
             ROUTE Zero Route"
**
          0.00 Zero Route Reach length ( metre)"
ff
                 0.086 0.086 0.086 0.082 c.m/sec"
**
  40
             HYDROGRAPH Combine 202"
**
             6 Combine "
FF
                Node #"
          202
**
**
             Maximum flow
                                            0.086
                                                  c.m/sec"
"
             Hydrograph volume
                                                    c.m"
                                          142.871
77
                     0.086 0.086
                                        0.086
                                                    0.086"
**
             HYDROGRAPH Start - New Tributary"
* *
             2 Start - New Tributary"
77
                     0.086
                              0.000 0.086 0.086"
  33
             CATCHMENT 206"
**
             2 Rectangular"
.,
            1 Equal length"
"
            2 Horton equation"
77
          206 No description"
**
       74.500 % Impervious"
**
        0.174 Total Area"
* *
       19.121 Flow length"
1.200 Overland Slope"
"
        0.044 Pervious Area"
77
       19.121 Pervious length"
        1.200 Pervious slope"
0.130 Impervious Area"
ŦŦ
**
**
       19.121 Impervious length"
11
        1.200 Impervious slope"
**
        0.250 Pervious Manning 'n'"
       25.000 Pervious Max.infiltration"
**
**
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
"
        7.500 Pervious Depression storage"
77
        0.015 Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
**
        0.000 Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
"
        2.000
                Impervious Depression storage"
,,
                     0.039
                             0.000 0.086
                                                  0.086 \text{ c.m/sec''}
                                    Pervious Impervious Total Area "
FF
             Catchment 206
                                    0.044
             Surface Area
                                               0.130 0.174 hectare"
             Time of concentration 13.265
Time to Centroid 94.292
                                                         3.264 minutes"
90.001 minutes"
77
                                               2.036
                                               89.474
**
* *
             Rainfall depth 44.904
Rainfall volume 19.92
                                                                    mm"
                                              44.904
                                                          44.904
                                               58.21
                                                           78.13
                                                                     c.m"
```

```
**
             Rainfall losses
                                   29.514
                                             2.000
                                                        9.016
                                                                  mm"
                                 15.391
6.83
0.343
             Runoff depth
Runoff volume
                                             42.904
                                                        35.888
                                                                  mm"
11
                                             55.62
                                                        62.45
                                                                  c.m"
77
             Runoff coefficient
                                             0.955
                                                        0.799
**
             Maximum flow
                                   0.005
                                             0.033
                                                        0.039
                                                                  c.m/sec"
             HYDROGRAPH Add Runoff "
  40
11
            4 Add Runoff "
**
                    0.039
                            0.039
                                       0.086
                                                0.086"
**
  51
             PIPE DESIGN"
        0.039 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
**
        1.000 Diameter metre"
,,
        1.000 Gradient %"
             Depth of flow
                                         0.088
                                                  metre"
77
             Velocity
                                         1.133 m/sec"
**
             Pipe capacity
                                         2.398
                                                  c.m/sec"
**
             Critical depth
                                         0.108
                                                  metre"
**
  53
             ROUTE Zero Route"
11
                Zero Route Reach length ( metre)"
         0.00
77
                  0.039 0.039 0.086 c.m/sec"
             HYDROGRAPH Combine 206"
  40
11
            6
                Combine "
"
                Node #"
          206
**
FF
             Maximum flow
                                                c.m/sec"
                                         0.039
71
             Hydrograph volume
                                        62.446 c.m"
17
                    0.039 0.039
                                     0.039
                                                 0.039"
             HYDROGRAPH Start - New Tributary"
**
            2 Start - New Tributary"
**
                              0.000
                    0.039
                                      0.039
                                                0.039"
 33
             CATCHMENT 204"
11
            2 Rectangular"
            1 Equal length"
77
            2 Horton equation"
          204 No description"
..
       65.000 % Impervious"
Ħ
       0.214 Total Area"
**
       24.318 Flow length"
**
        1.200 Overland Slope"
**
        0.075 Pervious Area"
Ħ
       24.318 Pervious length"
FF
       1.200 Pervious slope"
11
       0.139 Impervious Area"
       24.318 Impervious length"
**
       1.200 Impervious slope"
**
        0.250 Pervious Manning 'n'"
11
       25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
"
        0.500 Pervious Lag constant (hours)"
        7.500 Pervious Depression storage"
**
        0.015 Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
,,
        0.000
               Impervious Min.infiltration"
        0.500
               Impervious Lag constant (hours)"
Ħ
        2.000
               Impervious Depression storage"
                    0.044 0.000 0.039 0.039 c.m/sec"
11
             Catchment 204
                                 Pervious Impervious Total Area "
             Surface Area
                                             0.139 0.214 hectare"
                                  0.075
**
             Time of concentration 15.324
                                             2.352
                                                       4.452
                                                                 minutes"
             Time to Centroid 95.896
                                             89.474
                                                       90.513
                                                                minutes"
```

```
**
              Rainfall depth
                                      44.904
                                                 44.904
                                                             44.904
                                                                         mm"
              Rainfall volume 33.63 62.46 96.10
Rainfall losses 29.514 2.000 11.630
Runoff depth 15.391 42.904 33.274
Runoff volume 11.53 59.68 71.21
Runoff coefficient 0.343 0.955 0.741
Maximum flow 0.008 0.036 0.044
                                                                         c.m"
                                                                        mm"
                                                                        c.m"
                                                                       11
**
              Maximum flow
                                                                       c.m/sec"
  40
             HYDROGRAPH Add Runoff "
             4 Add Runoff "
                  0.044 0.044 0.039 0.039"
**
  51
            PIPE DESIGN"
         0.044 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
         1.000 Diameter metre"
**
         1.000 Gradient %"
rı,
             Depth of flow
                                            0.094 metre"
H
                                            1.175 m/sec"
2.398 c.m/sec"
              Velocity
              Pipe capacity
**
              Critical depth
                                            0.114 metre"
**
              ROUTE Zero Route"
  53
**
          HYDROGRAPH Combine 204"
  40
**
             6 Combine "
Ħ.
           204 Node #"
m
n
              Maximum flow
                                            0.044
                                                     c.m/sec"
              Hydrograph volume
Ħ
                                           71.207
                                                      c.m"
                      0.044 0.044 0.044
                                                      0.044"
  40
             HYDROGRAPH Start - New Tributary"
11
             2 Start - New Tributary"
11
                     0.044
                                 0.000
                                         0.044 0.044"
  33
              CATCHMENT 210"
11
             2 Rectangular"
11
             1 Equal length"
**
             2 Horton equation"
         210 No description"
0.000 % Impervious"
"
"
        0.113 Total Area"
**
        12.151 Flow length"
1.200 Overland Slope"
11
        0.113 Pervious Area"
"
        12.151 Pervious length"
        1.200 Pervious slope"
0.000 Impervious Area"
11
11
**
        12.151 Impervious length"
11
        1.200 Impervious slope"
"
         0.250 Pervious Manning 'n'"
"
        25.000 Pervious Max.infiltration"
11
         5.000 Pervious Min.infiltration"
**
         0.500 Pervious Lag constant (hours)"
         7.500 Pervious Depression storage"
Ħ
         0.015 Impervious Manning 'n'"
**
         0.000 Impervious Max.infiltration"
11
         0.000
                 Impervious Min.infiltration"
         0.500
                 Impervious Lag constant (hours)"
"
         2.000
                 Impervious Depression storage"
"
                      0.018 0.000 0.044 0.044 c.m/sec"
11
              Catchment 210 Pervious Impervious Total Area "Surface Area 0.113 0.000 0.113 h
                                                  0.000 0.113 hectare"
```

```
Time of concentration 10.106 1.551 10.106 minutes" Time to Centroid 91.432 89.474 91.432 minutes" Rainfall depth 44.904 44.904 44.904 mm" Rainfall volume 50.74 0.00 50.74 c.m" Rainfall losses 29.514 2.000 29.514 mm" Runoff depth 15.391 42.904 15.391 mm" Runoff volume 17.39 0.00 17.39 c.m" Runoff coefficient 0.343 0.000 0.343 " Maximum flow 0.018 0.000 0.018 c.m/sec"
11
\mathbf{n}
**
                   HYDROGRAPH Add Runoff "
                 4 Add Runoff "
"
                        0.018 0.018 0.044 0.044"
             PIPE DESIGN"
   51
             0.018 Current peak flow c.m/sec"
"
             0.013 Manning 'n'"
             1.000 Diameter metre"
11
             1.000 Gradient %"
                  Depth of flow
Velocity
Pipe capacity
Critical depth
                                                                 0.062 metre"
**
                                                                0.899 m/sec"
11
                                                            2.398 c.m/sec"
0.073 metre"
**
                  ROUTE Zero Route"
11
              0.00 Zero Route Reach length ( metre)"
"
                         11
   40
                   HYDROGRAPH Combine 9999"
                6 Combine "
**
              9999 Node #"
11

      Maximum flow
      0.018
      c.m/se

      Hydrograph volume
      17.391
      c.m"

      0.018
      0.018
      0.018
      0.018"

      HYDROGRAPH
      Confluence
      204"

11
                                                                0.018 c.m/sec"
**
**
**
                7 Confluence "
11
11
              204 Node #"
11
11

      Maximum flow
      0.044
      c.m/sec"

      Hydrograph volume
      71.207
      c.m"

      0.018
      0.044
      0.018
      0.000"

                  Maximum flow
11
11
             POND DESIGN"
"
   54
**
             0.044 Current peak flow c.m/sec"
**
             0.080 Target outflow c.m/sec"
11
              71.2 Hydrograph volume c.m"
          3. Number of stages"
223.850 Minimum water level metre"
224.150 Maximum water level metre"
11
11
m
          223.850 Starting water level metre"
**
                   0 Keep Design Data: 1 = True; 0 = False"
                        Level Discharge Volume"

223.850 1.00E-07 2.490"

224.000 0.03300 13.040"

224.150 0.03430 44.580"
**
"
"
                 11
11
11
**
**
11
                 5 Next link "
**
                         0.018 0.027 0.027 0.000"
" 51
                  PIPE DESIGN"
```

```
0.027 Current peak flow c.m/sec"
         0.013 Manning 'n'"
**
         0.250 Diameter metre"
         1.150 Gradient %"
11
                                            0.112 metre"
1.240 m/sec"
0.064 c.m/sec"
             Depth of flow
11
              Velocity
**
              Pipe capacity
"
              Critical depth
                                            0.132 metre"
             ROUTE Pipe Route 24"
* *
         23.60 Pipe Route 24 Reach length ( metre)"
77
         0.382 X-factor <= 0.5"
**
        14.277 K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
11
        0.500 X-factor <= 0.5"
11
        30.000 K-lag (seconds)"
77
        0.500 Beta weighting factor"
**
        17.647 Routing time step ( seconds)"
11
            1 No. of sub-reaches"
**
             Peak outflow
                                            0.027 c.m/sec"
               0.018 0.027 0.027 0.000 c.m/sec"
**
             HYDROGRAPH Combine 5"
  40
* *
             6 Combine "
**
             5 Node #"
"
77
             Maximum flow
                                          0.027 c.m/sec"
71.207 c.m"
             Hydrograph volume 71.207 c.m"

0.018 0.027 0.027 0.027"

HYDROGRAPH Confluence 206"
* *
11
11
           7 Confluence "
11
,,
          206 Node #"
**
77
             Maximum flow
                                            0.039 c.m/sec"
              Hydrograph volume
11
                                          62.446 c.m"
                      0.018 0.039 0.027 0.000"
**
         POND DESIGN"
11
       0.039 Current peak flow c.m/sec"
        0.080 Target outflow c.m/sec"
11
         62.4 Hydrograph volume c.m"
"
          Number of stages"
       223.850 Minimum water level metre" 224.150 Maximum water level metre"
77
**
       223.850 Starting water level metre"
**
         0 Keep Design Data: 1 = True; 0 = False"
                 Level Discharge Volume" 223.850 0.01000 0.4900"
11
            224.000 0.01880 11.740"
224.150 0.01980 45.490"
Peak outflow 0.019 c.m/se
Maximum level 224.019 metre'
Maximum storage 16.086 c.m"
Centroidal lag 1.684 hours"
77
ŦŦ
**
                                            0.019 c.m/sec"
**
                                         224.019 metre"
"
**
                   0.018 0.039 0.019 0.000 c.m/sec"
FF
            HYDROGRAPH Next link "
**
            5 Next link "
**
                  0.018
                               0.019 0.019
                                                    0.000"
 51
         PIPE DESIGN"
**
         0.019 Current peak flow c.m/sec"
         0.013 Manning 'n'"
         0.250 Diameter metre"
         2.000 Gradient %"
```

**

```
0.081 metre"
1.383 m/sec"
0.084 c.m/sec"
**
               Depth of flow
11
               Velocity
Pipe capacity
Critical depth
77
**
                                                  0.110 metre"
           ROUTE Pipe Route 24"
**
         23.60 Pipe Route 24 Reach length ( metre)"
         0.453 X-factor <= 0.5"
77
         12.800 K-lag (seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
11
         30.000 K-lag ( seconds)"
TF
         0.500 Beta weighting factor"
11
         13.953 Routing time step (seconds)"
FF
             1 No. of sub-reaches"
              Peak outflow 0.019 c.m/sec" 0.018 0.019 0.000 c.m/sec"
77
**
**
               HYDROGRAPH Combine 5"
,,
               6 Combine "
11
               5 Node #"
**
            Hydrograph volume 134.263 c.m" 0.018 0.019 0.019 0.045"

HYDROGRAPH Confluence 5"

7 Confluence "
**
FF
**
ŦŦ
  40
**
"
             5 Node #"
**
**

      Maximum flow
      0.045
      c.m/sec"

      Hydrograph volume
      134.263
      c.m"

      0.018
      0.045
      0.019
      0.000"

              Maximum flow
               Hydrograph volume
71
11
          PIPE DESIGN"
**
  51
**
        0.045 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
**
         0.375 Diameter metre"
"
          0.400 Gradient %"
**
                                                 0.167 metre"
0.954 m/sec"
              Depth of flow
11
              Velocity
          Pipe capacity
Critical depth
ROUTE Pipe Route 78"
**
                                                0.111 c.m/sec"
0.154 metre"
**
**
         77.50 Pipe Route 78 Reach length ( metre)"
**
         0.346 X-factor <= 0.5"
        60.918 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
0.500 X-factor <= 0.5"
77
**
         30.000 K-lag ( seconds)"
**
        0.500 Beta weighting factor"
FF
         75.000 Routing time step (seconds)"
11
             1 No. of sub-reaches"
**
              Peak outflow
                                                0.045 c.m/sec"
               0.018 0.045 0.045 0.000 c.m/sec"
11
              HYDROGRAPH Combine 3"
FF
              6 Combine "
77
             3 Node #"
77
              Maximum flow 0.045 c.m/sc

Hydrograph volume 134.263 c.m"

0.018 0.045 0.045 0.045"

HYDROGRAPH Confluence 202"
                                                0.045 c.m/sec"
**
              7 Confluence "
```

```
11
         202 Node #"
11
11
             Maximum flow
                                        0.086 c.m/sec"
             Hydrograph volume
                                     142.871
**
                                                c.m"
FF
                    0.018 0.086 0.045 0.000"
* 7
  54
             POND DESIGN"
**
        0.086 Current peak flow c.m/sec"
**
        0.080 Target outflow c.m/sec"
77
        142.9 Hydrograph volume
                                  c.m"
77
           3.
              Number of stages"
**
      223.000
              Minimum water level
                                    metre"
11
      223.150 Maximum water level
                                    metre"
77
      223.000 Starting water level metre"
**
                Keep Design Data: 1 = True; 0 = False"
            0
77
                Level Discharge Volume"
77
                223.000 0.00100 0.4900"
**
                223.075 0.08990
                                  10.370"
                223.150 0.08500 39.990"
**
11
             Peak outflow
                                        0.075 c.m/sec"
**
             Maximum level
                                       223.062 metre"
77
             Maximum storage
                                                c.m"
                                        8.670
77
                                        1.537 hours"
             Centroidal lag
                 0.018 0.086
11
                                   0.075 0.000 c.m/sec"
             HYDROGRAPH Next link "
  40
77
            5 Next link "
                    0.018
                            0.075 0.075
                                               0.000"
"
  51
           PIPE DESIGN"
17
        0.075 Current peak flow c.m/sec"
**
        0.013
               Manning 'n'"
               Diameter
        0.375
                         metre"
**
        0.400
               Gradient %"
"
             Depth of flow
                                        0.226 metre"
11
             Velocity
                                        1.079
                                                 m/sec"
             Pipe capacity
                                        0.111
                                                c.m/sec"
"
             Critical depth
                                        0.200 metre"
11
            ROUTE Pipe Route 24"
77
        23.60
              Pipe Route 24 Reach length ( metre)"
**
        0.000
              X-factor <= 0.5"
* *
       16.407 K-lag (seconds)"
77
        0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
       30.000 K-lag (seconds)"
        0.618 Beta weighting factor"
       40.000 Routing time step (seconds)"
            1 No. of sub-reaches"
,,
             Peak outflow
                                         0.074 c.m/sec"
                            0.075
                                      0.074 0.000 c.m/sec"
                   0.018
11
            HYDROGRAPH Combine 3"
  40
            6 Combine "
"
               Node #"
**
**
            Maximum flow
                                         0.116
                                               c.m/sec"
            Hydrograph volume
                                     277.130
                                                c.m"
11
                   0.018 0.075
                                     0.074
11
 40
            HYDROGRAPH Confluence
                                     3 "
**
            7 Confluence "
**
               Node #"
11
**
            Maximum flow
                                       0.116 c.m/sec"
77
            Hydrograph volume 277.130
                                                 c.m"
```

```
11
                     0.018 0.116 0.074
                                               0.000"
**
  51
         PIPE DESIGN"
        0.116 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
77
        0.450 Diameter metre"
**
        0.400 Gradient %"
**
             Depth of flow
                                         0.262 metre"
**
             Velocity
                                        1.203 m/sec"
ŦŦ
             Pipe capacity
                                         0.180 c.m/sec"
**
             Critical depth
                                         0.237 metre"
             ROUTE Pipe Route 15"
  53
"
        15.00 Pipe Route 15 Reach length (metre)"
11
        0.000 X-factor <= 0.5"
•
        9.350 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
11
        0.500 X-factor <= 0.5"
11
       30.000 K-lag (seconds)"
**
        0.737 Beta weighting factor"
**
       35.294 Routing time step (seconds)"
**
           1 No. of sub-reaches"
FF
            Peak outflow
                                         0.114 c.m/sec"
                    0.018 0.116 0.114 0.000 c.m/sec"
ŦŦ
  40
             HYDROGRAPH Combine 201"
"
            6 Combine "
77
         201 Node #"
7 7
* *
            Maximum flow
                                        0.197 c.m/sec"
            Hydrograph volume 414.424 c.m"
0.018 0.116 0.114 0.197"
HYDROGRAPH Confluence 201"
"
11
  40
**
            7 Confluence "
**
         201 Node #"
77
             Maximum flow
                                        0.197 c.m/sec"
             Hydrograph volume
                                      414.424 c.m"
**
                    0.018 0.197 0.114 0.000"
**
  51
             PIPE DESIGN"
f 7
        0.197 Current peak flow c.m/sec"
"
              Manning 'n'"
        0.013
**
        0.525 Diameter metre"
**
        0.400 Gradient %"
**
            Depth of flow
                                         0.331 metre"
,,
             Velocity
                                        1.369 m/sec"
**
                                        0.272 c.m/sec"
0.298 metre"
             Pipe capacity
"
             Critical depth
  53
             ROUTE Pipe Route 16"
77
        15.70
               Pipe Route 16 Reach length ( metre)"
11
        0.000 X-factor <= 0.5"
ŦŦ
        8.603 K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
"
        0.500 X-factor <= 0.5"
**
       30.000 K-lag (seconds)"
        0.799 Beta weighting factor"
77
       37.500 Routing time step (seconds)"
77
           1 No. of sub-reaches"
77
            Peak outflow
                                         0.195
                                                 c.m/sec"
                    0.018
                            0.197
                                      0.195
                                                0.000 c.m/sec"
"
 40
            HYDROGRAPH Next link "
11
            5 Next link "
11
                    0.018 0.195 0.195 0.000"
```

```
40
              HYDROGRAPH Undo"
                Undo"
**
                       0.018
                                 0.197 0.195 0.000"
  56
               DIVERSION"
           201 Node number"
**
         0.197
                Overflow threshold"
**
          1.000
                 Required diverted fraction"
                 Conduit type; 1=Pipe; 2=Channel"
               Peak of diverted flow
                                        0.000
                                                     c.m/sec"
              Volume of diverted flow
                                            0.000
                                                      C.m"
               DIV00201.005hyd"
**
              No Flow Diverted"
11
                       0.018
                                 0.197
                                           0.197
                                                      0.000 c.m/sec"
              HYDROGRAPH Next link "
  40
"
                Next link "
                       0.018
                                 0.197
                                           0.197
                                                      0.000"
  40
              HYDROGRAPH Undo"
**
             1 Undo"
77
                                 0.197
                       0.018
                                           0.197
                                                     0.000"
**
  56
              DIVERSION"
77
                Node number"
             3
* *
         0.197
                 Overflow threshold"
11
                 Required diverted fraction"
         1.000
11
                 Conduit type; 1=Pipe;2=Channel"
**
              Peak of diverted flow
                                          0.000
                                                     c.m/sec"
77
              Volume of diverted flow
                                            0.000
                                                      c.m"
**
              DIV00003.005hyd"
,,
              No Flow Diverted"
77
                      0.018
                               0.197
                                           0.197
                                                     0.000 c.m/sec"
**
  40
              HYDROGRAPH Next link "
**
             5 Next link "
77
                      0.018
                                 0.197
                                           0.197
                                                     0.000"
  54
              POND DESIGN"
77
         0.197
                 Current peak flow
                                      c.m/sec"
ŧŧ
         0.090
                 Target outflow
                                  c.m/sec"
**
         414.4 Hydrograph volume
                                      c.m"
**
            3. Number of stages"
11
       221.600 Minimum water level
                                         metre"
       223.100
               Maximum water level
                                        metre"
"
       221.600
                 Starting water level
                                         metre"
ŦŦ
             0
                 Keep Design Data: 1 = True; 0 = False"
**
                  Level Discharge
                                      Volume"
                 221.600 0.01000
                                    0.01000"
11
                 222.350
                         0.04960
                                      99.140"
                 223.100
                         0.06830
                                      262.660"
              Peak outflow
                                                      c.m/sec"
                                             0.059
**
              Maximum level
                                           222.712
                                                      metre"
              Maximum storage
                                           178.090
                                                      c.m"
#
              Centroidal lag
                                                     hours"
                                             2.236
                                        0.059
                   0.018
                             0.197
                                               0.000 c.m/sec"
  40
              HYDROGRAPH Next link "
**
                 Next link "
**
                      0.018
                                0.059
                                           0.059
                                                     0.000"
  51
              PIPE DESIGN"
"
         0.059
                 Current peak flow
                                      c.m/sec"
FF
                 Manning 'n'"
         0.013
FF
         0.375
                 Diameter
                             metre"
         0.400
                 Gradient
11
              Depth of flow
                                             0.194
                                                      metre"
              Velocity
                                             1.018
                                                      m/sec"
```

```
77
             Pipe capacity
                                          0.111
                                                  c.m/sec"
**
             Critical depth
                                          0.176
                                                  metre"
             ROUTE Pipe Route 59"
77
         59.40 Pipe Route 59 Reach length ( metre)"
**
         0.257 X-factor <= 0.5"
71
        43.762 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
11
        30.000 K-lag (seconds)"
**
        0.500 Beta weighting factor"
        60.000 Routing time step ( seconds)"
**
            1 No. of sub-reaches"
11
             Peak outflow
                                         0.059
                                                  c.m/sec"
**
                     0.018 0.059
                                       0.059 0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
77
            5 Next link "
**
                    0.018
                             0.059
                                       0.059
                                                0.000"
**
  51
             PIPE DESIGN"
11
        0.059 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
11
        0.375 Diameter metre"
* *
        0.400 Gradient %"
**
             Depth of flow
                                          0.194 metre"
,,
             Velocity
                                          1.017 m/sec"
**
             Pipe capacity
                                          0.111
                                                  c.m/sec"
             Critical depth
                                          0.176 metre"
"
             ROUTE Pipe Route 31"
  53
**
        31.30 Pipe Route 31 Reach length (metre)"
**
        0.040 X-factor <= 0.5"
**
       23.072 K-lag ( seconds)"
11
        0.000 Default(0) or user spec.(1) values used"
77
        0.500 	ext{ X-factor} \le 0.5"
**
       30.000 K-lag (seconds)"
**
        0.500 Beta weighting factor"
77
       42.857 Routing time step (seconds)"
**
           1 No. of sub-reaches"
**
             Peak outflow
                                          0.058 c.m/sec"
**
                     0.018 0.059
                                        0.058 0.000 c.m/sec"
  40
            HYDROGRAPH Next link "
77
            5 Next link "
FF
                    0.018
                             0.058
                                       0.058
                                                0.000"
**
  51
            PIPE DESIGN"
        0.058 Current peak flow
                                  c.m/sec"
77
        0.013 Manning 'n'"
        0.375 Diameter metre"
**
        0.400 Gradient %"
             Depth of flow
                                          0.193
                                                  metre"
77
             Velocity
                                          1.017
                                                  m/sec"
"
             Pipe capacity
                                          0.111
                                                  c.m/sec"
**
             Critical depth
                                          0.175
                                                  metre"
11
             ROUTE Pipe Route 11"
"
        10.50
                 Pipe Route 11 Reach length ( metre)"
77
        0.000 X-factor <= 0.5"
ŦŦ
        7.742 K-lag (seconds)"
FF
        0.000 Default(0) or user spec.(1) values used"
**
        0.500
               X-factor <= 0.5"
       30.000
              K-lag (seconds)"
11
        0.736 Beta weighting factor"
**
       28.571 Routing time step (seconds)"
            1
                No. of sub-reaches"
```

11 11 11 11	Peak outflow 0.058 c.m/sec" 0.018 0.058 0.058 0.000 c.m/sec" 40 HYDROGRAPH Combine 9999" 6 Combine " 9999 Node #"
## ## ## ## ##	Maximum flow 0.065 c.m/sec" Hydrograph volume 432.757 c.m" 0.018 0.058 0.058 0.065" 40 HYDROGRAPH Confluence 9999" 7 Confluence " 9999 Node #"
77 77 77 71	Maximum flow 0.065 c.m/sec" Hydrograph volume 432.757 c.m" 0.018 0.065 0.058 0.000" START/RE-START TOTALS 9999" 3 Runoff Totals on EXIT"
## ## ##	Total Catchment area 1.154 hectare" Total Impervious area 0.922 hectare" Total % impervious 79.873" EXIT"

```
11
                 MIDUSS Output ---->"
                 MIDUSS version
                                                          Version 2.25 rev. 473"
**
                 MIDUSS created
                                                                 February-07-10"
            10
                 Units used:
                                                                       ie METRIC"
**
                 Job folder:
                                                   C:\swm\17233\Update 2025-3-20"
                 Output filename:
                                                                       pst104.out"
77
                 Licensee name:
                                                                              Bob"
,,
                 Company
**
                 Date & Time last used:
                                                       25/07/2024 at 11:47:29 AM"
              TIME PARAMETERS"
**
        10.000 Time Step"
11
       180.000 Max. Storm length"
11
      1500.000 Max. Hydrograph"
  32
              STORM Chicago storm"
**
             1 Chicago storm"
**
       670.324 Coefficient A"
11
         3.007 Constant B"
,,
         0.698 Exponent C"
11
         0.400 Fraction R"
**
       180.000 Duration"
**
         1.000 Time step multiplier"
"
              Maximum intensity
                                          107.682 mm/hr"
11
              Total depth
                                           52.991
                                                     mm"
**
                 005hyd Hydrograph extension used in this file"
"
  33
              CATCHMENT 201"
"
                Rectangular"
             2
11
             1
                 Equal length"
**
             2 Horton equation"
"
           201
               No description"
7.7
       100.000 % Impervious"
**
       0.320 Total Area"
**
        80.000 Flow length"
         1.200 Overland Slope"
FF
         0.000 Pervious Area"
**
        80.000 Pervious length"
**
        1.200 Pervious slope"
        0.320 Impervious Area"
"
        80.000 Impervious length"
FF
        1.200 Impervious slope"
77
         0.250 Pervious Manning 'n'"
        25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
11
         0.500 Pervious Lag constant (hours)"
11
         7.500 Pervious Depression storage"
11
         0.015
                 Impervious Manning 'n'"
11
         0.000
                 Impervious Max.infiltration"
**
         0.000
                 Impervious Min.infiltration"
         0.500
                 Impervious Lag constant (hours)"
"
         2.000
                 Impervious Depression storage"
                      0.096 0.000 0.000
                                                     0.000 c.m/sec"
              Catchment 201
                                    Pervious
                                                 Impervious Total Area "
              Surface Area
                                     0.000
                                                 0.320 0.320 hectare"
,,
              Time of concentration 27.007
                                                4.522
                                                           4.522
                                                                      minutes"
              Time to Centroid 108.702

    108.702
    65.711

    52.991
    52.991

    0.00
    169.57

    30.008
    2.000

    22.983
    50.991

    163.17

                                                           89.711 minutes" 52.991 mm"
                                                 89.711
                                                           89.711
77
              Rainfall depth
              Rainfall volume
                                                           169.57
                                                                      c.m"
**
              Rainfall losses
                                                           2.000
                                                                      mm"
              Runoff depth
                                                                       mm"
                                                           50.991
              Runoff volume 0.00
Runoff coefficient 0.000
                                                163.17
                                                           163.17
                                                                        c.m"
                                                0.962
                                                            0.962
```

```
**
            Maximum flow
                                   0.000
                                            0.096 0.096
                                                                c.m/sec"
 40
             HYDROGRAPH Add Runoff "
**
            4 Add Runoff "
"
                    0.096
                             0.096 0.000
                                                0.000"
  51
            PIPE DESIGN"
        0.096 Current peak flow
                                  c.m/sec"
**
        0.013 Manning 'n'"
        1.000 Diameter metre"
11
        1.000 Gradient %"
             Depth of flow
                                         0.136
                                                  metre"
**
             Velocity
                                         1.488
                                                  m/sec"
77
             Pipe capacity
                                         2.398 c.m/sec"
77
             Critical depth
                                         0.170
                                                  metre"
  53
             ROUTE Zero Route"
**
                Zero Route Reach length ( metre)"
**
                     0.096 0.096 0.000 c.m/sec"
  40
             HYDROGRAPH Combine 201"
            6 Combine "
77
          201
                Node #"
**
**
             Maximum flow
                                                 c.m/sec"
                                         0.096
**
                                                 c.m"
             Hydrograph volume
                                       163.172
                                     0.096
**
                     0.096 0.096
                                                 0.096"
**
             HYDROGRAPH Start - New Tributary"
,,
            2 Start - New Tributary"
11
                            0.000
                    0.096
                                      0.096 0.096"
"
  33
             CATCHMENT 202"
**
            2 Rectangular"
**
            1
                Equal length"
**
            2 Horton equation"
77
          202 No description"
**
      100.000 % Impervious"
* *
        0.333 Total Area"
**
       83.250 Flow length"
"
        1.200 Overland Slope"
П
        0.000 Pervious Area"
**
       83.250 Pervious length"
**
        1.200 Pervious slope"
**
        0.333 Impervious Area"
77
       83.250
               Impervious length"
11
        1.200 Impervious slope"
• •
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
11
              Pervious Min.infiltration"
        5.000
**
        0.500 Pervious Lag constant (hours)"
"
        7.500
               Pervious Depression storage"
11
        0.015
               Impervious Manning 'n'"
**
               Impervious Max.infiltration"
        0.000
**
        0.000
               Impervious Min.infiltration"
"
        0.500
               Impervious Lag constant (hours)"
**
        2.000
               Impervious Depression storage"
11
                    0.100 0.000
                                       0.096
                                                0.096 \text{ c.m/sec"}
77
             Catchment 202
                                  Pervious Impervious Total Area "
**
             Surface Area
                                  0.000
                                             0.333
                                                      0.333 hectare"
11
                                                       4.631
             Time of concentration 27.660
                                             4.631
                                                                minutes"
                                                       89.773 minutes"
**
             Time to Centroid 109.192
                                             89.773
                                 52.991
"
             Rainfall depth
                                             52.991
                                                       52.991
"
             Rainfall volume
                                 0.00
                                             176.46
                                                       176.46
                                                                 c.m"
                                 30.008
77
             Rainfall losses
                                             2.000
                                                       2.000
                                                                  mm"
             Runoff depth
                                                                  mm"
                                  22.983
                                             50.991
                                                       50.991
```

```
**
              Runoff volume
                                    0.00
                                               169.80
                                                         169.80
                                                                    c.m"
                                    0.000
                                              0.962
              Runoff coefficient
                                                         0.962
**
             Maximum flow
                                    0.000
                                               0.100
                                                        0.100
                                                                    c.m/sec"
             HYDROGRAPH Add Runoff "
  40
**
             4 Add Runoff "
                     0.100
                            0.100
                                        0.096
                                                   0.096"
  51
             PIPE DESIGN"
         0.100 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
         1.000 Diameter metre"
77
         1.000 Gradient %"
             Depth of flow
                                           0.139 metre"
**
                                          1.505 m/sec"
2.398 c.m/sec"
             Velocity
             Pipe capacity
77
             Critical depth
                                          0.174
                                                    metre"
             ROUTE Zero Route"
  53
ŦŦ
          0.00 Zero Route Reach length ( metre)"
                 0.100 0.100 0.100 0.096 c.m/sec"
  40
             HYDROGRAPH Combine 202"
FF
             6 Combine "
**
                Node #"
           202
**
77
             Maximum flow
                                           0.100
                                                  c.m/sec"
77
             Hydrograph volume
                                                   c.m"
                                         169.800
**
                    0.100 0.100
                                        0.100
                                                   0.100"
11
             HYDROGRAPH Start - New Tributary"
77
             2 Start - New Tributary"
**
                     0.100
                               0.000 0.100
                                                   0.100"
  33
             CATCHMENT 206"
"
            2 Rectangular"
77
            1 Equal length"
**
            2 Horton equation"
          206 No description"
**
       74.500 % Impervious"
"
        0.174 Total Area"
11
       19.121 Flow length"
        1.200 Overland Slope"
**
**
        0.044 Pervious Area"
* *
       19.121 Pervious length"
11
        1.200 Pervious slope"
0.130 Impervious Area"
"
11
       19.121 Impervious length"
77
        1.200 Impervious slope"
       0.250 Pervious Manning 'n'"
25.000 Pervious Max.infiltration"
**
**
ŦŦ
        5.000 Pervious Min.infiltration"
,,
        0.500 Pervious Lag constant (hours)"
11
        7.500 Pervious Depression storage"
11
        0.015 Impervious Manning 'n'"
**
        0.000 Impervious Max.infiltration"
**
        0.000
                Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
**
                Impervious Depression storage"
        2.000
"
                    0.048 0.000 0.100
                                                 0.100 \text{ c.m/sec''}
**
             Catchment 206
                                   Pervious Impervious Total Area "
             Surface Area
                                   0.044 0.130 0.174 hectare"
             Time of concentration 11.443
Time to Centroid 95.985
77
                                               1.916
                                                         3.189
                                                        90.059 minutes"
77
                                               89.145
**
             Rainfall depth
                                   52.991
                                                                   mm"
                                               52.991
                                                        52.991
             Rainfall volume 23.51
                                               68.69
                                                         92.20
                                                                    C.m"
```

```
30.008
22.983
10.20
"
             Rainfall losses
                                             2.000
                                                        9.142
             Runoff depth
Runoff volume
                                             50.991
                                                       43.849
                                                                  mm"
11
                                             66.10
                                                       76.30
                                                                  c.m"
77
             Runoff coefficient
                                             0.962
                                                       0.827
11
            Maximum flow
                                   0.009
                                             0.039
                                                       0.048
                                                                  c.m/sec"
            HYDROGRAPH Add Runoff "
  40
**
            4 Add Runoff "
**
                  0.048
                            0.048
                                      0.100
                                                0.100"
  51
            PIPE DESIGN"
        0.048 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
"
        1.000 Diameter metre"
        1.000 Gradient %"
             Depth of flow
                                         0.098 metre"
Ħ
             Velocity
                                        1.208 m/sec"
tt
             Pipe capacity
                                        2.398 c.m/sec"
11
             Critical depth
                                        0.120
                                                  metre"
11
  53
             ROUTE Zero Route"
"
         0.00 Zero Route Reach length (metre)"
**
                HYDROGRAPH Combine 206"
  40
11
            6 Combine "
II
                Node #"
          206
п
             Maximum flow
                                               c.m/sec"
                                         0.048
u
             Hydrograph volume
                                        76.298 c.m"
"
                                    0.048
                    0.048 0.048
                                                 0.048"
             HYDROGRAPH Start - New Tributary"
  40
**
            2 Start - New Tributary"
"
                    0.048
                            0.000
                                      0.048 0.048"
11
  33
             CATCHMENT 204"
"
            2 Rectangular"
11
            1 Equal length"
11
            2 Horton equation"
11
          204 No description"
**
       65.000 % Impervious"
"
       0.214 Total Area"
**
       24.318 Flow length"
**
       1.200 Overland Slope"
       0.075 Pervious Area"
24.318 Pervious length"
11
11
**
       1.200 Pervious slope"
n
        0.139 Impervious Area"
       24.318 Impervious length"
11
       1.200 Impervious slope"
11
        0.250 Pervious Manning 'n'"
"
       25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
"
        0.500 Pervious Lag constant (hours)"
"
        7.500 Pervious Depression storage"
        0.015 Impervious Manning 'n'"
0.000 Impervious Max.infiltration"
11
11
11
        0.000 Impervious Min.infiltration"
11
        0.500
               Impervious Lag constant (hours)"
**
        2.000
                Impervious Depression storage"
"
                    0.055 0.000 0.048 0.048 c.m/sec"
             Catchment 204 Pervious Impervious Total Area "Surface Area 0.075 0.139 0.214 he
"
11
                                 11
             Time of concentration 13.218
                                             2.213
                                                       4.363
                                                                 minutes"
"
             Time to Centroid 97.661
                                             89.147
                                                      90.810
                                                                minutes"
```

```
Rainfall depth 52.991 52.991 52.991
Rainfall volume 39.69 73.71 113.40
Rainfall losses 30.008 2.000 11.803
Runoff depth 22.983 50.991 41.188
Runoff volume 17.21 70.93 88.14
Runoff coefficient 0.434 0.962 0.777
Maximum flow 0.013 0.042 0.055
HYDROGRAPH Add Runoff "
77
                                                                                mm"
                                                                                c.m"
**
                                                                                mm"
                                                                                mm"
**
                                                                                c.m"
                                                                                **
**
                                                                              c.m/sec"
               HYDROGRAPH Add Runoff "
  40
**
               4 Add Runoff "
                    0.055 0.055 0.048 0.048"
77
  51
            PIPE DESIGN"
          0.055 Current peak flow c.m/sec"
77
          0.013 Manning 'n'"
          1.000 Diameter metre"
**
          1.000 Gradient %"
**
               Depth of flow
                                                 0.104 metre"
7.7
                Velocity
                                                  1.259 m/sec"
                Pipe capacity
                                                 2.398 c.m/sec"
FF
                Critical depth
                                                 0.128 metre"
**
                ROUTE Zero Route"
11
           0.00 Zero Route Reach length ( metre)"
**
                     0.055 0.055 0.055 0.048 c.m/sec"
  40
                HYDROGRAPH Combine 204"
7.5
               6 Combine "
77
                   Node #"
            204
**
11
                Maximum flow
                                                 0.055 c.m/sec"
                Hydrograph volume
11
                                               88.143 c.m"
                         0.055 0.055 0.055
                                                           0.055"
  40
              HYDROGRAPH Start - New Tributary"
**
               2 Start - New Tributary"
11
                        0.055
                                    0.000
                                             0.055 0.055"
  33
               CATCHMENT 210"
Ŧ7
               2 Rectangular"
**
              1 Equal length"
            2 Horton equation"
210 No description"
.,
7 F
77
          0.000 % Impervious"
**
         0.113 Total Area"
         12.151 Flow length"
1.200 Overland Slope"
**
**
**
         0.113 Pervious Area"
11
         12.151 Pervious length"
1.200 Pervious slope"
11
**
         0.000 Impervious Area"
**
         12.151 Impervious length"
FF
         1.200 Impervious slope"
          0.250 Pervious Manning 'n'"
"
77
         25.000 Pervious Max.infiltration"
11
         5.000 Pervious Min.infiltration"
17
          0.500 Pervious Lag constant (hours)"
          7.500 Pervious Depression storage"
ŦŦ
          0.015 Impervious Manning 'n'"
FF
          0.000 Impervious Max.infiltration"
77
          0.000 Impervious Min.infiltration"
          0.500 Impervious Lag constant (hours)"
**
          2.000
                   Impervious Depression storage"
               0.026 0.000 0.055 0.055 c.m/sec"

Catchment 210 Pervious Impervious Total Area "
Surface Area 0.113 0.000 0.113 he
**
**
                                                       0.000 0.113 hectare"
```

```
**
                       Time of concentration 8.717 1.460 8.717
                                                                                                             minutes"
                    Time of Concentration 8.717 1.460 8.717 minutes"
Time to Centroid 93.910 89.143 93.910 minutes"
Rainfall depth 52.991 52.991 52.991 mm"
Rainfall volume 59.88 0.00 59.88 c.m"
Rainfall losses 30.008 2.000 30.008 mm"
Runoff depth 22.983 50.991 22.983 mm"
Runoff volume 25.97 0.00 25.97 c.m"
Runoff coefficient 0.434 0.000 0.434 "
Maximum flow 0.026 0.000 0.026 c.m/sec"
**
77
11
Ħ
11
**
11
77
**
                    HYDROGRAPH Add Runoff "
                   4 Add Runoff "
Ħ
                           0.026 0.026 0.055 0.055"
                PIPE DESIGN"
    51
**
              0.026 Current peak flow c.m/sec"
"
              0.013 Manning 'n'"
11
              1.000 Diameter metre"
77
              1.000 Gradient %"
                                                                  0.074 metre"
1.009 m/sec"
                    Depth of flow
Velocity
Pipe capacity
11
FT
**
                                                                    2.398 c.m/sec"
                    Critical depth
**
                                                                0.089 metre"
                  ROUTE Zero Route"
11
                0.00 Zero Route Reach length ( metre)"
                          0.026 0.026 0.026 0.055 c.m/sec"
11
                     HYDROGRAPH Combine 9999"
77
                  6 Combine "
**
               9999 Node #"
77

      Maximum flow
      0.026
      c.m/se

      Hydrograph volume
      25.971
      c.m"

      0.026
      0.026
      0.026

      HYDROGRAPH
      Confluence
      204"

**
                                                                   0.026 c.m/sec"
,,
77
                HYDROGRAFI.
7 Confluence "
   40
**
               204 Node #"
**
"

      Maximum flow
      0.055
      c.m/sec"

      Hydrograph volume
      88.143
      c.m"

      0.026
      0.055
      0.026
      0.000"

"
                   Maximum flow
11
             POND DESIGN"
   54
FF
             0.055 Current peak flow c.m/sec"
**
              0.080 Target outflow c.m/sec"
               88.1 Hydrograph volume c.m"

3. Number of stages"
**
          223.850 Minimum water level metre"
224.150 Maximum water level metre"
223.850 Starting water level metre"
"
FF
**
                    0 Keep Design Data: 1 = True; 0 = False"
                          Level Discharge Volume"
223.850 1.00E-07 2.490"
224.000 0.03300 13.040"
224.150 0.03430 44.580"
**
                   Peak outflow 0.033 c.m/sec"
Maximum level 224.018 metre"
Maximum storage 16.729 c.m"
Centroidal lag 1.624 hours"
11
**
                          ntroidal lag 1.624 nours" 0.026 0.055 0.033 0.000 c.m/sec"
**
                   HYDROGRAPH Next link "
                    5 Next link "
                          0.026 0.033 0.033 0.000"
" 51
                    PIPE DESIGN"
```

```
**
        0.033 Current peak flow c.m/sec"
        0.013 Manning 'n'"
77
        0.250 Diameter metre"
        1.150 Gradient %"
**
                                        0.128 metre"
1.311 m/sec"
            Depth of flow
             Velocity
17
             Pipe capacity
                                        0.064 c.m/sec"
**
                                        0.148 metre"
             Critical depth
  53
            ROUTE Pipe Route 24"
        23.60 Pipe Route 24 Reach length ( metre)"
**
        0.360 X-factor <= 0.5"
**
       13.501 K-lag ( seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
ŦŦ
       30.000 K-lag ( seconds)"
77
       0.500 Beta weighting factor"
11
       17.143 Routing time step (seconds)"
           1 No. of sub-reaches"
**
            Peak outflow
                                        0.033 c.m/sec"
"
                                      0.033 0.000 c.m/sec"
                   0.026 0.033
  40
            HYDROGRAPH Combine 5"
**
            6 Combine "
**
            5 Node #"
FF
77
            Maximum flow
                                        0.033 c.m/sec"
            Hydrograph volume
77
                                       89.594 c.m"
            0.026 0.033 0.033 0.033"

HYDROGRAPH Confluence 206"
"
FF
11
           7 Confluence "
77
         206 Node #"
**
**
            Maximum flow
                                        0.048 c.m/sec"
            Hydrograph volume
                                      76.298 c.m"
11
ŦŦ
                    0.026 0.048 0.033 0.000"
         POND DESIGN"
**
       0.048 Current peak flow c.m/sec"
**
        0.080 Target outflow c.m/sec"
**
         76.3 Hydrograph volume c.m"
11
          3. Number of stages"
      223.850 Minimum water level metre"
77
      224.150 Maximum water level metre"
**
      223.850 Starting water level metre"
"
            0 Keep Design Data: 1 = True; 0 = False"
                 Level Discharge Volume"
77
FF
               223.850 0.01000 0.4900"
               224.000 0.01880 11.740"
224.150 0.01980 45.490"
ak outflow 0
**
**
"
             Peak outflow
                                        0.019 c.m/sec"
**
            Maximum level
                                      224.047 metre"
"
                            22.265
1 717
            Maximum storage
                                                c.m"
77
                                       1.717 hours"
             Centroidal lag
                 0.026 0.048 0.019 0.000 c.m/sec"
  40
           HYDROGRAPH Next link "
**
            5 Next link "
**
                    0.026
                            0.019 0.019
                                               0.000"
 51
           PIPE DESIGN"
77
        0.019 Current peak flow c.m/sec"
        0.013 Manning 'n'"
**
        0.250 Diameter metre"
        2.000
               Gradient %"
```

```
**
               Velocity
Pipe capacity
Critical depth
                Depth of flow
                                                 0.081 metre"
1.387 m/sec"
77
                                                0.084 c.m/sec"
                                                  0.111 metre"
           ROUTE Pipe Route 24"
         23.60 Pipe Route 24 Reach length (metre)"
0.453 X-factor <= 0.5"
**
         12.764 K-lag ( seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
         0.500 X-factor <= 0.5"
         30.000 K-lag ( seconds)"
         0.500 Beta weighting factor"
**
         13.953 Routing time step ( seconds)"
               1 No. of sub-reaches"
              Peak outflow 0.019 c.m/sec" 0.026 0.019 0.019 0.000 c.m/sec"
**
FF
               HYDROGRAPH Combine 5"
               6 Combine "
FF
               5 Node #"
77
             Maximum flow 0.052 c.m/sec"
Hydrograph volume 166.090 c.m"
0.026 0.019 0.019 0.052"
HYDROGRAPH Confluence 5"
**
11
11
11
             7 Confluence "
11
             5 Node #"
**

      Maximum flow
      0.052
      c.m/sec"

      Hydrograph volume
      166.090
      c.m"

      0.026
      0.052
      0.019
      0.000"

17
77
**
          PIPE DESIGN"
  51
**
         0.052 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
"
         0.375 Diameter metre"
**
         0.400 Gradient %"
**
          Depth of flow
                                                 0.181 metre"
**
              Velocity
                                                0.989 m/sec"
          Velocity
Pipe capacity
Critical depth
ROUTE Pipe Route 78"
FF
                                                0.111 c.m/sec"
**
                                                0.165 metre"
"
         77.50 Pipe Route 78 Reach length (metre)"
**
         0.330 X-factor <= 0.5"
11
         58.787 K-lag (seconds)"
         0.000 Default(0) or user spec.(1) values used"
11
**
         0.500 X-factor <= 0.5"
**
         30.000 K-lag ( seconds)"
**
        0.500 Beta weighting factor"
        75.000 Routing time step (seconds)"
**
            1 No. of sub-reaches"
Peak outflow
ŧŧ
                                               0.052 c.m/sec"
               0.026 0.052 0.052 0.000 c.m/sec"
**
              HYDROGRAPH Combine 3"
**
             6 Combine "
             3 Node #"
11
             Maximum flow 0.052 c.m/sc

Hydrograph volume 166.090 c.m"

0.026 0.052 0.052 0.052"

HYDROGRAPH Confluence 202"
                                               0.052 c.m/sec"
              7 Confluence "
```

```
**
          202 Node #"
**
**
            Maximum flow
                                        0.100 c.m/sec"
             Hydrograph volume 169.800 c.m" 0.026 0.100 0.052 0.000"
**
11
             POND DESIGN"
11
        0.100 Current peak flow c.m/sec"
11
        0.080 Target outflow c.m/sec"
**
        169.8 Hydrograph volume c.m"
              Number of stages"
           3.
       223.000 Minimum water level
                                    metre"
**
       223.150 Maximum water level metre"
**
       223.000 Starting water level metre"
**
            0
               Keep Design Data: 1 = True; 0 = False"
"
                 Level Discharge Volume"
"
                223.000 0.00100 0.4900"
**
                223.075 0.08990 10.370"
223.150 0.08500 39.990"
11
**
             Peak outflow
                                        0.087 c.m/sec"
11
             Maximum level
                                      223.073 metre"
                                     10.091
**
             Maximum storage
                                                c.m"
                                        1.528 hours"
11
             Centroidal lag
                  0.026 0.100 0.087 0.000 c.m/sec"
11
11
            HYDROGRAPH Next link "
"
            5 Next link "
71
                   0.026 0.087 0.087 0.000"
Ħ
  51
           PIPE DESIGN"
#
        0.087 Current peak flow c.m/sec"
**
              Manning 'n'"
        0.013
11
        0.375 Diameter metre"
"
        0.400 Gradient %"
11
            Depth of flow
                                        0.251 metre"
11
            Velocity
                                         1.113 m/sec"
"
           Pipe capacity
                                        0.111 c.m/sec"
11
                                        0.217 metre"
            Critical depth
11
           ROUTE Pipe Route 24"
'n
        23.60 Pipe Route 24 Reach length ( metre)"
Ħ
        0.000 X-factor <= 0.5"
**
       15.909 K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
**
       30.000 K-lag ( seconds)"
        0.664 Beta weighting factor"
11
       46.154 Routing time step (seconds)"
            1 No. of sub-reaches"
Ħ
             Peak outflow
                                        0.086 c.m/sec"
11
                0.026 0.087
                                      0.086 0.000 c.m/sec"
            HYDROGRAPH Combine 3"
 40
**
            6 Combine "
"
               Node #"
11
             Maximum flow
                                        0.134 c.m/sec"
            Hydrograph volume
                                     335.887
                                                c.m"
                                     0.086 0.134"
             0.026 0.087
           HYDROGRAPH Confluence 3"
 40
11
            7 Confluence "
11
            3 Node #"
11
**
            Maximum flow
                                       0.134 c.m/sec"
            Maximum flow 0.134
Hydrograph volume 335.887
**
                                              c.m"
```

```
0.026 0.134 0.086 0.000"
         PIPE DESIGN"
  51
"
         0.134 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
71
         0.450 Diameter metre"
**
         0.400 Gradient %"
77
              Depth of flow
                                            0.289 metre"
**
              Velocity
                                            1.242 m/sec"
**
              Pipe capacity
                                            0.180 c.m/sec"
* *
              Critical depth
                                            0.256 metre"
11
              ROUTE Pipe Route 15"
  53
**
                 Pipe Route 15 Reach length ( metre)"
         15.00
11
         0.000 X-factor <= 0.5"
**
         9.060 K-lag (seconds)"
77
         0.000 Default(0) or user spec.(1) values used"
         0.500 X-factor <= 0.5"
11
        30.000 K-lag ( seconds)"
ŦŦ
         0.785 Beta weighting factor"
11
        37.500 Routing time step (seconds)"
            1 No. of sub-reaches"
FF
             Peak outflow
                                            0.133 c.m/sec"
**
                      0.026 0.134 0.133 0.000 c.m/sec"
  40
              HYDROGRAPH Combine 201"
**
             6 Combine "
11
           201
                 Node #"
,,
,,
              Maximum flow
                                            0.228 c.m/sec"

      Maximum flow
      0.228
      c.m/s

      Hydrograph volume
      499.058
      c.m"

      0.026
      0.134
      0.133
      0.228"

      HYDROGRAPH Confluence
      201"

* *
77
11
             7 Confluence "
**
          201 Node #"
.,
11
             Maximum flow
                                         0.228 c.m/sec"
499.058 c.m"
              Hydrograph volume
**
                      0.026 0.228 0.133 0.000"
**
           PIPE DESIGN"
**
         0.228 Current peak flow c.m/sec"
         0.013 Manning 'n'"
**
         0.525 Diameter metre"
         0.400 Gradient %"
             Depth of flow
**
                                            0.368 metre"
              Velocity
                                            1.408 m/sec"
**
                                           0.272 c.m/sec"
0.323 metre"
              Pipe capacity
             Critical depth
  53
             ROUTE Pipe Route 16"
**
         15.70 Pipe Route 16 Reach length ( metre)"
**
         0.000 X-factor <= 0.5"
77
         8.365 K-lag (seconds)"
         0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
        30.000 K-lag (seconds)"
**
        0.822 Beta weighting factor"
        46.154 Routing time step ( seconds)"
**
           1 No. of sub-reaches"
**
            Peak outflow
                                            0.226 c.m/sec"
11
                      0.026
                              0.228
                                         0.226 0.000 c.m/sec"
             HYDROGRAPH Next link "
"
             5 Next link "
ŦŦ
                      0.026 0.226 0.226 0.000"
```

11

```
40
             HYDROGRAPH Undo"
**
             1 Undo"
77
                      0.026
                              0.228 0.226 0.000"
**
  56
              DIVERSION"
**
           201 Node number"
**
               Overflow threshold"
         0.229
**
         1.000 Required diverted fraction"
**
             0 Conduit type; 1=Pipe;2=Channel"
              Peak of diverted flow
                                     0.000 c.m/sec"
              Volume of diverted flow
                                          0.000
                                                    c.m"
**
              DIV00201.005hyd"
**
              No Flow Diverted"
77
                      0.026
                                0.228
                                         0.228
                                                   0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
**
             5 Next link "
**
                      0.026
                               0.228
                                        0.228
                                                   0.000"
  40
             HYDROGRAPH Undo"
**
             1 Undo"
, ,
                     0.026
                               0.228
                                        0.228 0.000"
**
  56
             DIVERSION"
,,
             3 Node number"
**
         0.200
                Overflow threshold"
FF
         1.000 Required diverted fraction"
**
             O Conduit type; 1=Pipe; 2=Channel"
              Peak of diverted flow
                                     0.028 c.m/sec"
**
              Volume of diverted flow
                                         16.994
                                                    c.m"
77
              DIV00003.005hyd"
**
              Diversion to Dverland Flow 0.028 cms"
11
                      0.026
                              0.228
                                        0.200 0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
**
             5 Next link "
* *
                     0.026
                              0.200
                                         0.200
                                                   0.000"
**
             POND DESIGN"
  54
"
        0.200 Current peak flow c.m/sec"
**
        0.090 Target outflow c.m/sec"
,,
        482.1
               Hydrograph volume
**
            3. Number of stages"
**
       221.600 Minimum water level
11
       223.100 Maximum water level
                                      metre"
,,
       221.600
                Starting water level metre"
11
            Ω
                Keep Design Data: 1 = True; 0 = False"
**
                  Level Discharge Volume"
**
                221.600 0.01000
                                    0.01000"
                222.350 0.04960
                                    99.140"
"
                223.100 0.06830
                                    262.660"
             Peak outflow
                                           0.062
                                                  c.m/sec"
11
             Maximum level
                                         222.834
                                                  metre"
* *
                                                   c.m"
             Maximum storage
                                         204.706
**
             Centroidal lag
                                          2.298
                                                  hours"
* *
                  0.026
                           0.200
                                      0.062 0.000 c.m/sec"
             HYDROGRAPH Next link "
  40
**
                Next link "
* *
                     0.026
                              0.062
                                         0.062
                                                   0.000"
**
             PIPE DESIGN"
  51
        0.062
                Current peak flow
                                  c.m/sec"
"
        0.013
               Manning 'n'"
        0.375
                Diameter
                           metre"
                Gradient
        0.400
                           용 !!
             Depth of flow
                                           0.200
                                                    metre"
**
             Velocity
                                           1.030
                                                   m/sec"
```

```
**
             Pipe capacity
                                          0.111 c.m/sec"
             Critical depth
"
                                         0.180 metre"
             ROUTE Pipe Route 59"
11
         59.40
                Pipe Route 59 Reach length ( metre)"
         0.246 X-factor <= 0.5"
11
        43.248 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
11
        0.500 X-factor <= 0.5"
        30.000 K-lag ( seconds)"
11
        0.500 Beta weighting factor"
        60.000 Routing time step ( seconds)"
11
            1 No. of sub-reaches"
             Peak outflow
                                        0.062
                                                 c.m/sec"
11
                    0.026
                            0.062
                                       0.062 0.000 c.m/sec"
**
            HYDROGRAPH Next link "
**
            5 Next link "
**
                     0.026
                             0.062 0.062
                                               0.000"
  51
             PIPE DESIGN"
**
        0.062 Current peak flow c.m/sec"
FF
        0.013
               Manning 'n'"
ŦŦ
        0.375 Diameter metre"
**
        0.400 Gradient %"
**
             Depth of flow
                                         0.200 metre"
             Velocity
                                        1.030 m/sec"
77
             Pipe capacity
                                         0.111 c.m/sec"
"
             Critical depth
                                         0.180
                                                 metre"
11
  53
             ROUTE Pipe Route 31"
,,
        31.30 Pipe Route 31 Reach length ( metre)"
**
        0.019 	 X-factor <= 0.5"
11
       22.790 K-lag (seconds)"
77
        0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
77
       30.000 K-lag ( seconds)"
**
        0.500 Beta weighting factor"
11
       42.857 Routing time step ( seconds)"
           1 No. of sub-reaches"
**
            Peak outflow
                                         0.062 c.m/sec"
77
                     0.026 0.062
                                       0.062 0.000 c.m/sec"
  40
            HYDROGRAPH Next link "
77
            5 Next link "
**
                    0.026
                             0.062
                                      0.062
                                                0.000"
"
            PIPE DESIGN"
11
        0.062 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
        0.375 Diameter metre"
        0.400 Gradient %"
            Depth of flow
                                         0.200
11
            Velocity
                                         1.030
                                                 m/sec"
**
             Pipe capacity
                                        0.111
                                                  c.m/sec"
11
             Critical depth
                                        0.180
                                                  metre"
            ROUTE Pipe Route 11"
"
        10.50 Pipe Route 11 Reach length (metre)"
**
        0.000 X-factor <= 0.5"
11
        7.645 K-lag ( seconds)"
        0.000 Default(0) or user spec.(1) values used"
**
        0.500 X-factor <= 0.5"
**
       30.000 K-lag ( seconds) "
       0.750 Beta weighting factor"
28.571 Routing time step (seconds)"
**
77
           1 No. of sub-reaches"
```

77 77 77 77		Peak outflow 0.06 0.026 0.062 0.062 HYDROGRAPH Combine 9999" 6 Combine " 9999 Node #"	
*** *** *** *** *** *** ***	40	Maximum flow 0.07 Hydrograph volume 508.48 0.026 0.062 0.062 HYDROGRAPH Confluence 9999" 7 Confluence " 9999 Node #"	
** ** ** ** ** ** ** ** ** **	38	Maximum flow 0.07 Hydrograph volume 508.48 0.026 0.075 0.062 START/RE-START TOTALS 9999" 3 Runoff Totals on EXIT" Total Catchment area Total Impervious area Total % impervious EXIT"	30 c.m"

```
77
                  MIDUSS Output ---->"
* 1
                                                          Version 2.25 rev. 473"
                  MIDUSS version
                                                                  February-07-10"
                  MIDUSS created
            10
                 Units used:
                                                                       ie METRIC"
                  Job folder:
                                                 C:\swm\17233\Update 2025-3-20"
ŦŦ
                  Output filename:
                                                                       pst254.out"
                  Licensee name:
                                                                               Bob"
**
                  Company
11
                  Date & Time last used:
                                                       25/07/2024 at 11:51:23 AM"
11
              TIME PARAMETERS"
        10.000 Time Step"
ŦŦ
       180.000 Max. Storm length"
**
      1500.000 Max. Hydrograph"
  32
              STORM Chicago storm"
77
             1 Chicago storm"
**
       721.533 Coefficient A"
**
         2.253 Constant B"
11
         0.679 Exponent C"
**
         0.400 Fraction R"
**
       180.000
                Duration"
"
         1.000 Time step multiplier"
**
              Maximum intensity
                                          127.011 mm/hr"
"
              Total depth
                                           63.151 mm"
* *
                  005hyd Hydrograph extension used in this file"
11
  33
              CATCHMENT 201"
"
                 Rectangular"
**
             1
                 Equal length"
* *
             2 Horton equation"
**
           201 No description"
77
       100.000 % Impervious"
,,
         0.320 Total Area"
"
        80.000 Flow length"
77
         1.200 Overland Slope"
         0.000 Pervious Area"
**
        80.000 Pervious length"
\mathbf{n}
        1.200 Pervious slope"
11
         0.320 Impervious Area"
**
        80.000 Impervious length"
FF
        1.200 Impervious slope"
77
         0.250 Pervious Manning 'n'"
**
        25.000 Pervious Max.infiltration"
**
         5.000 Pervious Min.infiltration"
77
         0.500 Pervious Lag constant (hours)"
77
         7.500 Pervious Depression storage"
         0.015
                 Impervious Manning 'n'"
**
         0.000 Impervious Max.infiltration"
         0.000 Impervious Min.infiltration"
11
         0.500
                 Impervious Lag constant (hours)"
         2.000
                 Impervious Depression storage"
"
                      0.113 0.000 0.000
                                                    0.000 c.m/sec"
              Catchment 201
                                     Pervious Impervious Total Area "
              Surface Area
                                                 0.320 0.320 hectare"
                                     0.000
              Time of concentration 23.743
                                                 4.233
                                                            4.233
                                                                       minutes"
**
              Time to Centroid 109.051
                                                89.371
                                                           89.371
                                                                      minutes"
              Rainfall depth 63.151 63.151
Rainfall volume 0.00 202.08
Rainfall losses 30.582 2.000 2.000
Runoff depth 32.569 61.151 61.151
Runoff volume 0.00 195.68
Runoff coefficient 0.000 0.968 0.968
FF
                                                                      mm"
                                                            63.151
FF
                                                                        c.m"
                                                                      mm"
FF
                                                                       mm"
77
                                                                        c.m"
11
```

```
77
             Maximum flow
                                  0.000 0.113 0.113 c.m/sec"
  40
             HYDROGRAPH Add Runoff "
77
            4 Add Runoff "
11
                     0.113
                            0.113 0.000 0.000"
            PIPE DESIGN"
  51
         0.113 Current peak flow c.m/sec"
FF
         0.013 Manning 'n'"
         1.000 Diameter
                          metre"
77
         1.000 Gradient %"
             Depth of flow
                                         0.148 metre"
**
             Velocity
                                         1.563 m/sec"
ŦŦ
             Pipe capacity
                                        2.398 c.m/sec"
11
             Critical depth
                                         0.185
                                                 metre"
             ROUTE Zero Route"
* *
                Zero Route Reach length ( metre)"
,,
                    0.113 0.113 0.000 c.m/sec"
  40
             HYDROGRAPH Combine 201"
77
            6 Combine "
**
                Node #"
          201
77
**
             Maximum flow
                                        0.113 c.m/sec"
**
             Hydrograph volume
                                       195.683
                                                c.m"
                    0.113 0.113 0.113
**
                                                0.113"
  40
             HYDROGRAPH Start - New Tributary"
**
            2 Start - New Tributary"
77
                    0.113
                           0.000 0.113 0.113"
  33
             CATCHMENT 202"
11
            2 Rectangular"
**
            1
               Equal length"
**
            2 Horton equation"
          202 No description"
* *
      100.000 % Impervious"
        0.333 Total Area"
11
       83.250 Flow length"
        1.200 Overland Slope"
**
        0.000 Pervious Area"
       83.250 Pervious length"
77
        1.200 Pervious slope"
ff
        0.333 Impervious Area"
,,
       83.250 Impervious length"
        1.200 Impervious slope"
77
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
"
        7.500 Pervious Depression storage"
77
        0.015
               Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
        0.000
               Impervious Min.infiltration"
        0.500
               Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
                    0.117 0.000 0.113
                                               0.113 \text{ c.m/sec''}
             Catchment 202
                                 Pervious Impervious Total Area "
             Surface Area
                                 0.000
                                            0.333 hectare"
                                                      4.335 minutes"
89.429 minutes"
63.151
FF
             Time of concentration 24.317
                                            4.335
             Time to Centroid 109.549 89.429
**
             Rainfall depth
                                 63.151 63.151
                                                      63.151
             Rainfall volume
                                0.00
                                            210.29
                                                      210.29
                                                                c.m"
11
             Rainfall losses
Runoff depth
                                30.582
                                           2.000
                                                      2.000
                                                                 mm"
                                 32.569 61.151
                                                      61.151
                                                                 mm"
```

```
203.63
11
                                   0.00
             Runoff volume
                                                         203.63
                                                                  c.m"
                                  0.000
**
             Runoff coefficient
                                              0.968
                                                        0.968
77
             Maximum flow
                                    0.000
                                              0.117
                                                       0.117
                                                                   c.m/sec"
             HYDROGRAPH Add Runoff "
"
             4 Add Runoff "
77
                     0.117 0.117
                                       0.113
                                                0.113"
  51
             PIPE DESIGN"
**
         0.117 Current peak flow c.m/sec"
11
         0.013 Manning 'n'"
         1.000 Diameter metre"
7.7
         1.000 Gradient %"
**
             Depth of flow
                                          0.151
                                                   metre"
**
             Velocity
                                          1.581 m/sec"
11
             Pipe capacity
                                         2.398 c.m/sec"
11
             Critical depth
                                         0.189
                                                   metre"
**
  53
             ROUTE Zero Route"
11
          0.00 Zero Route Reach length (metre)"
**
                 40
             HYDROGRAPH Combine 202"
77
            6 Combine "
          202
                Node #"
"
                7.7
77
             Maximum flow
                                          0.117
                                                  c.m/sec"
,,
             Hydrograph volume
                                        203.632
                                                  c.m"
77
                    0.117 0.117 0.117
                                                  0.117"
77
  40
             HYDROGRAPH Start - New Tributary"
**
            2 Start - New Tributary"
**
                     0.117
                             0.000
                                     0.117 0.117"
  33
             CATCHMENT 206"
**
            2 Rectangular"
**
            1 Equal length"
**
            2 Horton equation"
77
          206 No description"
**
       74.500 % Impervious"
**
        0.174 Total Area"
fT
       19.121 Flow length"
77
        1.200 Overland Slope"
* *
        0.044 Pervious Area"
**
       19.121 Pervious length"
        1.200 Pervious slope"
"
11
        0.130 Impervious Area"
       19.121 Impervious length"
11
        1.200 Impervious slope"
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
11
        5.000 Pervious Min.infiltration"
77
        0.500 Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
**
        0.015 Impervious Manning 'n'"
11
        0.000 Impervious Max.infiltration"
ŦŦ
        0.000
                Impervious Min.infiltration"
11
        0.500
                Impervious Lag constant (hours)"
**
        2.000
                Impervious Depression storage"
**
                     0.060 0.000 0.117 0.117 c.m/sec"
             Catchment 206
Surface Area
                               Pervious Impervious Total Area "
**
                                  0.044
                                              0.130 0.174 hectare"
             Time of concentration 10.060 1.793
Time to Centroid 97.246 88.972
Rainfall depth 63.151 63.151
**
                                                               minutes"
minutes"
                                                        3.068
**
                                                       90.248
             Rainfall depth
Rainfall volume
                                                                 mm"
                                                       63.151
77
                                  28.02
                                              81.86
                                                        109.88
                                                                  c.m"
```

```
**
             Rainfall losses
                                    30.582
                                               2.000
                                                          9.288
                                                                     mm"
                                    32.569
             Runoff depth
Runoff volume
                                               61.151
                                                          53.862
                                                                     mm"
**
                                    14.45
                                               79.27
                                                          93.72
                                                                     c.m"
             Runoff coefficient
                                    0.516
                                               0.968
                                                          0.853
77
             Maximum flow
                                    0.014
                                               0.046
                                                          0.060
                                                                     c.m/sec"
             HYDROGRAPH Add Runoff "
**
             4 Add Runoff "
77
                     0.060
                              0.060
                                         0.117
                                                   0.117"
**
  51
             PIPE DESIGN"
         0.060 Current peak flow c.m/sec"
77
                Manning 'n'"
         0.013
* *
         1.000
                Diameter metre"
**
                Gradient %"
         1.000
77
             Depth of flow
                                           0.109
                                                    metre"
**
             Velocity
                                           1.293
                                                    m/sec"
**
             Pipe capacity
                                           2.398
                                                    c.m/sec"
"
             Critical depth
                                           0.134
                                                    metre"
11
  53
             ROUTE Zero Route"
,,
          0.00
                Zero Route Reach length (metre)"
"
                    0.060 0.060 0.060 0.117 c.m/sec"
  40
             HYDROGRAPH Combine 206"
**
             6
                Combine "
**
          206
                Node #"
11
77
             Maximum flow
                                           0.060
                                                  c.m/sec"
,,
             Hydrograph volume
                                          93.721
                                                   c.m"
                     0.060 0.060
                                         0.060
                                                   0.060"
  40
             HYDROGRAPH Start - New Tributary"
71
             2 Start - New Tributary"
* *
                               0.000
                     0.060
                                         0.060
                                                   0.060"
,,
  33
             CATCHMENT 204"
11
            2 Rectangular"
77
            1 Equal length"
11
            2 Horton equation"
,,
          204 No description"
..
       65.000 % Impervious"
11
                Total Area"
        0.214
• •
       24.318 Flow length"
**
        1.200 Overland Slope"
**
        0.075 Pervious Area"
"
       24.318 Pervious length"
77
        1.200 Pervious slope"
**
        0.139 Impervious Area"
       24.318 Impervious length"
**
        1.200 Impervious slope"
"
        0.250 Pervious Manning 'n'"
11
       25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
ŧŧ
        0.500 Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
**
        0.015
                Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
FF
        0.000
                Impervious Min.infiltration"
ŢŢ
        0.500
                Impervious Lag constant (hours)"
۲ŧ
        2.000
                Impervious Depression storage"
                     0.070 0.000 0.060
                                                   0.060 c.m/sec"
"
             Catchment 204
                                   Pervious Impervious Total Area "
             Surface Area
11
                                   0.075
                                                         0.214 hectare"
                                               0.139
77
             Time of concentration 11.621
                                               2.072
                                                         4.200
                                                                    minutes"
             Time to Centroid 98.868
                                               88.972
                                                          91.178
                                                                   minutes"
```

```
63.151 63.151
47.30 87.84
30.582 2.000
32.569 61.151
77
              Rainfall depth
                                               63.151
                                                          63.151
              Rainfall volume
                                                          135.14
                                                                    c.m"
              Rainfall losses
                                                         12.004
                                                                    mm"
              Runoff depth
                                                        51.147
                                                                    mm"
77
             Runoff volume
                                   24.39
                                                          109.45
                                               85.06
                                                                    c.m"
             Runoff coefficient
                                  0.516
                                              0.968
                                                          0.810
                                                                    **
77
             Maximum flow
                                    0.021
                                               0.049
                                                          0.070
                                                                    c.m/sec"
             HYDROGRAPH Add Runoff "
  40
             4 Add Runoff "
                  0.070 0.070
                                        0.060
                                                 0.060"
77
  51
            PIPE DESIGN"
         0.070 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
         1.000 Diameter metre"
77
         1.000 Gradient %"
             Depth of flow
                                           0.117 metre"
11
             Velocity
                                           1.354
                                                   m/sec"
             Pipe capacity
                                          2.398 c.m/sec"
**
             Critical depth
                                          0.145 metre"
"
  53
             ROUTE Zero Route"
"
          0.00
                Zero Route Reach length ( metre)"
                    0.070 0.070 0.070 0.060 c.m/sec"
  40
             HYDROGRAPH Combine 204"
11
            6 Combine "
**
                Node #"
          204
**
77
             Maximum flow
                                          0.070
                                                   c.m/sec"
ŦŦ
             Hydrograph volume
                                         109.455
                                                   c.m"
**
                     0.070 0.070
                                                   0.070"
                                         0.070
  40
             HYDROGRAPH Start - New Tributary"
Ħ
            2 Start - New Tributary"
7 7
                     0.070
                               0.000
                                         0.070
                                                 0.070"
  33
             CATCHMENT 210"
11
            2 Rectangular"
II
            1 Equal length"
77
            2 Horton equation"
          210 No description"
**
        0.000 % Impervious"
11
        0.113 Total Area"
       12.151 Flow length"
1.200 Overland Slope"
**
**
        0.113 Pervious Area"
"
       12.151 Pervious length"
        1.200 Pervious slope"
0.000 Impervious Area"
11
FF
"
       12.151 Impervious length"
**
        1.200 Impervious slope"
"
        0.250 Pervious Manning 'n'"
11
       25.000 Pervious Max.infiltration"
11
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
        7.500 Pervious Depression storage"
"
        0.015 Impervious Manning 'n'"
11
        0.000 Impervious Max.infiltration"
**
        0.000
                Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
"
        2.000
                Impervious Depression storage"
11
                     0.036 0.000 0.070
                                                  0.070 c.m/sec"
11
             Catchment 210
                                  Pervious
                                              Impervious Total Area "
             Surface Area
                                   0.113
                                               0.000 0.113 hectare"
```

```
Time of concentration 7.664 1.366 7.664 minutes"
Time to Centroid 95.967 88.972 95.967 minutes"
Rainfall depth 63.151 63.151 63.151 mm"
Rainfall volume 71.36 0.00 71.36 c.m"
Rainfall losses 30.582 2.000 30.582 mm"
Runoff depth 32.569 61.151 32.569 mm"
Runoff volume 36.80 0.00 36.80 c.m"
Runoff coefficient 0.516 0.000 0.516 "
Maximum flow 0.036 0.000 0.036 c.m/sec"
HYDROGRAPH Add Runoff "
11
**
11
**
**
**
**
11
                    HYDROGRAPH Add Runoff "
* *
                   4 Add Runoff"
* *
                          0.036 0.036 0.070 0.070"
               PIPE DESIGN"
77
    51
* *
              0.036 Current peak flow c.m/sec"
77
              0.013 Manning 'n'"
77
              1.000 Diameter metre"
**
              1.000 Gradient %"
                  Depth of flow
Velocity
Pipe capacity
Critical depth
**
                                                                    0.086 metre"
77
                                                                   1.112 m/sec"
2.398 c.m/sec"
0.104 metre"
**
"
                   ROUTE Zero Route"
   53
**
               0.00 Zero Route Reach length (metre)"
**
                           0.036 0.036 0.036 0.070 c.m/sec"
   40
                     HYDROGRAPH Combine 9999"
                 6 Combine "
77
17
              9999 Node #"
11
                          77
                   Maximum flow 0.036 c.m/sec"
Hydrograph volume 36.802 c.m"
0.036 0.036 0.036 0.036"
HYDROGRAPH Confluence 204"
**
*
"
   40
                 7 Confluence "
               204 Node #"
* *
**

      Maximum flow
      0.070
      c.m/sec"

      Hydrograph volume
      109.455
      c.m"

      0.036
      0.070
      0.036
      0.000"

**
77
             POND DESIGN"
**
**
              0.070 Current peak flow c.m/sec"
77
              0.080 Target outflow c.m/sec"
          109.5 Hydrograph volume c.m"

3. Number of stages"

223.850 Minimum water level metre"

224.150 Maximum water level metre"
77
**
11
**
           223.850 Starting water level metre"
11
                    0 Keep Design Data: 1 = True; 0 = False"
                          Level Discharge Volume"
223.850 1.00E-07 2.490"
224.000 0.03300 13.040"
224.150 0.03430 44.580"
**
                    Peak outflow 0.033 c.m/sec"
Maximum level 224.055 metre"
Maximum storage 24.542 c.m"
Centroidal lag 1.654 hours"
11
**
**
                        0.036 0.070 0.033 0.000 c.m/sec"
                   HYDROGRAPH Next link "
                    5 Next link "
                          0.036 0.033 0.033 0.000"
" 51
                    PIPE DESIGN"
```

```
**
          0.033 Current peak flow c.m/sec"
ff
          0.013 Manning 'n'"
          0.250 Diameter metre"
1.150 Gradient %"
**
              Depth of flow
                                              0.129 metre"
**
               Velocity
                                              1.315 m/sec"
0.064 c.m/sec"
77
               Pipe capacity
**
               Critical depth
                                              0.148 metre"
   53
               ROUTE Pipe Route 24"
**
          23.60
                  Pipe Route 24 Reach length ( metre)"
**
         0.359 X-factor <= 0.5"
**
         13.463 K-lag (seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
         30.000 K-lag ( seconds)"
11
         0.500 Beta weighting factor"
"
         17.143 Routing time step (seconds)"
* 7
             1 No. of sub-reaches"
71
              Peak outflow
                                              0.033 c.m/sec"
                  0.036 0.033
**
                                           0.033 0.000 c.m/sec"
  40
              HYDROGRAPH Combine 5"
**
              6 Combine "
                Node #"
,,
                  **
11
             Hydrograph volume 0.033 c.m/sc

0.036 0.033 0.033 0.033"

HYDROGRAPH Confluence 206"
              Maximum flow
                                             0.033 c.m/sec"
**
7.5
* *
  40
**
            7 Confluence "
**
          206 Node #"
17
                  **
FF
             Maximum flow
                                             0.060 c.m/sec"
              Hydrograph volume
11
                    graph volume 93.721 c.m" 0.036 0.060 0.033 0.000"
ŦŦ
FF
          POND DESIGN"
11
         0.060 Current peak flow c.m/sec"
**
         0.080 Target outflow c.m/sec"
11
          93.7 Hydrograph volume c.m"

3. Number of stages"
ŦŦ
11
       223.850 Minimum water level metre"
224.150 Maximum water level metre"
223.850 Starting water level metre"
**
77
              0 Keep Design Data: 1 = True; 0 = False"
                   Level Discharge Volume"
                  223.850 0.01000 0.4900"
224.000 0.01880 11.740"
224.150 0.01980 45.490"
11
11
77
              Peak outflow
                                              0.019 c.m/sec"
**
              Maximum level
                                           224.082 metre"
             Maximum storage 30.149 c.m"
Centroidal lag 1.765 hours"
FT
                   0.036 0.060 0.019 0.000 c.m/sec"
11
  40
             HYDROGRAPH Next link "
**
              5 Next link "
**
                  0.036
                                0.019 0.019
                                                      0.000"
77
             PIPE DESIGN"
         0.019 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
         0.250 Diameter metre"
11
         2.000 Gradient %"
```

```
77
                 Depth of flow
                                                  0.082 metre"
**
                                                  1.391 m/sec"
0.084 c.m/sec"
                 Velocity
                Pipe capacity
Critical depth
77
                                                  0.111 metre"
77
               ROUTE Pipe Route 24"
**
         23.60 Pipe Route 24 Reach length (metre)"
**
          0.453 X-factor <= 0.5"
         12.721 K-lag ( seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
11
          0.500 X-factor <= 0.5"
**
         30.000 K-lag (seconds)"
          0.500 Beta weighting factor"
FF
         13.636 Routing time step (seconds)"
              1 No. of sub-reaches"
**
**
               Peak outflow
                                                  0.019 c.m/sec"
                0.036 0.019 0.019 0.000 c.m/sec"
11
   40
               HYDROGRAPH Combine 5"
11
               6 Combine "
ŦŦ
               5 Node #"
,,
FF

      Maximum flow
      0.053
      c.m/se

      Hydrograph volume
      204.771
      c.m"

      0.036
      0.019
      0.019
      0.053"

      HYDROGRAPH
      Confluence
      5"

               Maximum flow
                                                 0.053 c.m/sec"
11
  40
               7 Confluence "
11
             5 Node #"
**
               Maximum flow
Hydrograph volume
                                                 0.053 c.m/sec"
                 Hydrograph volume 204.771 c.m" 0.036 0.053 0.019 0.000"
**
           PIPE DESIGN"
11
          0.053 Current peak flow c.m/sec"
**
          0.013 Manning 'n'"
**
          0.375 Diameter metre"
          0.400 Gradient %"
             Depth of flow
                                                0.182 metre"
0.992 m/sec"
0.111 c.m/sec"
0.166 metre"
**
               Velocity
              Pipe capacity
Critical depth
11
**
             ROUTE Pipe Route 78"
  53
**
          77.50 Pipe Route 78 Reach length (metre)"
**
          0.329 X-factor <= 0.5"
**
         58.614 K-lag (seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
11
         0.500 X-factor <= 0.5"
         30.000 K-lag ( seconds)"
**
         0.500 Beta weighting factor"
11
         75.000 Routing time step ( seconds)"
             1 No. of sub-reaches"
**
               Peak outflow
                                                 0.053 c.m/sec"
                0.036 0.053 0.053 0.000 c.m/sec"
"
              HYDROGRAPH Combine 3"
**
             6 Combine "
               3 Node #"
77
                   11

      Maximum flow
      0.053
      c.m/se

      Hydrograph volume
      204.771
      c.m"

      0.036
      0.053
      0.053
      0.053"

      HYDROGRAPH
      Confluence
      202"

**
                                               0.053 c.m/sec"
**
FF
               7 Confluence "
```

```
**
          202 Node #"
**
77
             Maximum flow
                                        0.117 c.m/sec"
             Hydrograph volume
                                     203.632 c.m"
**
                    0.036 0.117 0.053 0.000"
11
  54
             POND DESIGN"
**
        0.117 Current peak flow c.m/sec"
**
               Target outflow c.m/sec"
        0.080
**
        203.6 Hydrograph volume c.m"
77
           3.
              Number of stages"
77
      223.000 Minimum water level metre"
77
      223.150 Maximum water level metre"
**
      223.000 Starting water level metre"
77
               Keep Design Data: 1 = True; 0 = False"
            0
FF
                Level Discharge Volume"
ff
                223.000 0.00100 0.4900"
**
                223.075 0.08990 10.370"
               223.150 0.08500 39.990"
11
ff
             Peak outflow
                                        0.089 c.m/sec"
ŦŦ
            Maximum level
                                      223.084 metre"
            Maximum storage
**
                                                c.m"
                                       13.922
11
                                        1.529 hours"
             Centroidal lag
                 0.036 0.117
**
                                   0.089 0.000 c.m/sec"
* *
            HYDROGRAPH Next link "
,,
            5 Next link "
77
                   0.036 0.089 0.089 0.000"
77
  51
           PIPE DESIGN"
**
        0.089 Current peak flow c.m/sec"
11
        0.013
               Manning 'n'"
**
        0.375
               Diameter metre"
77
        0.400
               Gradient %"
**
            Depth of flow
                                        0.255 metre"
11
                                         1.117
            Velocity
                                                 m/sec"
77
            Pipe capacity
                                        0.111 c.m/sec"
**
            Critical depth
                                        0.219 metre"
77
            ROUTE Pipe Route 24"
**
        23.60 Pipe Route 24 Reach length ( metre)"
**
        0.000
              X-factor <= 0.5"
**
       15.849
              K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
,,
        0.500
              X-factor <= 0.5"
**
       30.000
              K-laq (seconds)"
        0.674
              Beta weighting factor"
**
       46.154 Routing time step (seconds)"
            1 No. of sub-reaches"
**
            Peak outflow
                                         0.088 c.m/sec"
11
                   0.036
                            0.089
                                      0.088 0.000 c.m/sec"
"
            HYDROGRAPH Combine 3"
 40
77
            6 Combine "
77
            3
               Node #"
ŦŦ
77
            Maximum flow
                                         0.136 c.m/sec"
            Hydrograph volume
**
                                     413.715
                                                c.m"
* *
                   0.036 0.089
                                     0.088
                                               0.136"
**
 40
           HYDROGRAPH Confluence
                                     3"
**
            7 Confluence "
**
            3 Node #"
**
**
            Maximum flow
                                       0.136
                                                c.m/sec"
            Hydrograph volume 413.715
**
                                                 c.m"
```

```
**
                       0.036 0.136 0.088
                                                   0.000"
" 51
          PIPE DESIGN"
         0.136 Current peak flow c.m/sec"
"
         0.013 Manning 'n'"
**
         0.450 Diameter metre"
11
         0.400 Gradient %"
                                            0.293 metre"
              Depth of flow
11
              Velocity
                                            1.247 m/sec"
              Pipe capacity
                                            0.180 c.m/sec"
              Critical depth
                                            0.259
                                                     metre"
              ROUTE Pipe Route 15"
**
         15.00 Pipe Route 15 Reach length ( metre)"
Tt
         0.000 X-factor <= 0.5"
**
         9.025 K-lag (seconds)"
         0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
"
        30.000 K-lag ( seconds) "
**
         0.777 Beta weighting factor"
Ħ
        40.000 Routing time step (seconds)"
**
            1 No. of sub-reaches"
11
              Peak outflow
                                            0.135 c.m/sec"
**
                      0.036 0.136 0.135 0.000 c.m/sec"
  40
              HYDROGRAPH Combine 201"
11
             6 Combine "
11
          201 Node #"
11
11
             Maximum flow
                                            0.248 c.m/sec"
             Hydrograph volume 609.398 c.m"

0.036 0.136 0.135 0.248"

HYDROGRAPH Confluence 201"
**
11
  40
"
            7 Confluence "
"
          201 Node #"
"
11
              Maximum flow
                                            0.248 c.m/sec"

      Maximum flow
      0.248
      c.m/s

      Hydrograph volume
      609.398
      c.m"

      0.036
      0.248
      0.135
      0.000"

11
11
  51
          PIPE DESIGN"
11
         0.248 Current peak flow c.m/sec"
"
         0.013 Manning 'n'"
11
         0.525 Diameter metre"
**
         0.400 Gradient %"
"
             Depth of flow
                                            0.394 metre"
"
                                            1.424 m/sec"
              Velocity
11
                                            0.272 c.m/sec"
0.337 metre"
              Pipe capacity
              Critical depth
  53
              ROUTE Pipe Route 16"
11
         15.70 Pipe Route 16 Reach length (metre)"
11
         0.000 X-factor <= 0.5"
         8.267 K-lag ( seconds) "
**
         0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
**
        30.000 K-lag (seconds)"
        0.856 Beta weighting factor"
"
        50.000 Routing time step (seconds)"
11
            1 No. of sub-reaches"
Ħ
             Peak outflow
                                            0.246
                                                     c.m/sec"
                      0.036 0.248
                                         0.246 0.000 c.m/sec"
11
 40
             HYDROGRAPH Next link "
"
             5 Next link "
11
                      0.036 0.246 0.246 0.000"
```

```
40
              HYDROGRAPH Undo"
                Undo"
11
                       0.036
                                 0.248 0.246 0.000"
  56
              DIVERSION"
**
           201 Node number"
77
         0.200
                 Overflow threshold"
11
         1.000
                 Required diverted fraction"
                 Conduit type; 1=Pipe; 2=Channel"
              Peak of diverted flow
                                            0.048
                                                     c.m/sec"
              Volume of diverted flow
                                            28.915
                                                      c.m"
77
              DIV00201.005hyd"
**
              Divert to Overland Flow 0.048 cms"
**
                       0.036 0.248
                                          0.200
                                                     0.000 c.m/sec"
  40
              HYDROGRAPH Next link "
**
             5 Next link "
**
                       0.036
                                 0.200
                                           0.200
                                                     0.000"
              HYDROGRAPH Undo"
  40
* *
                Undo"
,,
                      0.036
                                 0.248
                                           0.200
                                                     0.000"
**
  56
              DIVERSION"
,,
             3 Node number"
11
         0.200
                 Overflow threshold"
         1.000
                 Required diverted fraction"
**
                 Conduit type; 1=Pipe; 2=Channel"
,,
              Peak of diverted flow
                                            0.048
                                                      c.m/sec"
"
              Volume of diverted flow
                                            28.915
"
              DIV00003.005hvd"
**
              Diversion to Dverland Flow 0.048 cms"
**
                      0.036
                                 0.248
                                          0.200
                                                     0.000 c.m/sec"
              HYDROGRAPH Next link "
  40
**
                 Next link "
"
                      0.036
                                0.200
                                           0.200
                                                     0.000"
**
              POND DESIGN"
* *
         0.200
                 Current peak flow
                                      c.m/sec"
**
         0.090
                Target outflow c.m/sec"
77
         580.5 Hydrograph volume
                                       c.m"
* 7
            3. Number of stages"
       221.600 Minimum water level
,,
                                        metre"
**
       223.100 Maximum water level
                                       metre"
77
       221.600
                 Starting water level
                                         metre"
**
             0
                 Keep Design Data: 1 = True; 0 = False"
                   Level Discharge
                                      Volume"
11
                 221.600 0.01000
                                      0.01000"
                 222.350
                           0.04960
                                     99.140"
                 223.100 0.06830
                                      262.660"
              Peak outflow
                                             0.066
                                                      c.m/sec"
              Maximum level
                                           223.026
                                                     metre"
              Maximum storage
                                           246.502
                                                      c.m"
"
              Centroidal lag
                                             2.391
                                                     hours"
                   0.036
                                        0.066 0.000 c.m/sec"
                            0.200
  40
              HYDROGRAPH Next link "
**
                 Next link "
**
                      0.036
                                0.066
                                           0.066
                                                     0.000"
  51
              PIPE DESIGN"
**
         0.066 Current peak flow
                                     c.m/sec"
**
         0.013
                 Manning 'n'"
**
         0.375
                            metre"
                 Diameter
77
                            용 !!
         0.400
                 Gradient
**
              Depth of flow
                                             0.209
                                                      metre"
**
              Velocity
                                             1.049
                                                      m/sec"
```

```
**
             Pipe capacity
                                        0.111 c.m/sec"
             Critical depth
"
                                       0.188 metre"
**
            ROUTE Pipe Route 59"
  53
        59.40 Pipe Route 59 Reach length ( metre)"
11
**
        0.227 X-factor <= 0.5"
       42.465 K-lag ( seconds)"
**
       0.000 Default(0) or user spec.(1) values used"
       0.500 X-factor <= 0.5"
tt
       30.000 K-lag (seconds)"
        0.500 Beta weighting factor"
**
       60.000 Routing time step (seconds)"
**
           1 No. of sub-reaches"
"
            Peak outflow
                                       0.066 c.m/sec"
п
                    0.036 0.066
                                    0.066 0.000 c.m/sec"
n
            HYDROGRAPH Next link "
  40
TT.
            5 Next link "
11
                 0.036 0.066
                                     0.066
                                              0.000"
##
          PIPE DESIGN"
11
        0.066 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
"
        0.375 Diameter metre"
11
        0.400 Gradient %"
117
            Depth of flow
                                       0.209 metre"
11
                                       1.049 m/sec"
            Velocity
            Pipe capacity
                                        0.111 c.m/sec"
11
            Critical depth
                                       0.188 metre"
**
 53
           ROUTE Pipe Route 31"
**
        31.30 Pipe Route 31 Reach length (metre)"
"
       0.000 X-factor <= 0.5"
"
       22.382 K-lag (seconds)"
"
       0.000 Default(0) or user spec.(1) values used"
11
       0.500 X-factor <= 0.5"
       30.000 K-lag ( seconds) "
11
       0.509 Beta weighting factor"
**
       42.857 Routing time step (seconds)"
11
           1 No. of sub-reaches"
"
            Peak outflow
                                       0.066 c.m/sec"
11
                    0.036 0.066
                                     0.066 0.000 c.m/sec"
11
           HYDROGRAPH Next link "
 40
11
           5 Next link "
11
                   0.036 0.066 0.066
                                              0.000"
H.
 51
          PIPE DESIGN"
11
        0.066 Current peak flow c.m/sec"
        0.013 Manning 'n'"
**
        0.375 Diameter metre"
        0.400 Gradient %"
"
            Depth of flow
                                        0.209 metre"
            Velocity
                                        1.049 m/sec"
11
                                       0.111 c.m/sec"
0.187 metre"
            Pipe capacity
          Critical depth
 53
           ROUTE Pipe Route 11"
        10.50 Pipe Route 11 Reach length ( metre)"
11
        0.000 X-factor <= 0.5"
        7.510 K-lag (seconds)"
11
       0.000 Default(0) or user spec.(1) values used"
       0.500 X-factor <= 0.5"
11
       30.000 K-lag ( seconds)"
       0.761 Beta weighting factor"
11
       30.000 Routing time step ( seconds)"
               No. of sub-reaches"
```

11 11 11 11	Peak outflow 0.036 0.066 40 HYDROGRAPH Combine 6 Combine " 9999 Node #"	0.066	c.m/sec" 0.000 c.m/sec	; "
"" "" "" "" "" "" "" "" "" "" "" "" ""	Maximum flow Hydrograph volume 0.036 0.066 HYDROGRAPH Confluence 7 Confluence " 9999 Node #"	616.501 0.066		
"" "" "" "" "" ""	Maximum flow Hydrograph volume 0.036 0.087 START/RE-START TOTALS 99 Runoff Totals on EXIT Total Catchment area Total Impervious area Total % impervious EXIT"	99"	c.m"	hectare" hectare"

```
**
                 MIDUSS Output ----->"
11
                                                       Version 2.25 rev. 473"
                 MIDUSS version
•
                                                              February-07-10"
                MIDUSS created
            10
                Units used:
                                                                    ie METRIC"
**
                 Job folder:
                                                 C:\swm\17233\Update 2025-3-20"
**
                 Output filename:
                                                                    pst504.out"
77
                Licensee name:
                                                                          Bob"
77
                 Company
**
                 Date & Time last used:
                                                     25/07/2024 at 12:07:54 PM"
  31
             TIME PARAMETERS"
       10.000 Time Step"
11
**
       180.000 Max. Storm length"
**
      1500.000 Max. Hydrograph"
  32
             STORM Chicago storm"
**
             1 Chicago storm"
FF
       766.038 Coefficient A"
ŦŦ
         1.898 Constant B"
         0.668 Exponent C"
"
         0.400 Fraction R"
11
       180.000 Duration"
**
         1.000 Time step multiplier"
             Maximum intensity
                                        141.545 mm/hr"
77
             Total depth
                                         71.090 mm"
**
                005hyd Hydrograph extension used in this file"
FF
  33
             CATCHMENT 201"
.,
             2 Rectangular"
7 7
            1 Equal length"
**
            2 Horton equation"
77
          201 No description"
7 F
      100.000 % Impervious"
"
       0.320 Total Area"
**
       80.000 Flow length"
77
        1.200 Overland Slope"
**
        0.000 Pervious Area"
**
       80.000 Pervious length"
77
        1.200 Pervious slope"
        0.320 Impervious Area"
11
       80.000 Impervious length"
11
       1.200 Impervious slope"
**
        0.250 Pervious Manning 'n'"
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
11
        0.500 Pervious Lag constant (hours)"
        7.500 Pervious Depression storage"
**
        0.015 Impervious Manning 'n'"
**
        0.000 Impervious Max.infiltration"
**
        0.000
                Impervious Min.infiltration"
**
        0.500
                Impervious Lag constant (hours)"
"
        2.000
                Impervious Depression storage"
77
                     0.126 0.000 0.000
                                                 0.000 c.m/sec"
             Catchment 201
                                   Pervious Impervious Total Area "
             Surface Area
                                   0.000
                                               0.320 0.320 hectare"
11
             Time of concentration 22.444
                                              4.053
                                                         4.053
                                                                   minutes"
FF
             Time to Centroid 109.162
                                               89.168
                                                        89.168
                                                                   minutes"
                                  71.090 71.090 71.090

0.00 227.49 227.49

30.909 2.000 2.000

40.180 69.090 69.090

221.09 221.09
11
             Rainfall depth
             Rainfall volume
                                                                   c.m"
             Rainfall losses
11
             Runoff depth
                                                                    mm"
77
             Runoff volume
                                  0.00
                                              221.09
                                                         221.09
                                                                    c.m"
             Runoff coefficient 0.000 0.972
                                                         0.972
```

```
71
             Maximum flow
                                  0.000 0.126 0.126
                                                              c.m/sec"
FF
            HYDROGRAPH Add Runoff "
  40
**
            4 Add Runoff "
**
                    0.126
                            0.126 0.000
                                                0.000"
            PIPE DESIGN"
  51
**
        0.126 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
71
        1.000 Diameter
                         metre"
        1.000 Gradient %"
             Depth of flow
                                        0.156 metre"
**
             Velocity
                                        1.614 m/sec"
**
             Pipe capacity
                                        2.398 c.m/sec"
**
                                        0.196
             Critical depth
                                                metre"
             ROUTE Zero Route"
  53
"
         0.00 Zero Route Reach length (metre)"
77
                   HYDROGRAPH Combine 201"
  40
            6 Combine "
FF
               Node #"
          201
11
             Maximum flow
                                               c.m/sec"
                                        0.126
FF
             Hydrograph volume
                                      221.087
                                               c.m"
"
                    0.126 0.126
                                                0.126"
                                      0.126
            HYDROGRAPH Start - New Tributary"
  40
            2 Start - New Tributary"
**
                    0.126
                           0.000 0.126
                                             0.126"
            CATCHMENT 202"
**
            2 Rectangular"
            1
               Equal length"
**
            2
               Horton equation"
11
          202 No description"
**
      100.000 % Impervious"
ff
        0.333 Total Area"
11
       83.250 Flow length"
77
        1.200 Overland Slope"
77
        0.000 Pervious Area"
FF
       83.250 Pervious length"
**
       1.200 Pervious slope"
11
        0.333 Impervious Area"
       83.250 Impervious length"
FF
        1.200 Impervious slope"
**
        0.250 Pervious Manning 'n'"
11
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
11
        7.500
               Pervious Depression storage"
**
        0.015
               Impervious Manning 'n'"
77
        0.000
               Impervious Max.infiltration"
**
        0.000
               Impervious Min.infiltration"
**
        0.500
               Impervious Lag constant (hours)"
Ħ
        2.000
               Impervious Depression storage"
                    0.131 0.000
                                     0.126
                                               0.126 c.m/sec"
"
            Catchment 202
                                 Pervious Impervious Total Area "
11
                                            0.333
             Surface Area
                                0.000
                                                   0.333 hectare"
**
            Time of concentration 22.987
                                            4.151
                                                     4.151
                                                              minutes"
                                                     89.219 minutes"
11
            Time to Centroid 109.671 89.219
**
            Rainfall depth
                                                     71.090
                                 71.090 71.090
**
            Rainfall volume
                                0.00
                                            236.73
                                                     236.73
                                                               c.m"
                               30.909
FF
            Rainfall losses
                                            2.000
                                                     2.000
                                                                mm"
**
            Runoff depth
                                            69.090
                                                     69.090
                                                                mm"
```

```
**
              Runoff volume
                                    0.00
                                               230.07
                                                         230.07
                                                                     c.m"
              Runoff coefficient
77
                                   0.000
                                              0.972
                                                         0.972
**
              Maximum flow
                                    0.000
                                               0.131
                                                       0.131
                                                                     c.m/sec"
  40
             HYDROGRAPH Add Runoff "
**
             4 Add Runoff "
11
                0.131 0.131 0.126 0.126"
  51
            PIPE DESIGN"
         0.131 Current peak flow c.m/sec"
11
         0.013 Manning 'n'"
         1.000 Diameter metre"
         1.000 Gradient %"
              Depth of flow
                                           0.159
                                                   metre"
**
              Velocity
                                           1.633 m/sec"
**
              Pipe capacity
                                          2.398 c.m/sec"
77
                                           0.200 metre"
              Critical depth
  53
              ROUTE Zero Route"
**
          0.00 Zero Route Reach length (metre)"
**
                 0.131 0.131 0.131 0.126 c.m/sec"
  40
             HYDROGRAPH Combine 202"
             6 Combine "
11
           202
                Node #"
**
"
              Maximum flow
                                          0.131 c.m/sec"
             Hydrograph volume
**
              Hydrograph volume 230.069 0.131 0.131 0.131
                                                   c.m"
11
                                                 0.131"
             HYDROGRAPH Start - New Tributary"
             2 Start - New Tributary"
FF
                    0.131 0.000 0.131 0.131"
  33
             CATCHMENT 206"
**
             2 Rectangular"
77
             1 Equal length"
**
             2 Horton equation"
.,
           206 No description"
11
        74.500 % Impervious"
7 7
        0.174 Total Area"
**
        19.121 Flow length"
11
        1.200 Overland Slope"
       0.044 Pervious Area"
19.121 Pervious length"
**
77
**
        1.200 Pervious slope"
11
        0.130 Impervious Area"
11
        19.121 Impervious length"
**
        1.200 Impervious slope"
11
        0.250 Pervious Manning 'n'"
**
        25.000 Pervious Max.infiltration"
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
77
        7.500 Pervious Depression storage"
ŦŦ
        0.015 Impervious Manning 'n'"
        0.000 Impervious Max.infiltration"
11
        0.000 Impervious Min.infiltration"
77
        0.500
                Impervious Lag constant (hours)"
**
        2.000
                Impervious Depression storage"
                     0.068 0.000 0.131 0.131 c.m/sec"
                            Pervious Impervious Total Area "
**
             Catchment 206
                                            0.130 0.174 hectare"
**
                                  0.044
             Surface Area
             Time of concentration 9.510
Time to Centroid 97.845
Rainfall depth 71.090
Rainfall volume 31.54
                                              1.717 3.011 minutes"
88.885 90.372 minutes"
71.090 71.090 mm"
92.15 123.70 c.m"
**
"
11
```

```
Rainfall losses
Runoff depth
Runoff volume
**
                                   30.909
40.180
17.83
                                   30.909
                                              2.000
                                                         9.372
                                                                   mm"
77
                                             69.090
                                                        61.718
                                                                   mm"
77
                                              89.56
                                                        107.39
                                                                   c.m"
**
             Runoff coefficient
                                              0.972
                                  0.565
                                                        0.868
7.7
             Maximum flow
                                   0.017
                                              0.051
                                                        0.068
                                                                   c.m/sec"
  40
             HYDROGRAPH Add Runoff "
**
            4 Add Runoff "
**
                   0.068
                             0.068
                                        0.131
                                                0.131"
  51
             PIPE DESIGN"
        0.068 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
**
        1.000 Diameter metre"
        1.000 Gradient %"
77
             Depth of flow
                                         0.116 metre"
77
             Velocity
                                         1.342 m/sec"
**
             Pipe capacity
                                         2.398 c.m/sec"
             Critical depth
                                         0.143
                                                   metre"
77
  53
             ROUTE Zero Route"
         0.00 Zero Route Reach length (metre)"
**
                   40
             HYDROGRAPH Combine 206"
11
                Combine "
            6
**
          206
                Node #"
**
             Maximum flow
                                          0.068
                                                 c.m/sec"
"
             Hydrograph volume
                                        107.389
                                                 c.m"
77
                     0.068 0.068
                                        0.068
                                                  0.068"
  40
             HYDROGRAPH Start - New Tributary"
FF
                Start - New Tributary"
"
                     0.068
                              0.000
                                        0.068
                                                0.068"
  33
             CATCHMENT 204"
11
            2 Rectangular"
**
            1
                Equal length"
**
            2 Horton equation"
11
          204 No description"
FF
       65.000 % Impervious"
FF
                Total Area"
        0.214
**
       24.318 Flow length"
"
        1.200 Overland Slope"
        0.075 Pervious Area"
FF
       24.318 Pervious length"
• •
        1.200 Pervious slope"
       0.139 Impervious Area"
24.318 Impervious length"
**
11
11
       1.200 Impervious slope"
77
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
77
        0.500 Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
,,
        0.015
                Impervious Manning 'n'"
**
        0.000
                Impervious Max.infiltration"
**
        0.000
                Impervious Min.infiltration"
"
        0.500
                Impervious Lag constant (hours)"
11
        2.000
                Impervious Depression storage"
**
                     0.081 0.000 0.068 0.068 c.m/sec"
77
             Catchment 204
                                  Pervious Impervious Total Area "
             Surface Area
                                  0.075
**
                                              0.139 0.214 hectare"
             Time of concentration 10.985
                                              1.984
                                                        4.130
                                                                  minutes"
**
             Time to Centroid 99.160
                                              88.885
                                                       91.335
                                                                 minutes"
```

```
п
                                   71.090
             Rainfall depth
                                              71.090
                                                         71.090
                                                                   mm"
**
             Rainfall volume
                                    53.25
                                              98.89
                                                         152.13
                                                                   c.m"
Ħ
             Rainfall losses
                                    30.909
                                              2.000
                                                                   mm"
                                                         12.118
             Runoff depth
                                   40.180
                                              69.090
                                                        58.971
                                                                   mm"
             Runoff volume
                                    30.09
                                              96.10
                                                         126.20
                                                                   C.m"
**
             Runoff coefficient
                                  0.565
                                              0.972
                                                         0.830
**
             Maximum flow
                                    0.026
                                              0.055
                                                         0.081
                                                                  c.m/sec"
**
  40
             HYDROGRAPH Add Runoff "
**
            4 Add Runoff "
"
                    0.081
                             0.081
                                        0.068
                                                0.068"
,,,
  51
            PIPE DESIGN"
11
        0.081 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
#
        1.000 Diameter metre"
"
        1.000 Gradient
"
             Depth of flow
                                          0.125
                                                   metre"
11
             Velocity
                                          1.413 m/sec"
"
             Pipe capacity
                                          2.398 c.m/sec"
**
             Critical depth
                                          0.156
                                                   metre"
  53
             ROUTE Zero Route"
m
                Zero Route Reach length ( metre)"
         0.00
                   0.081 0.081 0.081 0.068 c.m/sec"
11
  40
             HYDROGRAPH Combine 204"
"
            6 Combine "
"
                Node #"
          204
11
11
             Maximum flow
                                          0.081
                                                  c.m/sec"
             Hydrograph volume
                                        126.199
                                                  c.m"
"
                     0.081 0.081
                                                  0.081"
                                        0.081
Ħ
  40
             HYDROGRAPH Start - New Tributary"
Ħ
            2 Start - New Tributary"
11
                    0.081
                              0.000
                                        0.081
                                                  0.081"
11
  33
             CATCHMENT 210"
11
            2 Rectangular"
*
            1
                Equal length"
11
            2 Horton equation"
11
          210 No description"
11
        0.000 % Impervious"
11
        0.113 Total Area"
11
       12.151 Flow length"
11
        1.200 Overland Slope"
**
        0.113 Pervious Area"
       12.151 Pervious length"
1.200 Pervious slope"
"
"
**
        0.000 Impervious Area"
**
       12.151 Impervious length"
        1.200 Impervious slope"
H.
        0.250 Pervious Manning 'n'"
       25.000 Pervious Max.infiltration"
11
        5.000 Pervious Min.infiltration"
        0.500 Pervious Lag constant (hours)"
"
        7.500 Pervious Depression storage"
11
        0.015
                Impervious Manning 'n'"
11
        0.000
                Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
"
        0.500
                Impervious Lag constant (hours)"
        2.000
                Impervious Depression storage"
11
                     0.043 0.000 0.081
                                                  0.081 c.m/sec"
11
             Catchment 210
                                  Pervious
                                              Impervious Total Area "
11
             Surface Area
                                   0.113
                                              0.000 0.113 hectare"
```

```
Time of concentration 7.245 1.308 7.245 minutes"
Time to Centroid 96.555 88.885 96.555 minutes"
Rainfall depth 71.090 71.090 71.090 mm"
Rainfall volume 80.33 0.00 80.33 c.m"
Rainfall losses 30.909 2.000 30.909 mm"
Runoff depth 40.180 69.090 40.180 mm"
Runoff volume 45.40 0.00 45.40 c.m"
Runoff coefficient 0.565 0.000 0.565 "
Maximum flow 0.043 0.000 0.043 c.m/sec"
**
**
11
**
**
**
                    HYDROGRAPH Add Runoff "
                   4 Add Runoff"
**
ŦŦ
                          0.043 0.043 0.081 0.081"
               PIPE DESIGN"
    51
              0.043 Current peak flow c.m/sec"
7.7
              0.013 Manning 'n'"
77
              1.000 Diameter metre"
**
              1.000 Gradient %"
                   Depth of flow
Velocity
Pipe capacity
Critical depth
                                                                     0.093 metre"
**
                                                                    1.171 m/sec"
**
                                                                    2.398 c.m/sec"
0.114 metre"
"
                     ROUTE Zero Route"
**
                0.00 Zero Route Reach length (metre)"
77
                            40
                     HYDROGRAPH Combine 9999"
                 6 Combine "
**
* *
                9999 Node #"
77
                          77

      Maximum flow
      0.043
      c.m/sec"

      Hydrograph volume
      45.404
      c.m"

      0.043
      0.043
      0.043"

      HYDROGRAPH Confluence
      204"

**
,,
77
                 7 Confluence "
77
77
               204 Node #"
**

      Maximum flow
      0.081
      c.m/sec"

      Hydrograph volume
      126.199
      c.m"

      0.043
      0.081
      0.043
      0.000"

11
11
              POND DESIGN"
   54
11
              0.081 Current peak flow c.m/sec"
**
              0.080 Target outflow c.m/sec"
**
              126.2 Hydrograph volume c.m"
           3. Number of stages"
223.850 Minimum water level metre"
224.150 Maximum water level metre"
11
ŧŧ
**
"
           223.850 Starting water level metre"
77
                    0 Keep Design Data: 1 = True; 0 = False"
                          Level Discharge Volume"

223.850 1.00E-07 2.490"

224.000 0.03300 13.040"

224.150 0.03430 44.580"
**
"
**
                    Peak outflow 0.034 c.m/sec"
Maximum level 224.085 metre"
Maximum storage 30.852 c.m"
Centroidal lag 1.676 hours"
**
11
                   0.043 0.081 0.034 0.000 c.m/sec"

HYDROGRAPH Next link "
                  5 Next link "
                          0.043 0.034 0.034 0.000"
" 51
                  PIPE DESIGN"
```

```
**
        0.034 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
17
        0.250 Diameter metre"
**
        1.150 Gradient %"
             Depth of flow
                                        0.129 metre"
77
             Velocity
                                        1.317 m/sec"
**
             Pipe capacity
                                       0.064 c.m/sec"
FF
             Critical depth
                                        0.149 metre"
  53
             ROUTE Pipe Route 24"
11
        23.60 Pipe Route 24 Reach length (metre)"
**
        0.358 X-factor <= 0.5"
**
       13.435 K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
11
        0.500 X-factor <= 0.5"
**
       30.000 K-lag (seconds)"
**
        0.500 Beta weighting factor"
       17.143 Routing time step (seconds)"
11
           1 No. of sub-reaches"
**
            Peak outflow
                                       0.034 c.m/sec"
**
                    0.043 0.034
                                      0.034 0.000 c.m/sec"
            HYDROGRAPH Combine 5"
  40
77
            6 Combine "
FF
            5 Node #"
"
11
                                       0.034 c.m/sec"
            Maximum flow
            Hydrograph volume 125.475 c.m" 0.043 0.034 0.034 0.034" HYDROGRAPH Confluence 206"
FF
11
  40
ŦŦ
            7 Confluence "
"
         206 Node #"
77
77
            Maximum flow
                                        0.068 c.m/sec"
             Hydrograph volume
**
                                     107.389
                                                c.m"
                   0.043 0.068 0.034 0.000"
77
"
             POND DESIGN"
  54
        0.068 Current peak flow c.m/sec"
77
        0.080 Target outflow c.m/sec"
**
        107.4 Hydrograph volume c.m"
**
          3. Number of stages"
**
      223.850 Minimum water level metre"
**
      224.150 Maximum water level metre"
11
      223.850 Starting water level metre"
11
            0
               Keep Design Data: 1 = True; 0 = False"
FF
                 Level Discharge Volume"
**
               223.850 0.01000 0.4900"
"
               224.000 0.01880 11.740"
224.150 0.01980 45.490"
77
             Peak outflow
                                       0.020 c.m/sec"
            Maximum level
                                      224.107 metre"
            Centroidal lag
**
            Maximum storage
                                                c.m"
                                       1.804 hours"
                 **
            HYDROGRAPH Next link "
 40
"
            5 Next link "
77
                    0.043 0.020 0.020
                                              0.000"
77
 51
           PIPE DESIGN"
**
        0.020 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
        0.250 Diameter metre"
**
        2.000
             Gradient
```

```
11
                                                  0.082 metre"
1.395 m/sec"
                Depth of flow
               Velocity
Pipe capacity
Critical depth
77
**
                                                  0.084 c.m/sec"
**
                                                   0.112 metre"
           ROUTE Pipe Route 24"
         23.60 Pipe Route 24 Reach length ( metre)"
**
         0.453 X-factor <= 0.5"
**
         12.690 K-lag (seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
          0.500 X-factor <= 0.5"
         30.000 K-lag ( seconds)"
**
         0.500 Beta weighting factor"
         13.636 Routing time step ( seconds)"
11
             1 No. of sub-reaches"
               Peak outflow 0.020 c.m/sec 0.043 0.020 0.020 0.000 c.m/sec"
**
**
               HYDROGRAPH Combine 5"
               6 Combine "
**
               5 Node #"
**
              Hydrograph volume 232.497 c.m"

0.043 0.020 0.020 0.053"

HYDROGRAPH Confluence 5"

7 Confluence "
Ŧ Ŧ
**
**
ŦŦ
               5 Node #"
**

      Maximum flow
      0.053
      c.m/sec"

      Hydrograph volume
      232.497
      c.m"

      0.043
      0.053
      0.020
      0.000"

**
              Maximum flow
Hydrograph volume
11
* *
            PIPE DESIGN"
**
**
          0.053 Current peak flow c.m/sec"
**
          0.013 Manning 'n'"
11
          0.375 Diameter metre"
11
          0.400 Gradient %"
FF
             Depth of flow
                                                   0.183 metre"
11
                                                  0.994 m/sec"
               Velocity
              Pipe capacity
Critical depth
**
                                                  0.111 c.m/sec"
                                                  0.167 metre"
**
           ROUTE Pipe Route 78"
77
         77.50 Pipe Route 78 Reach length (metre)"
77
         0.328 X-factor <= 0.5"
         58.489 K-lag (seconds)"
11
         0.000 Default(0) or user spec.(1) values used"
77
11
         0.500 X-factor <= 0.5"
FF
         30.000 K-lag (seconds)"
**
         0.500 Beta weighting factor"
"
         75.000 Routing time step ( seconds)"
             1 No. of sub-reaches"
77
**
              Peak outflow
                                                  0.053 c.m/sec"
                0.043 0.053 0.000 c.m/sec"
**
              HYDROGRAPH Combine 3"
**
              6 Combine "
**
             3 Node #"
11

      Maximum flow
      0.053
      c.m/se

      Hydrograph volume
      232.497
      c.m"

      0.043
      0.053
      0.053
      0.053"

      HYDROGRAPH
      Confluence
      202"

               Maximum flow
                                                  0.053 c.m/sec"
"
77
FF
               7 Confluence "
```

```
**
          202 Node #"
"
77
             Maximum flow
                                            0.131 c.m/sec"

      Maximum flow
      0.131 c.m/s

      Hydrograph volume
      230.069 c.m"

      0.043 0.131 0.053 0.000"

**
           POND DESIGN"
11
         0.131 Current peak flow c.m/sec"
**
         0.080 Target outflow c.m/sec"
**
         230.1 Hydrograph volume c.m"
           3. Number of stages"
11
       223.000 Minimum water level
                                        metre"
77
       223.150 Maximum water level metre"
       223.000 Starting water level metre"
**
             0
                 Keep Design Data: 1 = True; 0 = False"
**
                  Level Discharge Volume"
ŦŦ
                 223.000 0.00100 0.4900"
                 223.075 0.08990 10.370"
223.150 0.08500 39.990"
**
**
"
              Peak outflow
                                            0.089 c.m/sec"
11
              Maximum level
                                          223.083 metre"
              Maximum storage 223.083
Centroidal lag 1520
ŦŦ
                                                    c.m"
**
                                            1.529 hours"
                   0.043 0.131 0.089 0.000 c.m/sec"
11
11
             HYDROGRAPH Next link "
             5 Next link "
**
                  0.043 0.089 0.089 0.000"
11
           PIPE DESIGN"
  51
77
         0.089 Current peak flow c.m/sec"
11
         0.013
               Manning 'n'"
FF
         0.375 Diameter metre"
**
         0.400 Gradient %"
                                            0.255 metre"
1.117 m/sec"
**
              Depth of flow
**
              Velocity
11
              Pipe capacity
                                            0.111 c.m/sec"
77
              Critical depth
                                            0.219 metre"
ŦŦ
  53
             ROUTE Pipe Route 24"
**
         23.60 Pipe Route 24 Reach length ( metre)"
77
         0.000 X-factor <= 0.5"
**
        15.847 K-lag (seconds)"
77
         0.000 Default(0) or user spec.(1) values used"
77
         0.500 X-factor <= 0.5"
**
        30.000 K-lag (seconds)"
**
        0.675 Beta weighting factor"
        46.154 Routing time step ( seconds)"
FF
             1 No. of sub-reaches"
77
              Peak outflow
                                            0.089 c.m/sec"
**
                                          0.089 0.000 c.m/sec"
                 0.043 0.089
**
             HYDROGRAPH Combine 3"
**
             6 Combine "
**
             3 Node #"
11
                 11
11
              Maximum flow
                                            0.142 c.m/sec"
              Hydrograph volume
            Hydrograph volume 468.701 c.m"
0.043 0.089 0.089 0.142"
HYDROGRAPH Confluence 3"
11
11
**
  40
ŦŦ
             7 Confluence "
**
             3 Node #"
**
,,
              Maximum flow
                                            0.142
              Maximum flow 0.142
Hydrograph volume 468.701
                                                     c.m/sec"
**
                                                      c.m"
```

```
11
                      0.043 0.142 0.089 0.000"
11
         PIPE DESIGN"
  51
77
         0.142 Current peak flow c.m/sec"
 * *
         0.013 Manning 'n'"
77
         0.450 Diameter metre"
Ŧ Ŧ
         0.400 Gradient %"
**
             Depth of flow
                                         0.302 metre"
              Velocity
                                          1.257 m/sec"
**
              Pipe capacity
                                         0.180 c.m/sec"
**
             Critical depth
                                          0.264 metre"
11
  53
             ROUTE Pipe Route 15"
         15.00 Pipe Route 15 Reach length ( metre)"
**
         0.000 X-factor <= 0.5"
.
         8.952 K-lag (seconds)"
77
         0.000 Default(0) or user spec.(1) values used"
**
         0.500 X-factor <= 0.5"
77
        30.000 K-lag ( seconds)"
71
        0.801 Beta weighting factor"
11
        40.000 Routing time step (seconds)"
77
            1 No. of sub-reaches"
"
             Peak outflow
                                         0.142 c.m/sec"
                     0.043 0.142 0.142 0.000 c.m/sec"
11
  40
             HYDROGRAPH Combine 201"
11
            6 Combine "
**
          201 Node #"
**
             Maximum flow 0.261 c.m/sec"
Hydrograph volume 689.787 c.m"
0.043 0.142 0.142 0.261"
HYDROGRAPH Confluence 201"
**
**
77
  40
"
            7 Confluence "
77
         201 Node #"
ŦŦ
11
                                       0.261 c.m/sec"
689.787 c.m"
             Maximum flow
             Hydrograph volume
ŦŦ
77
                     0.043 0.261 0.142 0.000"
**
          PIPE DESIGN"
FF
        0.261 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
**
        0.525 Diameter metre"
**
        0.400 Gradient %"
77
             Depth of flow
                                          0.413 metre"
**
             Velocity
                                         1.431 m/sec"
                                         0.272 c.m/sec"
0.346 metre"
11
             Pipe capacity
77
             Critical depth
  53
             ROUTE Pipe Route 16"
**
        15.70 Pipe Route 16 Reach length (metre)"
"
        0.000 X-factor <= 0.5"
**
        8.229 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
"
        0.500 	 X-factor <= 0.5"
       30.000 K-lag ( seconds)"
7.
        0.869 Beta weighting factor"
       54.545 Routing time step ( seconds)"
**
            1 No. of sub-reaches"
**
            Peak outflow
                                          0.259 c.m/sec"
11
                     0.043 0.261
                                       0.259
                                                0.000 c.m/sec"
            HYDROGRAPH Next link "
**
            5 Next link "
**
                     0.043 0.259 0.259 0.000"
```

```
**
  40
             HYDROGRAPH Undo"
             1 Undo"
                    0.043 0.261 0.259 0.000"
  56
             DIVERSION"
**
             0 Node number"
11
         0.174 Overflow threshold"
         1.000 Required diverted fraction"
11
             O Conduit type; 1=Pipe; 2=Channel"
             Peak of diverted flow 0.087 c.m/sec"
              Volume of diverted flow
                                        53.193
              DIV00000.005hyd"
**
             Divert to Overland Flow 0.087 cms"
**
                     0.043 0.261
                                     0.174
                                                  0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
             5 Next link "
77
                     0.043
                             0.174 0.174 0.000"
             DIVERSION"
"
             3 Node number"
7.5
              Overflow threshold"
         0.174
11
         1.000 Required diverted fraction"
77
             O Conduit type; 1=Pipe; 2=Channel"
**
             Peak of diverted flow 0.000 c.m/.
Volume of diverted flow 0.000 c.m"
                                                c.m/sec"
77
ŦŦ
             DIV00003.005hyd"
,,
             No Flow Diverted"
77
                     0.043 0.174 0.174 0.000 c.m/sec"
**
  40
             HYDROGRAPH Next link "
11
            5 Next link "
•
                     0.043
                           0.174 0.174
                                              0.000"
  54
             POND DESIGN"
77
         0.174 Current peak flow c.m/sec"
* *
         0.090 Target outflow c.m/sec"
11
         636.6 Hydrograph volume c.m"
           3. Number of stages"
11
       221.600 Minimum water level metre"
      223.100 Maximum water level metre" 221.600 Starting water level metre"
11
11
                Keep Design Data: 1 = True; 0 = False"
,,
                  Level Discharge Volume"
                221.600 0.01000 0.01000"
                222.350 0.04960 99.140"
                223.100 0.06830 262.660"
             Peak outflow
                                          0.068 c.m/sec"
             Maximum level
                                       223.097 metre"
             Maximum storage
                                       262.063
                                                 c.m"
77
             Centroidal lag
                                        2.441 hours"
                  0.043 0.174
                                    0.068 0.000 c.m/sec"
**
  40
            HYDROGRAPH Next link "
            5 Next link "
**
                    0.043 0.068 0.068
                                                0.000"
**
             PIPE DESIGN"
FF
        0.068 Current peak flow c.m/sec"
        0.013
              Manning 'n'"
**
        0.375 Diameter
                          metre"
77
        0.400 Gradient %"
             Depth of flow
                                         0.212 metre"
             Velocity
                                         1.055 m/sec"
             Pipe capacity
                                        0.111
                                                  c.m/sec"
             Critical depth
                                         0.190
                                                metre"
" 53
             ROUTE Pipe Route 59"
```

```
**
        59.40
                 Pipe Route 59 Reach length (metre)"
        0.220 X-factor <= 0.5"
       42.219 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
**
        0.500 X-factor <= 0.5"
       30.000 K-lag ( seconds)"
**
        0.500 Beta weighting factor"
**
       60.000 Routing time step (seconds)"
77
           1 No. of sub-reaches"
**
            Peak outflow
                                         0.068
                                                c.m/sec"
**
                    0.043 0.068
                                       0.068 0.000 c.m/sec"
            HYDROGRAPH Next link "
  40
.,
            5 Next link "
                    0.043
                            0.068 0.068
                                             0.000"
  51
            PIPE DESIGN"
11
        0.068 Current peak flow c.m/sec"
11
        0.013 Manning 'n'"
        0.375 Diameter metre"
FF
        0.400 Gradient %"
* *
            Depth of flow
                                        0.212
**
             Velocity
                                         1.055 m/sec"
* *
             Pipe capacity
                                        0.111
                                              c.m/sec"
**
             Critical depth
                                        0.190 metre"
11
            ROUTE Pipe Route 31"
11
        31.30 Pipe Route 31 Reach length (metre)"
,,
        0.000 X-factor <= 0.5"
ŧŧ
       22.247 K-lag (seconds)"
77
        0.000 Default(0) or user spec.(1) values used"
**
        0.500 X-factor <= 0.5"
11
       30.000 K-lag ( seconds)"
FŦ
       0.516 Beta weighting factor"
**
       42.857 Routing time step ( seconds)"
"
           1 No. of sub-reaches"
77
            Peak outflow
                                        0.068 c.m/sec"
**
                    0.043 0.068
                                      0.068 0.000 c.m/sec"
            HYDROGRAPH Next link "
            5 Next link "
7 f
                    0.043
                            0.068
                                       0.068
                                              0.000"
**
            PIPE DESIGN"
11
        0.068 Current peak flow c.m/sec"
ŦŦ
        0.013
              Manning 'n'"
**
        0.375 Diameter metre"
"
        0.400 Gradient %"
            Depth of flow
                                         0.212
                                                metre"
**
            Velocity
                                        1.055 m/sec"
* *
             Pipe capacity
                                        0.111 c.m/sec"
11
                                        0.190 metre"
            Critical depth
            ROUTE Pipe Route 11"
 53
**
        10.50 Pipe Route 11 Reach length (metre)"
        0.000 	 X-factor <= 0.5"
11
        7.463 K-lag (seconds)"
        0.000 Default(0) or user spec.(1) values used"
        0.500 	 X-factor <= 0.5"
       30.000 K-lag (seconds)"
11
       0.770 Beta weighting factor"
       30.000 Routing time step (seconds)"
           1 No. of sub-reaches"
            Peak outflow
                                         0.068 c.m/sec"
                    0.043 0.068
                                     0.068 0.000 c.m/sec"
" 40
            HYDROGRAPH Combine 9999"
```

** ** **		6 Combine " 9999 Node #"			
"		Maximum flow	0.095		
"		Hydrograph volume	681.031		
**	40	0.043 0.068	0.068	0.095"	
,,	40	HYDROGRAPH Confluence	9999"		
-		7 Confluence "			
11		9999 Node #"			
77			0.005	/	
"		Maximum flow	0.095	c.m/sec"	
11		Hydrograph volume	681.031	c.m"	
11	38	0.043 0.095		0.000"	
11	30	START/RE-START TOTALS 9999	••		
"		a remora rocerp on marr		4 454	
n		Total Catchment area		1.154	hectare"
79		Total Impervious area		0.922	hectare"
	1.0	Total % impervious		79.873"	
**	19	EXIT"			

```
**
                MIDUSS Output ---->"
77
                MIDUSS version
                                                       Version 2.25 rev. 473"
**
                MIDUSS created
                                                                February-07-10"
            10
                Units used:
                                                                    ie METRIC"
77
                Job folder:
                                               C:\swm\17233\Update 2025-3-20"
**
                Output filename:
                                                                   pst1004.out"
11
                Licensee name:
                                                                          Bob"
11
                Company
**
                 Date & Time last used:
                                                     25/07/2024 at 11:37:53 AM"
**
              TIME PARAMETERS"
77
        10.000 Time Step"
11
       180.000
               Max. Storm length"
      1500.000 Max. Hydrograph"
**
  32
             STORM Chicago storm"
11
             1 Chicago storm"
**
       801.041 Coefficient A"
        1.501 Constant B"
FT
         0.657 Exponent C"
               Fraction R"
         0.400
**
       180.000 Duration"
**
         1.000 Time step multiplier"
11
             Maximum intensity
                                                  mm/hr"
                                         155.782
"
             Total depth
                                         78.830
                                                 mm"
77
                005hyd Hydrograph extension used in this file"
**
  33
             CATCHMENT 201"
,,
            2
                Rectangular"
"
                Equal length"
            1
ŦŦ
            2
              Horton equation"
**
          201 No description"
11
       100.000 % Impervious"
77
        0.320 Total Area"
**
        80.000 Flow length"
**
        1.200 Overland Slope"
"
        0.000 Pervious Area"
**
       80.000 Pervious length"
**
        1.200 Pervious slope"
**
        0.320 Impervious Area"
"
        80.000
                Impervious length"
77
        1.200 Impervious slope"
**
        0.250 Pervious Manning 'n'"
**
       25.000 Pervious Max.infiltration"
"
        5.000
               Pervious Min.infiltration"
77
        0.500 Pervious Lag constant (hours)"
77
        7.500 Pervious Depression storage"
**
        0.015
                Impervious Manning 'n'"
**
        0.000
                Impervious Max.infiltration"
"
        0.000
                Impervious Min.infiltration"
77
        0.500
                Impervious Lag constant (hours)"
,,
        2.000
                Impervious Depression storage"
,,
                     0.138 0.000 0.000
                                                   0.000 c.m/sec"
77
             Catchment 201
                                   Pervious Impervious Total Area "
**
             Surface Area
                                   0.000
                                               0.320 0.320 hectare"
             Time of concentration 21.531
                                               3.901
                                                         3.901
                                                                   minutes"
**
             Time to Centroid 108.691
                                              89.036
                                                        89.036
11
             Rainfall depth
                                   78.830
                                              78.830
                                                         78.830
                                                                   mm"
77
             Rainfall volume
                                   0.00
                                               252.26
                                                         252.26
                                                                    c.m"
                                   2.000
47.591 76.830
0.00
ŦŦ
                                   31.239
             Rainfall losses
                                                         2.000
                                                                    mm"
**
             Runoff depth
                                                         76.830
                                                                    mm"
**
             Runoff volume 0.00 245.86
Runoff coefficient 0.000 0.975
                                                         245.86
                                                                    C.m"
11
                                                         0.975
```

```
**
             Maximum flow
                                      0.000 0.138 0.138 c.m/sec"
  40
              HYDROGRAPH Add Runoff "
**
             4 Add Runoff "
**
                      0.138
                               0.138 0.000
                                                      0.000"
11
             PIPE DESIGN"
         0.138 Current peak flow c.m/sec"
11
         0.013 Manning 'n'"
         1.000 Diameter metre"
**
         1.000 Gradient %"
              Depth of flow
                                             0.163
                                                      metre"
              Velocity
                                             1.661 m/sec"
**
              Pipe capacity
                                            2.398 c.m/sec"
                                            0.206
              Critical depth
                                                      metre"
              ROUTE Zero Route"
Ħ
          0.00 Zero Route Reach length ( metre)"
11
                     40
              HYDROGRAPH Combine 201"
             6 Combine "
11
           201
                 Node #"
**
11
                                                     c.m/sec"
              Maximum flow
                                            0.138
**
                                                     c.m"
              Hydrograph volume
                                           245.856

      aph volume
      245.856

      0.138
      0.138

      0.138
      0.138

*
                                                     0.138"
11
              HYDROGRAPH Start - New Tributary"
п
             2 Start - New Tributary"
Ħ
                               0.000
                      0.138
                                         0.138 0.138"
11
  33
              CATCHMENT 202"
"
             2 Rectangular"
**
             1
                 Equal length"
"
             2 Horton equation"
**
           202 No description"
**
       100.000 % Impervious"
11
        0.333 Total Area"
17
        83.250 Flow length"
tr
         1.200 Overland Slope"
**
        0.000 Pervious Area"
        83.250 Pervious length"
11
        1.200 Pervious slope"
**
        0.333 Impervious Area"
"
        83.250 Impervious length"
11
        1.200 Impervious slope"
11
        0.250 Pervious Manning 'n'"
"
        25.000 Pervious Max.infiltration"
**
         5.000 Pervious Min.infiltration"
11
         0.500 Pervious Lag constant (hours)"
11
         7.500 Pervious Depression storage"
**
         0.015
                 Impervious Manning 'n'"
**
         0.000
                 Impervious Max.infiltration"
**
         0.000
                 Impervious Min.infiltration"
**
         0.500
                 Impervious Lag constant (hours)"
11
                 Impervious Depression storage"
         2.000
                      0.144 0.000 0.138 0.138 c.m/sec"
11
              Catchment 202
                                     Pervious Impervious Total Area "
11
              Surface Area
                                    0.000
                                                 0.333 hectare"
11
                                                            3.995
              Time of concentration 22.052
                                                 3.995
                                                                      minutes"
                                                           3.995 minutes"
89.085 minutes"
              Time to Centroid 109.160 89.085
Rainfall depth 78.830 78.830
Rainfall volume 0.00 262.50
Rainfall losses 31.239 2.000
Runoff depth 47.591 76.830
"
                                                            78.830
**
                                                            262.50
                                                                       c.m"
**
                                                            2.000
                                                                        mm"
                                                            76.830
                                                                        mm"
```

```
FF
              Runoff volume
                                    0.00
                                              255.84
                                                        255.84
                                                                    c.m"
**
             Runoff coefficient
                                  0.000
                                             0.975
                                                        0.975
**
             Maximum flow
                                    0.000
                                              0.144
                                                         0.144
                                                                  c.m/sec"
**
  40
             HYDROGRAPH Add Runoff "
**
             4 Add Runoff "
11
                     0.144 0.144 0.138 0.138"
  51
            PIPE DESIGN"
**
         0.144 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
**
         1.000 Diameter metre"
**
         1.000 Gradient %"
**
             Depth of flow
                                          0.166
                                                   metre"
17
             Velocity
                                          1.680 m/sec"
**
                                          2.398 c.m/sec"
0.210 metre"
              Pipe capacity
             Critical depth
  53
             ROUTE Zero Route"
**
          0.00 Zero Route Reach length (metre)"
**
                 0.144 0.144 0.144 0.138 c.m/sec"
             HYDROGRAPH Combine 202"
  40
**
            6 Combine "
**
          202
                Node #"
11
**
             Maximum flow
                                          0.144 c.m/sec"
             Hydrograph volume
**
                  ograph volume 255.844
0.144 0.144 0.144
                                                  c.m"
11
                                                  0.144"
  40
             HYDROGRAPH Start - New Tributary"
,,
            2 Start - New Tributary"
"
                     0.144 0.000 0.144 0.144"
• •
  33
             CATCHMENT 206"
,,
            2 Rectangular"
**
            1 Equal length"
77
            2 Horton equation"
**
          206 No description"
,,
        74.500 % Impervious"
77
        0.174 Total Area"
ff
       19.121 Flow length"
**
        1.200 Overland Slope"
        0.044 Pervious Area"
77
       19.121 Pervious length"
**
        1.200 Pervious slope"
       0.130 Impervious Area"
19.121 Impervious length"
**
11
        1.200 Impervious slope"
77
        0.250 Pervious Manning 'n'"
..
       25.000 Pervious Max.infiltration"
11
        5.000 Pervious Min.infiltration"
11
        0.500 Pervious Lag constant (hours)"
**
        7.500 Pervious Depression storage"
**
        0.015
                Impervious Manning 'n'"
11
        0.000 Impervious Max.infiltration"
11
        0.000 Impervious Min.infiltration"
ŧŧ
        0.500
                Impervious Lag constant (hours)"
"
        2.000
                Impervious Depression storage"
**
                     0.075 0.000 0.144
                                                 0.144 c.m/sec"
77
             Catchment 206
                                   Pervious Impervious Total Area "
**
             Surface Area
                                   0.044
                                              0.130 0.174 hectare"
                                                        2.959 minutes"
90.451 minutes"
             Time to Centroid 98.009
Rainfall walker
             Time of concentration 9.123
                                              1.653
                                              88.849
78.830
102.19
"
                                             78.830
                                                        78.830
                                                                  mm"
             Rainfall volume 34.98
ŦŦ
                                                         137.16
                                                                   c.m"
```

```
Rainfall losses
                                    31.239
                                              2.000
                                                         9.456
                                                                   mm"
**
             Runoff depth
Runoff volume
                                   47.591
                                              76.830
                                                        69.374
                                                                   mm"
                                  21.12 99.59
0.604 0.975
                                                        120.71
                                                                   c.m"
* *
             Runoff coefficient
                                                         0.880
"
             Maximum flow
                                    0.019
                                              0.056
                                                        0.075
                                                                   c.m/sec"
**
             HYDROGRAPH Add Runoff "
**
             4 Add Runoff "
"
                     0.075
                             0.075
                                                0.144"
                                       0.144
  51
             PIPE DESIGN"
**
         0.075 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
"
         1.000 Diameter metre"
**
         1.000 Gradient %"
**
             Depth of flow
                                          0.122
                                                   metre"
             Velocity
                                         1.386 m/sec"
**
             Pipe capacity
                                         2.398 c.m/sec"
"
             Critical depth
                                         0.151
                                                   metre"
"
  53
             ROUTE Zero Route"
         0.00 Zero Route Reach length (metre)"
FŦ
                0.075 0.075 0.075 0.144 c.m/sec"
ŦŦ
  40
             HYDROGRAPH Combine 206"
**
            6 Combine "
"
          206
                Node #"
11
17
             Maximum flow
                                          0.075
                                                 c.m/sec"
**
             Hydrograph volume
                                        120.711
                                                  C.m"
77
                    0.075 0.075 0.075
                                                  0.075"
  40
             HYDROGRAPH Start - New Tributary"
**
            2 Start - New Tributary"
"
                                      0.075 0.075"
                     0.075
                             0.000
  33
             CATCHMENT 204"
71
            2 Rectangular"
**
            1 Equal length"
**
            2 Horton equation"
ŦŦ
          204 No description"
**
       65.000 % Impervious"
**
        0.214 Total Area"
       24.318 Flow length"
11
        1.200 Overland Slope"
**
        0.075 Pervious Area"
"
       24.318 Pervious length"
        1.200 Pervious slope"
TT
        0.139 Impervious Area"
**
       24.318 Impervious length"
       1.200 Impervious slope"
0.250 Pervious Manning 'n'"
**
**
11
       25.000 Pervious Max.infiltration"
**
        5.000 Pervious Min.infiltration"
**
        0.500 Pervious Lag constant (hours)"
"
        7.500 Pervious Depression storage"
77
        0.015 Impervious Manning 'n'"
11
        0.000 Impervious Max.infiltration"
        0.000
                Impervious Min.infiltration"
**
        0.500
                Impervious Lag constant (hours)"
**
                Impervious Depression storage"
        2.000
11
                     0.091 0.000 0.075 0.075 c.m/sec"
             Catchment 204
Surface Area
                               Pervious Impervious Total Area "
**
                                  0.075
                                              0.139 0.214 hectare"
77
             Time of concentration 10.538
                                              1.909
                                                        4.068
                                                                 minutes"
77
             Time to Centroid 99.125
                                                        91.419
                                              88.849
                                                                 minutes"
```

```
Rainfall depth
                                    78.830
                                               78.830
                                                          78.830
                                                                     mm"
              Rainfall volume
                                    59.04
                                               109.65
                                                          168.70
                                                                     c.m"
                                    31.239
47.591
**
             Rainfall losses
                                                          12.234
                                               2.000
                                                                     mm"
             Runoff depth
                                               76.830
                                                         66.597
                                                                     mm"
**
             Runoff volume
                                    35.65
                                               106.87
                                                         142.52
                                                                     c.m"
FF
             Runoff coefficient
                                    0.604
                                               0.975
                                                          0.845
**
             Maximum flow
                                    0.031
                                               0.060
                                                          0.091
                                                                     c.m/sec"
             HYDROGRAPH Add Runoff "
  40
17
             4 Add Runoff "
Ŧ 7
                                                 0.075"
                   0.091
                              0.091
                                        0.075
**
  51
            PIPE DESIGN"
         0.091 Current peak flow c.m/sec"
77
         0.013 Manning 'n'"
         1.000 Diameter metre"
**
         1.000 Gradient %"
             Depth of flow
                                           0.133
                                                    metre"
77
                                           1.466
             Velocity
                                                    m/sec"
             Pipe capacity
                                           2.398
                                                    c.m/sec"
**
             Critical depth
                                          0.166
                                                    metre"
**
  53
             ROUTE Zero Route"
* #
          0.00
                Zero Route Reach length ( metre)"
                     0.091 0.091 0.091 0.075 c.m/sec"
  40
             HYDROGRAPH Combine 204"
**
                Combine "
**
           204
                Node #"
11
**
             Maximum flow
                                           0.091
                                                   c.m/sec"
11
             Hydrograph volume
                                         142.517
                                                   c.m"
**
                     0.091 0.091
                                         0.091
                                                   0.091"
  40
             HYDROGRAPH Start - New Tributary"
"
             2 Start - New Tributary"
77
                     0.091
                               0.000
                                         0.091
                                                 0.091"
  33
             CATCHMENT 210"
**
            2 Rectangular"
"
            1 Equal length"
71
            2 Horton equation"
          210 No description"
11
        0.000 % Impervious"
ŦŦ
        0.113 Total Area"
       12.151 Flow length"
1.200 Overland Slope"
ŦŦ
17
11
        0.113 Pervious Area"
**
       12.151 Pervious length"
        1.200 Pervious slope"
0.000 Impervious Area"
11
11
ff
       12.151 Impervious length"
**
                Impervious slope"
        1.200
rr
        0.250
                Pervious Manning 'n'"
11
       25.000 Pervious Max.infiltration"
77
        5.000
                Pervious Min.infiltration"
11
                Pervious Lag constant (hours)"
        0.500
                Pervious Depression storage"
        7.500
FF
        0.015
                Impervious Manning 'n'"
**
        0.000
                Impervious Max.infiltration"
FF
        0.000
                Impervious Min.infiltration"
        0.500
                Impervious Lag constant (hours)"
77
        2.000
                Impervious Depression storage"
77
                     0.049 0.000 0.091
                                                   0.091 c.m/sec"
**
             Catchment 210
                                   Pervious
                                               Impervious Total Area "
             Surface Area
                                    0.113
                                               0.000 0.113 hectare"
```

17

```
Time of concentration 6.950 1.259 6.950 minutes"
Time to Centroid 96.777 88.849 96.777 minutes"
Rainfall depth 78.830 78.830 78.830 mm"
Rainfall volume 89.08 0.00 89.08 c.m"
Rainfall losses 31.239 2.000 31.239 mm"
Runoff depth 47.591 76.830 47.591 mm"
Runoff volume 53.78 0.00 53.78 c.m"
Runoff coefficient 0.604 0.000 0.604 "
Maximum flow 0.049 0.000 0.049 c.m/sec"
HYDROGRAPH Add Runoff "
**
n
Ħ
**
11
**
**
                    HYDROGRAPH Add Runoff "
**
                   4 Add Runoff "
**
                          0.049 0.049 0.091 0.091"
               PIPE DESIGN"
   51
tt
              0.049 Current peak flow c.m/sec"
**
              0.013 Manning 'n'"
**
              1.000 Diameter metre"
              1.000 Gradient %"
                  Depth of flow
Velocity
Pipe capacity
Critical depth
"
                                                                      0.099 metre"
**
                                                                     1.214 m/sec"
**
                                                                     2.398 c.m/sec"
0.121 metre"
                     ROUTE Zero Route"
   53
u
                0.00 Zero Route Reach length (metre)"
11
                            0.049 0.049 0.049 0.091 c.m/sec"
                    HYDROGRAPH Combine 9999"
   40
                 6 Combine "
11
**
                9999 Node #"
11
                          **

      Maximum flow
      0.049
      c.m/se

      Hydrograph volume
      53.778
      c.m"

      0.049
      0.049
      0.049"

      HYDROGRAPH
      Confluence
      204"

"
                                                                      0.049 c.m/sec"
**
11
"
   40
                 7 Confluence "
m
               204 Node #"
11
11

      Maximum flow
      0.091
      c.m/sec"

      Hydrograph volume
      142.517
      c.m"

      0.049
      0.091
      0.049
      0.000"

"
**
"
              POND DESIGN"
11
11
              0.091 Current peak flow c.m/sec"
m
              0.080 Target outflow c.m/sec"
Ħ
              142.5 Hydrograph volume c.m"
           3. Number of stages"
223.850 Minimum water level metre"
224.150 Maximum water level metre"
**
"
**
           223.850 Starting water level metre"
**
                     0 Keep Design Data: 1 = True; 0 = False"
                          Level Discharge Volume"
223.850 1.00E-07 2.490"
224.000 0.03300 13.040"
224.150 0.03430 44.580"
"
"
11
**
                    Peak outflow 0.034 c.m/sec"
Maximum level 224.117 metre"
Maximum storage 37.601 c.m"
Centroidal lag 1.698 hours"
11
11
**
"
                   0.049 0.091 0.034 0.000 c.m/sec"
HYDROGRAPH Next link "
"
11
                     5 Next link "
                          0.049 0.034 0.034 0.000"
" 51
                   PIPE DESIGN"
```

```
77
         0.034 Current peak flow c.m/sec"
         0.013 Manning 'n'"
**
         0.250 Diameter metre"
         1.150 Gradient %"
**
             Depth of flow
                                            0.130 metre"
1.320 m/sec"
              Velocity
77
              Pipe capacity
                                            0.064 c.m/sec"
7.7
              Critical depth
                                             0.150 metre"
  53
              ROUTE Pipe Route 24"
         23.60 Pipe Route 24 Reach length ( metre)"
77
         0.357 X-factor <= 0.5"
**
        13.408 K-lag (seconds)"
**
         0.000 Default(0) or user spec.(1) values used"
         0.500 X-factor <= 0.5"
**
        30.000 K-lag ( seconds)"
11
         0.500 Beta weighting factor"
77
        17.143 Routing time step (seconds)"
            1 No. of sub-reaches"
**
             Peak outflow
                                            0.034 c.m/sec"
                     0.049 0.034 0.000 c.m/sec"
11
              HYDROGRAPH Combine 5"
  40
**
             6 Combine "
**
             5 Node #"
11
                                            0.034 c.m/sec"
              Maximum flow

      Maximum flow
      0.034 c.m/s

      Hydrograph volume
      141.691 c.m"

      0.049 0.034 0.034
      0.034 0.034"

      HYDROGRAPH Confluence
      206"

**
77
            7 Confluence "
**
          206 Node #"
**
11
              Maximum flow
                                            0.075 c.m/sec"
              Hydrograph volume
**
                                         120.711
                                                     c.m"
                      0.049 0.075 0.034 0.000"
**
**
          POND DESIGN"
77
         0.075 Current peak flow c.m/sec"
77
         0.080 Target outflow c.m/sec"
* *
         120.7 Hydrograph volume c.m"
**
           3. Number of stages"
       223.850 Minimum water level metre"
11
**
       224.150 Maximum water level metre"
**
       223.850 Starting water level metre"
**
             0 Keep Design Data: 1 = True; 0 = False"
11
                  Level Discharge Volume"
11
                 223.850 0.01000 0.4900"
                 224.000 0.01880 11.740"
224.150 0.01980 45.490"
ak outflow 0
**
11
              Peak outflow
                                            0.020 c.m/sec"
ff
              Maximum level
                                          224.134 metre"
              Maximum storage 41.972 c.m"
Centroidal lag 1.845 hours"
**
**
                   0.049 0.075 0.020 0.000 c.m/sec"
             HYDROGRAPH Next link "
  40
77
             5 Next link "
ŦŦ
                      0.049 0.020 0.020
                                                    0.000"
**
  51
           PIPE DESIGN"
"
         0.020 Current peak flow c.m/sec"
11
         0.013 Manning 'n'"
11
         0.250 Diameter metre"
         2.000
               Gradient %"
```

```
**
                                                   0.082 metre"
                Depth of flow
                                                   1.398 m/sec"
0.084 c.m/sec"
0.113 metre"
11
                Velocity
Pipe capacity
Critical depth
                Velocity
11
           ROUTE Pipe Route 24"
**
         23.60 Pipe Route 24 Reach length ( metre)"
         0.452 X-factor <= 0.5"
11
         12.657 K-lag (seconds)"
11
         0.000 Default(0) or user spec.(1) values used"
ff.
         0.500 X-factor <= 0.5"
         30.000 K-lag ( seconds)"
         0.500 Beta weighting factor"
         13.636 Routing time step ( seconds)"
               1 No. of sub-reaches"
               Peak outflow 0.020 c.m/sec" 0.049 0.020 0.020 0.000 c.m/sec"
11
ti
               HYDROGRAPH Combine 5"
               6 Combine "
**
               5 Node #"
11
              Hydrograph volume 262.019 c.m"
0.049 0.020 0.020 0.054"

HYDROGRAPH Confluence 5"
7 Confluence "
                    77
11
**
  40
              7 Confluence "
               5 Node #"
**
               Maximum flow
Hydrograph volume

      Maximum flow
      0.054
      c.m/s

      Hydrograph volume
      262.019
      c.m"

      0.049
      0.054
      0.020
      0.000"

                                                  0.054 c.m/sec"
**
          PIPE DESIGN"
  51
**
         0.054 Current peak flow c.m/sec"
**
         0.013 Manning 'n'"
11
          0.375 Diameter metre"
**
         0.400 Gradient %"
              Depth of flow
                                                   0.184 metre"
0.996 m/sec"
0.111 c.m/sec"
rr.
m
               Velocity
              Pipe capacity
Critical depth
m
**
                                                   0.168 metre"
          ROUTE Pipe Route 78"
         77.50 Pipe Route 78 Reach length (metre)"
**
          0.327 X-factor <= 0.5"
11
         58.364 K-lag (seconds)"
11
         0.000 Default(0) or user spec.(1) values used"
        0.500 X-factor <= 0.5"
n
         30.000 K-lag (seconds)"
**
         0.500 Beta weighting factor"
**
         75.000 Routing time step (seconds)"
             1 No. of sub-reaches"
Peak outflow
"
                                                  0.054 c.m/sec"
                Peak outflow 0.054 c.m/sec" 0.049 0.054 0.054 0.000 c.m/sec"
"
               HYDROGRAPH Combine 3"
11
               6 Combine "
**
              3 Node #"
m

      Maximum flow
      0.054
      c.m/se

      Hydrograph volume
      262.019
      c.m"

      0.049
      0.054
      0.054
      0.054"

      HYDROGRAPH
      Confluence
      202"

m
                                                              c.m/sec"
**
11
               7 Confluence "
```

```
11
          202 Node #"
**
             Maximum flow
                                           0.144 c.m/sec"

      Maximum flow
      0.144
      c.m/s

      Hydrograph volume
      255.844
      c.m"

      0.049
      0.144
      0.054
      0.000"

**
              POND DESIGN"
**
         0.144 Current peak flow c.m/sec"
**
         0.080 Target outflow c.m/sec"
**
         255.8 Hydrograph volume c.m"
            3. Number of stages"
**
       223.000 Minimum water level
                                       metre"
**
       223.150 Maximum water level metre"
       223.000 Starting water level metre"
"
             0
                 Keep Design Data: 1 = True; 0 = False"
**
                 Level Discharge Volume"
                 223.000 0.00100 0.4900"
                 223.075 0.08990 10.370"
223.150 0.08500 39.990"
11
**
              Peak outflow
                                           0.088 c.m/sec"
**
              Maximum level
                                         223.100 metre"
                                         20.197
**
              Maximum storage
Centroidal lag
                                                   c.m"
**
                                           1.530 hours"
              Centroidal lag
                   0.049 0.144 0.088 0.000 c.m/sec"
**
**
             HYDROGRAPH Next link "
"
             5 Next link "
• •
                   0.049 0.088 0.088 0.000"
  51
           PIPE DESIGN"
11
         0.088 Current peak flow c.m/sec"
* *
         0.013 Manning 'n'"
11
         0.375 Diameter metre"
**
         0.400 Gradient %"
11
              Depth of flow
                                            0.253 metre"
              Velocity
                                            1.114 m/sec"
11
              Pipe capacity
                                            0.111 c.m/sec"
77
                                            0.218 metre"
              Critical depth
11
  53
             ROUTE Pipe Route 24"
,,
         23.60 Pipe Route 24 Reach length (metre)"
"
        0.000 	 X-factor <= 0.5"
**
        15.882 K-lag (seconds)"
77
         0.000 Default(0) or user spec.(1) values used"
,,
        0.500 X-factor <= 0.5"
"
        30.000 K-lag (seconds)"
7.5
        0.668 Beta weighting factor"
77
        46.154 Routing time step ( seconds)"
**
             1 No. of sub-reaches"
**
             Peak outflow
                                            0.088 c.m/sec"
**
                                         0.088 0.000 c.m/sec"
                   0.049 0.088
  40
             HYDROGRAPH Combine 3"
77
             6 Combine "
,,
                 Node #"
**
                 **
              Maximum flow
                                           0.141 c.m/sec"
             Hydrograph volume
11
                                         510.291
                                                    c.m"
                                         0.088 0.141"
**
              0.049 0.088
            HYDROGRAPH Confluence 3"
**
  40
             7 Confluence "
**
             3 Node #"
11
11
              Maximum flow
             Maximum flow 0.141 c.m/
Hydrograph volume 510.291 c.m"
                                           0.141 c.m/sec"
```

```
**
                     0.049 0.141 0.088
                                                0.000"
  51
         PIPE DESIGN"
**
        0.141 Current peak flow c.m/sec"
ŦŦ
        0.013 Manning 'n'"
11
        0.450 Diameter metre"
        0.400 Gradient %"
77
             Depth of flow
                                        0.300 metre"
,,
             Velocity
                                         1.255 m/sec"
11
             Pipe capacity
                                         0.180 c.m/sec"
             Critical depth
                                          0.263
**
  53
             ROUTE Pipe Route 15"
        15.00 Pipe Route 15 Reach length (metre)"
**
        0.000 X-factor <= 0.5"
        8.963 K-lag ( seconds)"
77
        0.000 Default(0) or user spec.(1) values used"
        0.500 	 X-factor <= 0.5"
**
       30.000 K-lag ( seconds) "
        0.797 Beta weighting factor"
**
       40.000 Routing time step (seconds)"
**
           1 No. of sub-reaches"
**
                    0.141 c.m/sec"
0.049 0.141 0.000 c.m/sec"
            Peak outflow
11
  40
            HYDROGRAPH Combine 201"
**
           6 Combine "
**
          201 Node #"
**
11
           Maximum flow
                                        0.274 c.m/sec"
            Hydrograph volume 756.147 c.m"

0.049 0.141 0.141 0.274"

HYDROGRAPH Confluence 201"
**
**
  40
FF
           7 Confluence "
"
         201 Node #"
**
            Maximum flow
                                        0.274 c.m/sec"
             Hydrograph volume 756.147 c.m" 0.049 0.274 0.141 0.000"
FF
FF
          PIPE DESIGN"
  51
"
        0.274 Current peak flow c.m/sec"
77
        0.013 Manning 'n'"
**
        0.525 Diameter metre"
ŦŦ
        0.400 Gradient %"
**
            Surcharged HGL
                                        0.407 %"
**
             Velocity
                                        1.267 m/sec"
                                         0.272 c.m/sec"
77
             Pipe capacity
**
            Critical depth
                                         0.000 metre"
  53
            ROUTE Pipe Route 16"
**
        15.70 Pipe Route 16 Reach length ( metre)"
FF
        0.000 X-factor <= 0.5"
**
        8.963 K-lag (seconds)"
"
        0.000 Default(0) or user spec.(1) values used"
11
        0.500 X-factor <= 0.5"
       30.000 K-lag (seconds)"
**
       0.797 Beta weighting factor"
**
       40.000 Routing time step ( seconds)"
**
            1 No. of sub-reaches"
            Peak outflow
                                         0.274 c.m/sec"
77
                    0.049 0.274
                                      0.274 0.000 c.m/sec"
 40
            HYDROGRAPH Next link "
**
            5 Next link "
77
                    0.049 0.274 0.274 0.000"
```

```
* *
  40
             HYDROGRAPH Undo"
             1 Undo"
17
                      0.049
                                0.274 0.274 0.000"
  56
              DIVERSION"
77
           201
               Node number"
11
         0.174 Overflow threshold"
**
         1.000 Required diverted fraction"
             O Conduit type; 1=Pipe; 2=Channel"
                                     0.100
              Peak of diverted flow
                                                    c.m/sec"
              Volume of diverted flow
                                          62.197
              DIV00201.005hyd"
"
              Divert to Overland Flow 0.1 cms"
77
                      0.049
                              0.274
                                         0.174
                                                    0.000 c.m/sec"
              HYDROGRAPH Next link "
**
             5 Next link "
77
                      0.049
                                0.174
                                        0.174
                                                  0.000"
11
             HYDROGRAPH Undo"
  40
             1 Undo"
**
                      0.049
                                0.274
                                        0.174
                                                  0.000"
**
  56
             DIVERSION"
**
             3
               Node number"
"
               Overflow threshold"
         0.150
"
         1.000 Required diverted fraction"
**
             O Conduit type; 1=Pipe; 2=Channel"
**
              Peak of diverted flow
                                          0.124
                                                     c.m/sec"
**
              Volume of diverted flow
                                          90.997
                                                     c.m"
Ŧ F
              DIV00003.005hyd"
**
              Diversion to Dverland Flow 0.024 cms"
**
                      0.049
                              0.274
                                         0.150
                                                    0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
**
             5 Next link "
**
                     0.049
                              0.150
                                         0.150
                                                    0.000"
"
  54
             POND DESIGN"
77
         0.150
               Current peak flow
                                     c.m/sec"
**
         0.090
                Target outflow c.m/sec"
**
         665.1
               Hydrograph volume
                                     c.m"
"
            3.
                Number of stages"
77
       221.600
               Minimum water level
                                       metre"
**
       223.100 Maximum water level
                                      metre"
**
                Starting water level
       221.600
                                       metre"
                Keep Design Data: 1 = True; 0 = False"
**
                  Level Discharge Volume"
77
                 221.600 0.01000 0.01000"
                         0.04960
                 222.350
                                    99.140"
**
                 223.100 0.06830
                                     262.660"
             Peak outflow
                                                    c.m/sec"
                                            0.068
77
             Maximum level
                                          223.072
                                                    metre"
             Maximum storage
                                          256.632
                                                    c.m"
**
             Centroidal lag
                                            2.466
                                                  hours"
                   0.049
                           0.150
                                       0.068
                                               0.000 c.m/sec"
  40
             HYDROGRAPH Next link "
               Next link "
**
                      0.049
                               0.068
                                         0.068
                                                    0.000"
  51
             PIPE DESIGN"
**
        0.068
                Current peak flow
                                    c.m/sec"
        0.013
                Manning 'n'"
77
        0.375
                Diameter
                           metre"
        0.400
                Gradient
**
             Depth of flow
                                            0.211
                                                    metre"
             Velocity
                                            1.053
                                                    m/sec"
```

```
**
             Critical depth
             Pipe capacity
                                         0.111
                                                c.m/sec"
**
                                        0.189
                                                metre"
            ROUTE Pipe Route 59"
77
        59.40 Pipe Route 59 Reach length (metre)"
**
        0.222 X-factor <= 0.5"
77
       42.294 K-lag (seconds)"
77
        0.000 Default(0) or user spec.(1) values used"
**
        0.500 X-factor <= 0.5"
*
       30.000 K-lag (seconds)"
**
        0.500 Beta weighting factor"
       60.000 Routing time step (seconds)"
**
           1 No. of sub-reaches"
**
            Peak outflow
                                        0.068
                                                c.m/sec"
**
                    0.049 0.068
                                      0.068 0.000 c.m/sec"
            HYDROGRAPH Next link "
  40
**
            5 Next link "
**
                   0.049
                            0.068
                                      0.068
                                               0.000"
  51
            PIPE DESIGN"
77
        0.068 Current peak flow c.m/sec"
* *
        0.013 Manning 'n'"
,,
        0.375 Diameter metre"
        0.400 Gradient %"
F #
            Depth of flow
                                        0.211 metre"
* *
            Velocity
                                        1.053 m/sec"
**
             Pipe capacity
                                        0.111
                                                c.m/sec"
                                        0.189 metre"
            Critical depth
11
            ROUTE Pipe Route 31".
11
        31.30 Pipe Route 31 Reach length (metre)"
77
        0.000 X-factor <= 0.5"
11
       22.291 K-lag (seconds)"
* *
       0.000 Default(0) or user spec.(1) values used"
,,
        0.500 X-factor <= 0.5"
**
       30.000 K-lag (seconds)"
77
       0.514 Beta weighting factor"
**
       42.857 Routing time step (seconds)"
**
           1 No. of sub-reaches"
**
            Peak outflow
                                        0.068 c.m/sec"
**
                    0.049 0.068
                                      0.068 0.000 c.m/sec"
 40
           HYDROGRAPH Next link "
**
           5 Next link "
**
                    0.049
                            0.068
                                      0.068
                                               0.000"
**
 51
           PIPE DESIGN"
        0.068 Current peak flow c.m/sec"
**
        0.013 Manning 'n'"
        0.375 Diameter metre"
71
        0.400 Gradient %"
"
            Depth of flow
                                         0.211 metre"
**
                                         1.053 m/sec"
            Velocity
* *
            Pipe capacity
                                        0.111 c.m/sec"
            Critical depth
                                        0.189 metre"
            ROUTE Pipe Route 11"
77
        10.50 Pipe Route 11 Reach length ( metre)"
11
        0.000 X-factor <= 0.5"
ŦŦ
        7.479 K-lag (seconds)"
**
        0.000 Default(0) or user spec.(1) values used"
**
        0.500
              X-factor <= 0.5"
**
       30.000 K-lag (seconds)"
11
       0.767 Beta weighting factor"
11
       30.000 Routing time step ( seconds)"
77
               No. of sub-reaches"
           1
```

**		Peak outflow	0.067	c.m/sec"	
**		0.049 0.068	0.067	0.000 c.m/sec	11
**	40	HYDROGRAPH Combine 99	999"		
77		6 Combine "			
**		9999 Node #"			
**		Ħ			
**		Maximum flow	0.101	c.m/sec"	
71		Hydrograph volume	719.259	c.m"	
77		0.049 0.068	0.067	0.101"	
77	40	HYDROGRAPH Confluence	9999"		
**		7 Confluence "			
11		9999 Node #"			
**		TI .			
**		Maximum flow	0.101	c.m/sec"	
**		Hydrograph volume	719.259		
11		0.049 0.101	0.067	0.000"	
**	38	START/RE-START TOTALS 9999) "		
**		3 Runoff Totals on EXIT"			
**		Total Catchment area		1.154	hectare"
71		Total Impervious area		0.922	hectare"
77		Total % impervious		79.873"	
ŧτ	19	EXIT"			